

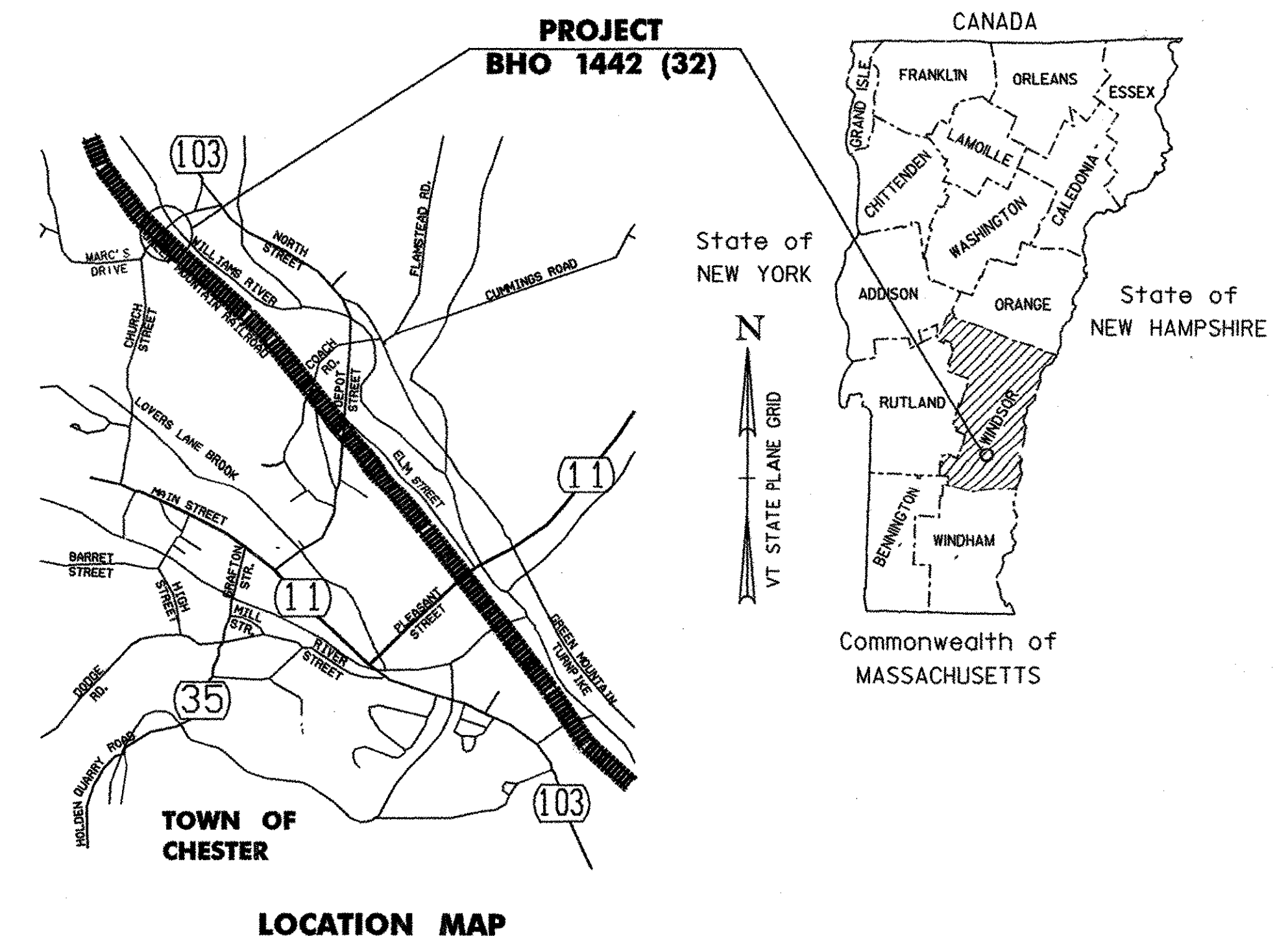
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- 21-22. ROADWAY CROSS SECTIONS

STATE OF VERMONT AGENCY OF TRANSPORTATION



PROPOSED IMPROVEMENT BRIDGE PROJECT TOWN OF CHESTER COUNTY OF WINDSOR



RECORD PLANS

CONTRACTOR: FW WHITCOMB CONSTRUCTION CORP. - WALPOLE, NH
RESIDENT ENGINEER: WILL STODDARD
CONSTRUCTION BEGAN: MAY 26, 2004
CONSTRUCTION COMPLETE: NOVEMBER 19, 2004
RECORD PLANS BY: WILL STODDARD & N. GARBACIK

I HEREBY CERTIFY THAT ALL THE CONSTRUCTION REQUIRED BY THIS SET OF DRAWINGS HAS BEEN ACCOMPLISHED AS INDICATED HEREIN.

BY *Will Stoddard* RESIDENT ENGINEER
DATE 10-30-06

NOTE: Any further information concerning final quantities, amounts or other details relative to this project may be found at Central Files in the electronic archives.

CHURCH ST. (TH 5), BRIDGE NO 11

PROJECT LOCATION: BEGINNING AT A POINT ON CHURCH STREET (TH 5) APPROXIMATELY 0.15 MILES WEST OF THE JUNCTION OF VT 103 AND CHURCH STREET AND EXTENDING EASTERLY ALONG CHURCH STREET (TH 5) FOR 0.033 MILES.

PROJECT DESCRIPTION: REMOVAL OF EXISTING CONCRETE DECK, STEEL BEAMS AND BEARINGS; CONSTRUCTION OF NEW STEEL BEAM AND CAST-IN-PLACE CONCRETE DECK SUPERSTRUCTURE ON EXISTING SUBSTRUCTURES; AND RELATED APPROACH WORK.

LENGTH OF STRUCTURE : 116 FEET = 0.022 MILES
LENGTH OF ROADWAY : 60 FEET = 0.011 MILES
LENGTH OF PROJECT : 176 FEET = 0.033 MILES

STANDARDS

B-5	SLOPE GRADING, EMBANKMENTS (EARTH & ROCK), MUCK EXCAVATION	06-01-94
E-100	CONSTRUCTION APPROACH SIGNS	01-02-04
E-101	CONSTRUCTION SIGN DETAILS	05-30-03
E-102	CONSTRUCTION SIGN DETAILS	05-30-03
E-102A	CONSTRUCTION SIGN DETAILS	08-08-95
E-103	MAINLINE TRAFFIC CONTROL	09-23-98
E-106	DIVIDED HIGHWAY, ONE LANE CLOSED	08-08-95
E-107	TRAFFIC CONTROL - MISCELLANEOUS DETAILS	06-30-03
E-107A	DELINEATION, BARRICADES AND DETOURS FOR CONSTRUCTION AREAS	08-08-95
E-108	BREAKAWAY BARRICADE DETAILS	08-18-95
E-110	CONSTRUCTION ZONE - LONGITUDINAL DROP OFFS	08-08-95
E-121	MAJOR MAINTENANCE OPERATION LANE CLOSURE	08-08-95
E-160	STANDARD SIGN PLACEMENT - CONVENTIONAL ROADS	05-20-99
G-1	FLANGED CHANNEL STEEL SIGN POST	01-03-00
G-1D	STEEL BEAM GUARDRAIL W/ STEEL AND WOOD POSTS	01-03-00
G-18	STEEL BEAM GUARDRAIL APPROACH END TERMINAL, HEAVY DUTY STEEL BEAM GUARDRAIL, STEEL BEAM GUARDRAIL TRAILING END TERMINAL, STEEL BEAM MEDIAN BARRIER, ANCHOR FOR STEEL BEAM RAIL	06-01-94
SB-R6-82	PRECAST CONCRETE TEMPORARY TRAFFIC BARRIER	01-06-95
SB-R7-90	BRIDGE RAILING HEAVY DUTY STEEL BEAM	01-11-95
T-1	BRIDGE RAILING HEAVY DUTY STEEL BEAM	06-01-94
T-2	TEMPORARY EROSION CONTROL DETAILS	06-01-94

CONVENTIONAL SYMBOLS

COUNTY LINE	
TOWN LINE	
LIMITS OF ACCESS	
POINT OF ACCESS	
FENCE LINE	
STONE WALL	
TRAVELED WAY	
GUARD RAIL	
RAILROAD	
SURVEY LINE	
CULVERT	
POWER POLE	
TELEPHONE POLE	
TREES	
CONTROL OF ACCESS	
PROPERTY LINE	
R.O.W. TAKING LINE	
SLOPE RIGHTS	
TOP OF CUT	
TOE OF SLOPE	

SURVEYED BY : DUBOIS & KING
SURVEYED DATE : 12/99

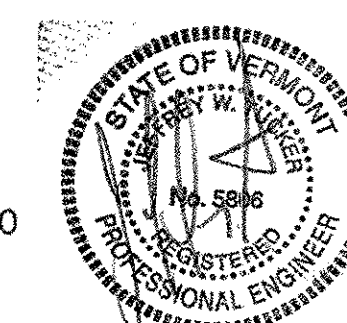
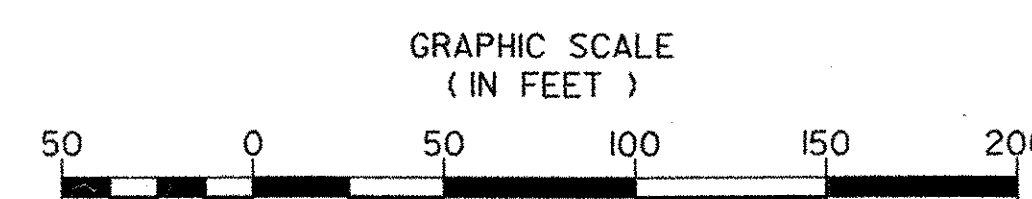
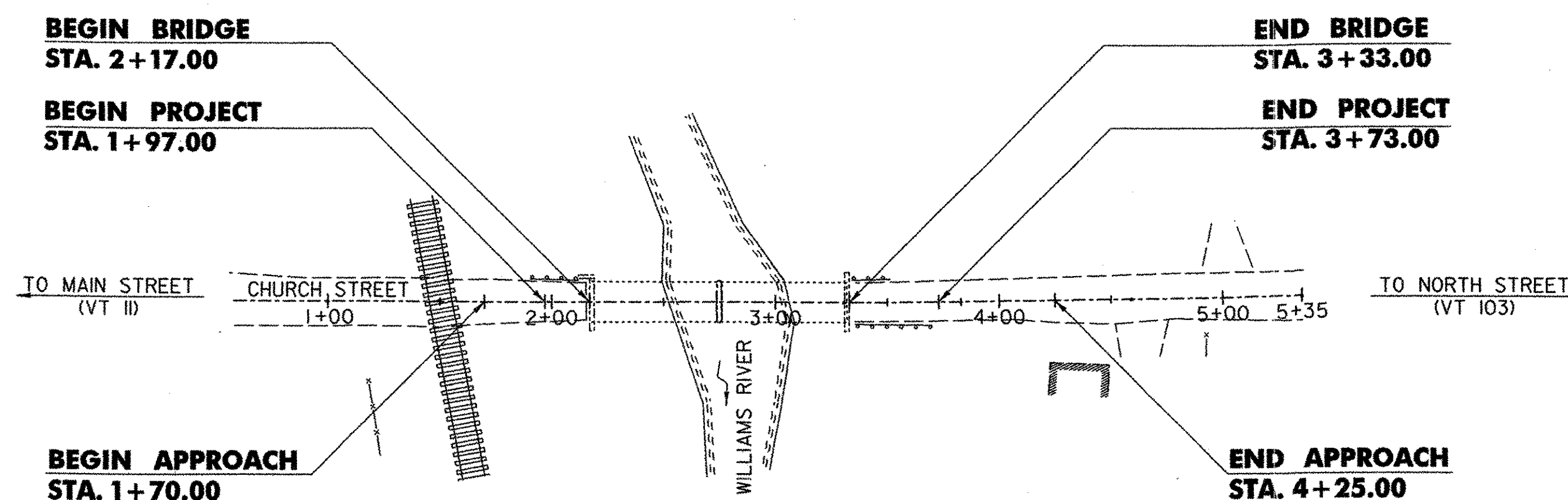
DATUM

VERTICAL LOCAL
HORIZONTAL LOCAL

PLANS PREPARED BY:

DuBois & King

ENGINEERING PLANNING MANAGEMENT DEVELOPMENT

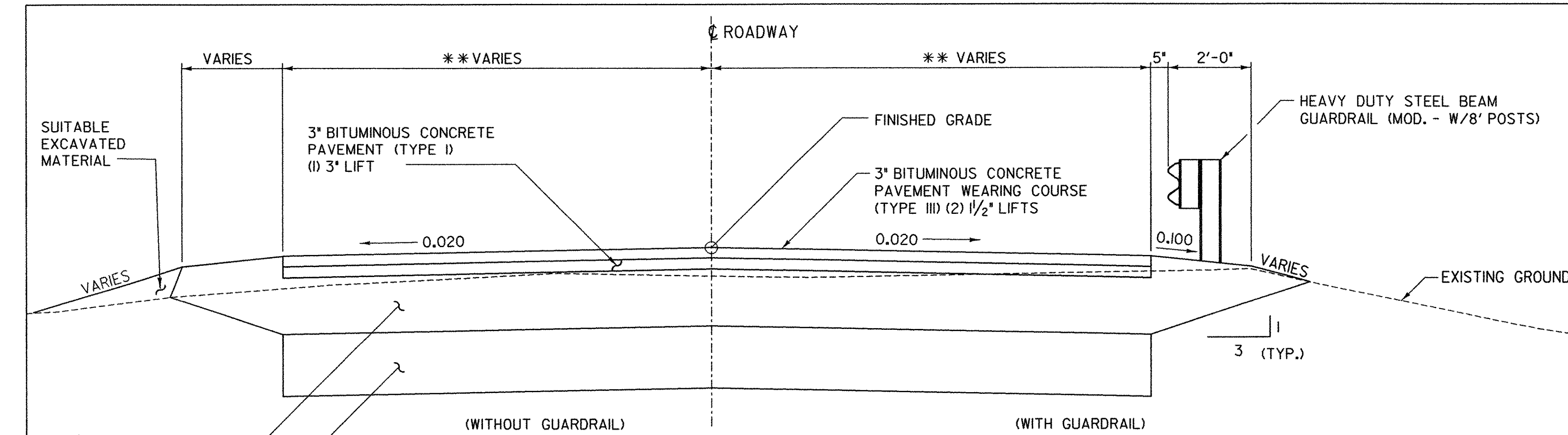


THESE PLANS ARE SUBJECT TO SUCH ENGINEERING CHANGES AS MAY BE REQUIRED BY THE FEDERAL HIGHWAY ADMINISTRATION OR THE DIRECTOR OF PROJECT DEVELOPMENT. CONSTRUCTION IS TO BE CARRIED ON IN ACCORDANCE WITH THESE PLANS AND THE STANDARD SPECIFICATIONS FOR CONSTRUCTION DATED 2001, AS APPROVED BY THE FEDERAL HIGHWAY ADMINISTRATION ON JANUARY 4, 2001 FOR USE ON THIS PROJECT, INCLUDING ALL SUBSEQUENT REVISIONS AND SUCH REVISED SPECIFICATIONS AND SPECIAL PROVISIONS AS ARE INCORPORATED IN THESE PLANS.

APPROVED *Susan S. Spaulding* DATE 2-5-04
SUSAN S. SPAULDING
MUNICIPAL PROJECT MANAGER

APPROVED *Robert J. King* DATE 2/19/04
DIRECTOR OF PROGRAM DEVELOPMENT

PROJECT NAME :
CHESTER BHO 1442 (32)



TYPICAL ROADWAY SECTION

SCALE 1/2" = 1'-0"
1 0 2

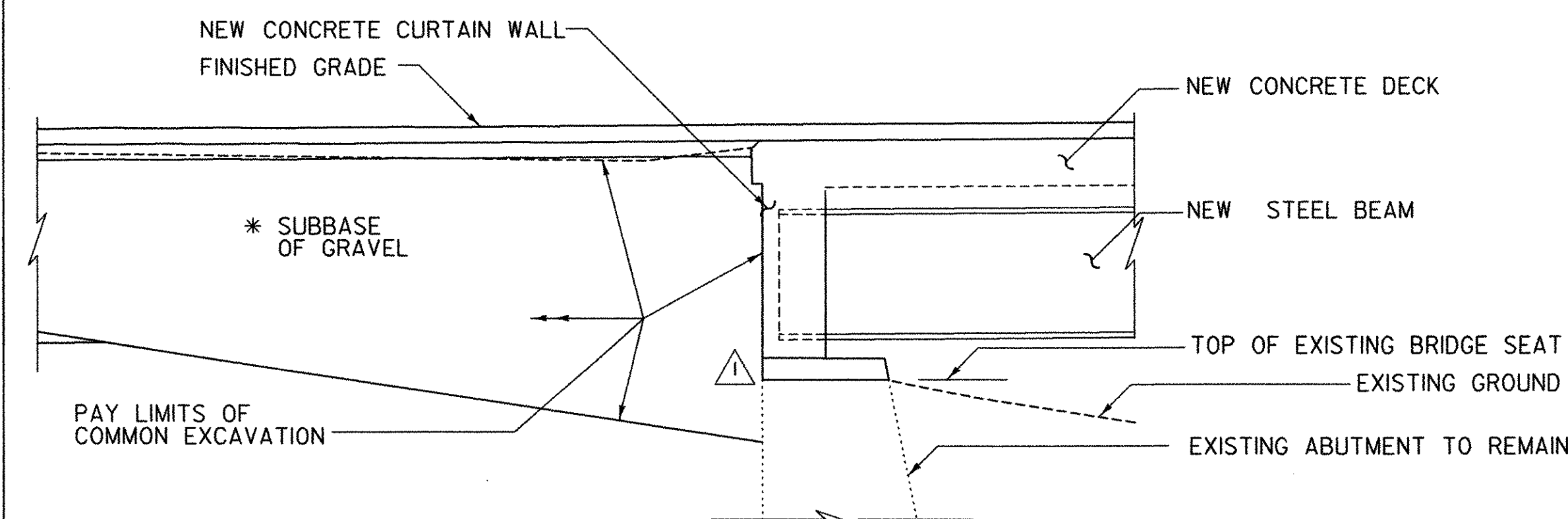
* FOR MATERIAL DEPTH DETAILS SEE SHEET 6
** ROADWAY WIDTH VARIES THROUGHOUT PROJECT. REFER TO PLAN SHEET AND ROADWAY CROSS SECTIONS FOR WIDTHS AT SPECIFIC STATIONS.

TRAFFIC DATA

2000 ADT = 530
2000 DHV = 75
2000 ADTT = 11
2020 ADT = 725
2020 DHV = 105
2020 ADTT = 14
T = 2%
D = 60%
DESIGN SPEED = 25 MPH
ESALS (FLEXIBLE) TOTAL BOTH DIRECTIONS (2000-2020) 229,000 (2000-2040) 731,000

MATERIAL TOLERANCE TABLE

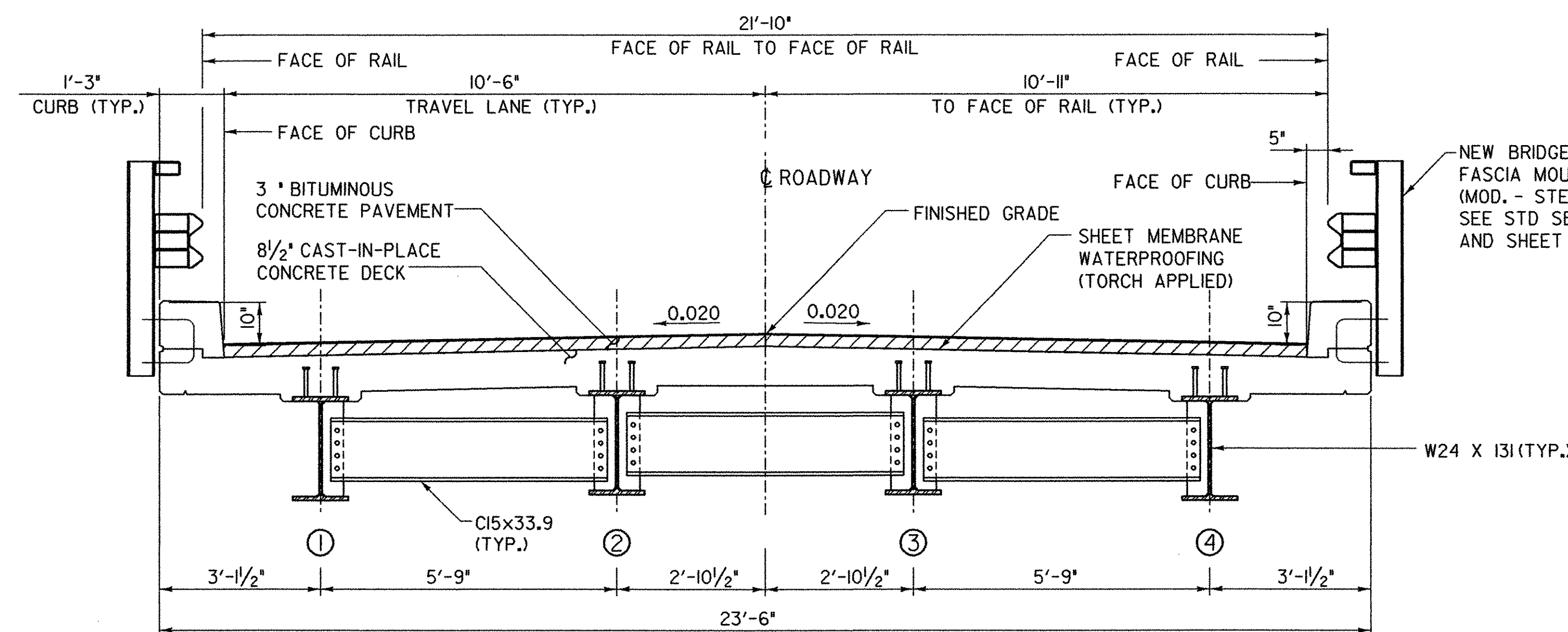
MATERIAL ITEM	TOLERANCE
BITUMINOUS CONCRETE PAVEMENT	± 1/4" TOTAL THICKNESS
SUBBASE OF CRUSHED GRAVEL	± 1"
GRANULAR BACKFILL FOR STRUCTURES	± 1"



TYPICAL ABUTMENT SECTION

SCALE 1/2" = 1'-0"
1 0 2

NOTES:
THE PAY LIMITS OF STRUCTURE EXCAVATION AND GRANULAR BACKFILL FOR THE NEW WING WALLS, SHALL BE AS SHOWN ON SHEET 18.



TYPICAL BRIDGE SECTION

SCALE 1/2" = 1'-0"
1 0 2

TYPICAL SECTIONS

3" BITUMINOUS CONCRETE PAVEMENT (TYPE III)
3" BITUMINOUS CONCRETE PAVEMENT (TYPE I)

BRIDGE DECK PAVEMENT

TOP LIFT = 1 1/2" TYPE III
BOTTOM LIFT = 1 1/2" TYPE IV
EMULSIFIED ASPHALT BETWEEN LIFTS

* NOTE: ALL BITUMINOUS CONCRETE PAVEMENT SHALL BE PG-58-34

FINAL HYDRAULICS REPORT

HYDROLOGIC DATA

DRAINAGE AREA= _____
CHARACTER OF TERRAIN: _____
CHARACTER & TYPE OF STREAM: _____
NATURE OF STREAMBED: _____
02.33= _____ 050= _____
Q10= _____ Q100= _____
Q25= _____ Q500= _____
DATE OF FLOOD OF RECORD: _____
WATER SURFACE ELEV.: _____ ESTIMATED DISCHARGE: _____
NATURAL STREAM VELOCITY @ Q _____
ICE CONDITIONS: _____ DEBRIS: _____
DOES THE STREAM REACH MAXIMUM HIGHWATER ELEVATION RAPIDLY _____
IS ORDINARY RISE RAPID? _____
IS STAGE AFFECTED BY UPSTREAM OR DOWNSTREAM CONDITIONS? _____
IF YES, DESCRIBE: _____
WATERSHED STORAGE _____ HEADWATERS _____ UNIFORM THROUGHOUT WATERSHED _____
IMMEDIATELY ABOVE SITE _____

EXISTING STRUCTURE

STRUCTURE TYPE: TWO SPAN STEEL BEAM WITH CONCRETE DECK YEAR BUILT: 1940
CLEAR SPAN (NORMAL TO STREAM): 109 FT.
VERTICAL CLEARANCE ABOVE STREAMBED: _____
WATERWAY OF FULL OPENING: _____
DISPOSITION OF STRUCTURE: REMOVE EXISTING SUPERSTRUCTURE
EXISTING ABUTMENTS AND PIER ARE TO REMAIN
TYPE OF MATERIAL UNDER SUBSTRUCTURE: _____
WATER SURFACE ELEV. @ Q2.33= _____ VELOCITY= _____
Q10= _____
Q25= _____
Q50= _____
Q100= _____
LONG TERM STREAM BED CHANGES: _____
IS THE ROADWAY OVERTOPPED BELOW THE Q100? _____ FREQUENCY: _____
RELIEF ELEVATION: _____ DISCHARGE OVER ROAD @ Q100: _____

UPSTREAM STRUCTURE: TOWN: _____ DISTANCE: _____
HIGHWAY NO.: _____ STRUCTURE NO.: _____
NOTE: _____
DOWNSTREAM STRUCTURE: TOWN: _____ DISTANCE: _____
HIGHWAY NO.: _____ STRUCTURE NO.: _____
NOTE: _____

DESIGN CRITERIA:
1. DESIGN LIVE LOAD AASHTO HS-25-44
2. DESIGN SPAN 114.0 FT.
3. ALLOWABLE LOAD FOR SPREAD FOOTINGS ON SOIL N/A ON LEDGE N/A
4. ALLOWABLE LOAD FOR PILING N/A TYPE _____ ESTIMATED LENGTH N/A
5. STRUCTURAL STEEL AASHTO M270/M270, GRADE 50 W
6. REINFORCING STEEL GRADE 60
7. CONCRETE, HIGH PERFORMANCE CLASS A f'c 4000 psi
CONCRETE, HIGH PERFORMANCE CLASS B f'c 3500 psi

TRAFFIC MAINTENANCE:
1. IS TRAFFIC TO BE MAINTAINED? NO IF YES, ON EXISTING STRUCTURE N/A OR ON TEMPORARY BRIDGE N/A
EXISTING BRIDGE TO BE CLOSED, TRAFFIC TO BE DETOURED AROUND CONSTRUCTION SITE
2. TEMPORARY BRIDGE REQUIREMENTS: ONE OR TWO WAY N/A TRAFFIC CONTROL SIGNALS REQUIRED N/A
MINIMUM CLEAR SPAN (NORMAL TO STREAM): _____ VERTICAL CLEARANCE ABOVE STREAMBED: _____
WATERWAY OF FULL OPENING: _____
ARE SIDEWALKS REQUIRED? _____ IF SO, ON WHAT SIDE? _____
STRUCTURE TYPE: _____

PROPOSED STRUCTURE

STRUCTURE TYPE: STEEL BEAMS WITH COMPOSITE CONCRETE DECK (2 SPAN CONTINUOUS)
CLEAR SPAN (NORMAL TO STREAM): 109 FT.
VERTICAL CLEARANCE ABOVE STREAMBED: _____
WATERWAY OF FULL OPENING: _____
WATER SURFACE ELEV. @ Q2.33= _____ VELOCITY= _____
Q10= _____
Q25= _____
Q50= _____
Q100= _____
IS THE ROADWAY OVERTOPPED BELOW THE Q100? _____ FREQUENCY: _____
RELIEF ELEVATION: _____ DISCHARGE OVER ROAD @ Q100: _____
AVERAGE LOW ELEVATION OF SUPERSTRUCTURE: _____
VERTICAL CLEARANCE @ Q25 _____
SCOUR: _____
REQUIRED CHANNEL PROTECTION: _____

PERMIT INFORMATION

AVERAGE DAILY FLOW: _____ DEPTH: _____
ORDINARY LOW WATER: _____ DEPTH: _____
ORDINARY HIGH WATER: _____ DEPTH: _____

ADDITIONAL COMMENTS

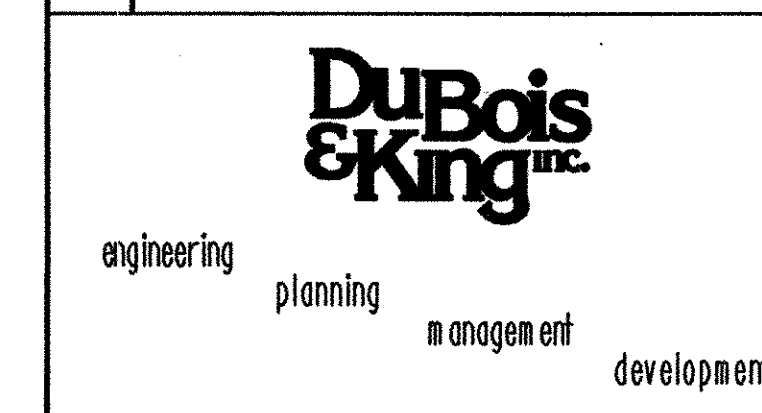
NO CHANNEL OR OTHER WORK IS BEING PERFORMED UNDER THIS PROJECT, WHICH HAS ANY IMPACT ON HYDRAULICS. THEREFORE, A HYDRAULIC ANALYSIS HAS NOT BEEN PERFORMED.

STRENGTH RF = $\frac{\phi M_N - 1.3 M_{DL}}{A \times M_{LL+I}}$
*SERVICEABILITY RF = B $\left[\frac{.95 F_y S_{LL+I} - M_{DL} \frac{S_{LL+I}}{S_{DL}} - M_{SD} \frac{S_{LL+I}}{S_{DL}}}{1.67 M_{LL+I}} \right]$

LOAD FACTOR LOAD RATING (TONS)

LOADING LEVELS (LOAD FACTOR)	TRUCK						
	H	HS	3S2	6 AXLE	3A STR	4A STR	5A SEMI
INVENTORY A=2.17 B=1.00	50	58					
POSTED A=1.55 B=1.40	70	82	109		38	79	103
OPERATING A=1.30 B=1.67		97	130	125	45	94	

REVISIONS		
NO.	DESCRIPTION	BY & DATE
1	CONCRETE ABUTMENT CAP ADDED TO TYPICAL ABUTMENT SECTION	APG 2/3/04



STATE OF VERMONT AGENCY OF TRANSPORTATION

Town Of CHESTER Bridge No. 11
Highway No. TH 5 Log Sta. -
Surv. Sta. -

CHURCH STREET OVER WILLIAMS RIVER

PRELIMINARY INFORMATION SHEET

Designed By B. C. AUSTIN Drawn By G. J. WOJCIECHOWSKI
Checked By J. TUCKER Date 03/01 Bridge Design Supervisor J. TUCKER Date 03/01

PROJECT CHESTER PROJECT NO. BHO 1442 (32)
I.G.C. Info. ...ZJ365p12 .dgn

D & K DWG NO. 11345 Sheet 2 of 22

02/10/2004

BRIDGE QUANTITY SHEET

NO.	ITEM	UNIT	QUANTITY BREAKDOWN							TOTAL	GRAND TOTAL
			SUPER STRUCT.	ABUTMENT. NO. 1	ABUTMENT. NO. 2	PIER	EROSION CONTROL	ROADWAY	FULL E & C		
203.15	COMMON EXCAVATION	CY						290		290	
203.28	EXCAVATION OF SURFACE AND PAVEMENT	CY						32		32	
203.31	SAND BORROW	CY						30		30	
204.25	STRUCTURE EXCAVATION	CY			70					70	
204.30	GRANULAR BACKFILL FOR STRUCTURES	CY			45					45	
210.10	COLD PLANING - BITUMINOUS CONCRETE	SY						110		110	
301.15	SUBBASE OF GRAVEL	CY						160		160	
406.27	MEDIUM DUTY BITUMINOUS CONC. PAVEMENT (PG 58-34)	TON	45					85		130	
501.33	CONCRETE, HIGH PERFORMANCE CLASS A	CY	95	1	1					97	
501.34	CONCRETE, HIGH PERFORMANCE CLASS B	CY		3	17	6				26	
506.50	STRUCTURAL STEEL (ROLLED BEAM)	LB	70450							70450	
507.15	REINFORCING STEEL	LB		45	1625					1670	
507.16	DRILLING AND GROUTING DOWELS	LF		25		15				40	
507.17	EPOXY COATED REINFORCING STEEL	LB	21400	100	250	750				22500	
508.15	SHEAR CONNECTORS (1144 - 7/8" X 7')	LS	1							1	
514.10	WATER REPELLENT	GAL	8	2	2	8				20	
519.20	SHEET MEMBRANE WATERPROOFING (TORCH APPLIED)	SY	300							300	
524.10	JOINT SEALER, HOT POURED	GAL	1							1	
525.43	BRIDGE RAILING-HDSB/FASCIA MOUNTED/HANDRAIL (MOD. - STEEL TUBING)	LF	271							271	
529.10	REMOVAL OF BRIDGE PAVEMENT	SY	280							280	
529.20	PARTIAL REMOVAL OF STRUCTURE	EA	0.7	0.1	0.1	0.1				1	
531.10	BEARING DEVICE ASSEMBLY	EA		4	4	4				12	
580.13	REPAIR CONCRETE SUBSTRUCTURE SURFACE CLASS I	SY		1	1	2				4	
580.14	REPAIR CONCRETE SUBSTRUCTURE SURFACE CLASS II	SY		1	1	2				4	
580.15	REPAIR CONCRETE SUBSTRUCTURE SURFACE CLASS III	CY				1				1	
609.10	DUST CONTROL WITH WATER	MGAL						10		10	
613.11	STONE FILL, TYPE II	CY						90		90	
621.21	HEAVY DUTY STEEL BEAM GUARDRAIL (MOD. - W/8' POSTS	LF						190		190	
621.60	ANCHOR FOR STEEL BEAM RAIL	EA						4		4	
621.80	REMOVAL AND DISPOSAL OF GUARD RAIL	LF						80		80	
621.90	TEMPORARY TRAFFIC BARRIER	LF						60		60	
630.10	UNIFORMED TRAFFIC OFFICERS	HR						50		50	
630.15	FLAGGERS	HR						50		50	
631.10	FIELD OFFICE - ENGINEERS	LS							1	1	
631.16	TESTING EQUIPMENT - CONCRETE	LS							1	1	
631.17	TESTING EQUIPMENT - BITUMINOUS	LS							1	1	
631.25	FIELD OFFICE - TELEPHONE (N.A.B.I.)	LU							1	1	
635.10	MOBILIZATION	LS	0.8					0.2		1	
641.10	TRAFFIC CONTROL	LS						1		1	
646.20	4" WHITE LINE	LF						510		510	
646.21	4" YELLOW LINE	LF						510		510	
649.31	GEOTEXTILE UNDER STONE FILL	SY						170		170	
649.51	GEOTEXTILE FOR SILT FENCE	SY					140			140	
651.15	SEED	LB					25			25	
651.18	FERTILIZER	LB					50			50	
651.20	AGRICULTURAL LIMESTONE	TON					1			1	
651.25	HAY MULCH	TON					1			1	
651.26	HAY BALES FOR EROSION CONTROL	EA					50			50	
651.35	TOPSOIL	CY					50			50	
652.10	EROSION AND SEDIMENT CONTROL PLAN	LS					1			1	
652.20	MONITORING EROSION AND SEDIMENT CONTROL PLAN	HR					80			80	
652.30	FIELD MAINTENANCE OF EROSION AND SEDIMENT CONTROL PLAN (N.A.B.I.)	LU					1			1	

02/13/2004

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION							
NO.	DESCRIPTION	BY & DATE								
			Town Of	CHESTER	Bridge No.	11				
			Highway No.	TH 5	Log Sta.	-				
					Surv. Sta.	-				
			CHURCH STREET OVER WILLIAMS RIVER							
			QUANTITY SHEET							
 ENGINEERING PLANNING MANAGEMENT DEVELOPMENT			Designed By		B. AUSTIN		Drawn By		G. J. WOJCIECHOWSKI	
			Checked By		J. TUCKER		Date		10/03	
							Bridge Design Supervisor			
							Date		10/03	
			PROJECT		CHESTER		PROJECT NO.		BHO 1442 (32)	
			I.G.C. Info.		...zj365q03 .dgn					
			D & K DWG NO.		11346		Sheet 3 of 22			

LAYOUT COORDINATES			
STA. 1+00.00	POB	N 99°1.94'	E 9890.62
STA. 2+17.00	BEGIN BRIDGE	N 100°18.10'	E 10001.90
STA. 3+30.18	PC	N 100°53.21'	E 10109.97
STA. 3+33.00	END BRIDGE	N 100°53.94'	E 10112.22
STA. 3+94.90	PI	N 100°73.07'	E 10171.09
STA. 4+59.61	PT	N 100°93.96'	E 10231.86
STA. 5+35.40	POE	N 101°18.73'	E 10303.97
HVCTRL 1	CONTROL POINT	N 100°00.00'	E 10000.00
HVCTRL 2	CONTROL POINT	N 101°01.85'	E 10068.70

BOB PANTO
805 CHURCH ST
CHESTER VT 05143
802-875-3476

BEGIN APPROACH
STA. 1+70.00

BEGIN PROJECT
STA. 1+97.00

BEGIN BRIDGE
STA. 2+17.00

END APPROACH
STA. 4+25.00

END PROJECT
STA. 3+73.00

END BRIDGE
STA. 3+33.00

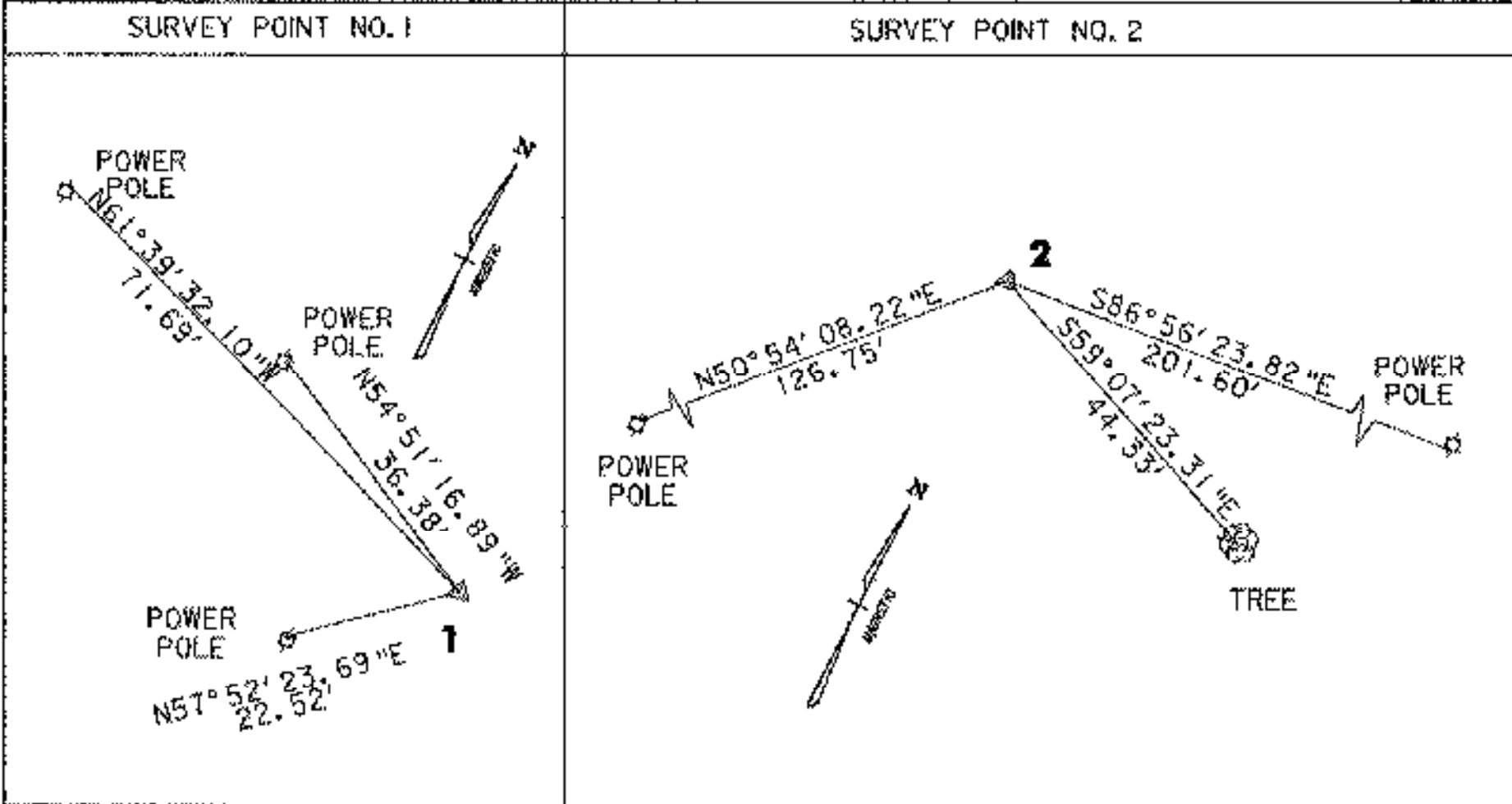
BENCHMARK #3
TOP OF CONCRETE BOUND
ELEVATION 98.3100

BENCHMARK #2
SPIKE IN POWER POLE #28
ELEVATION 102.6860

BENCHMARK #1
TOP OF SPINDLE NUT AT ARROW
ON FIRE HYDRANT
ELEVATION 105.5580

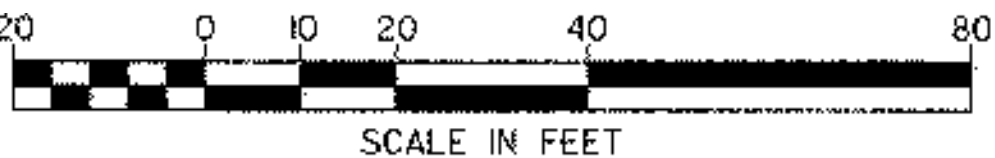
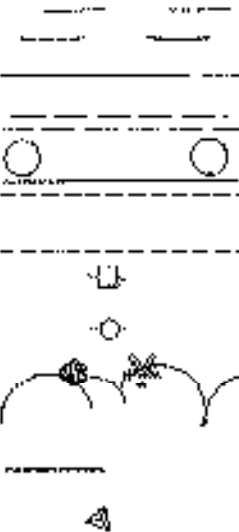
PLAN

TIES



LEGEND

- EXISTING EDGE OF RIVER
- EXISTING TRAVELED WAY
- EXISTING GUARD RAIL
- CULVERT
- UTILITY POLE
- TELEPHONE POLE
- TREES
- TREE LINE
- OVERHEAD LINES
- SURVEY POINT



CURVE DATA

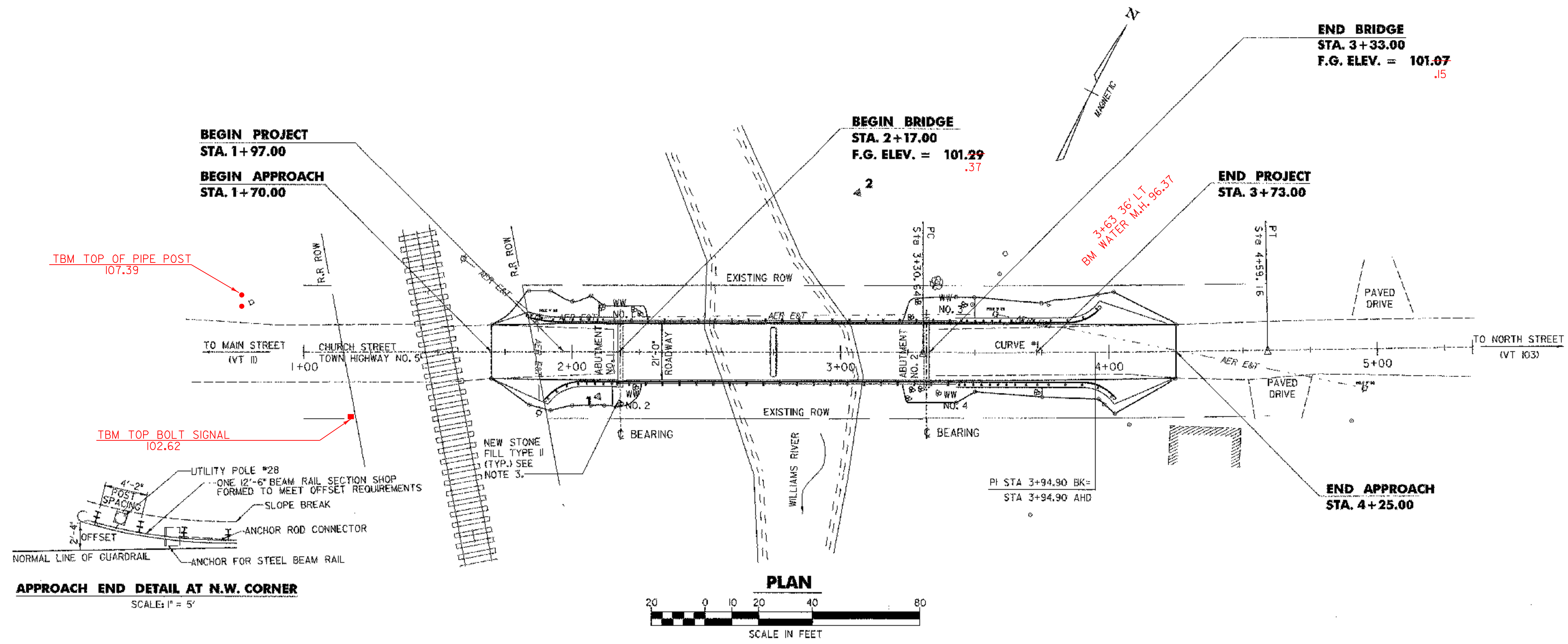
CURVE # 1	
Δ	= 0°57'50"
R	= 7639.50'
T	= 64.26'
L	= 128.52'
D	= 0°45'00"

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
			Town Of	CHESTER
			Highway No.	TH 5
			CHURCH STREET OVER WILLIAMS RIVER	
			Bridge No.	11
			Log Sta.	-
			Surv. Sta.	-
			TIE SHEET	
			Designed By	G. J. WOJCIECHOWSKI
			Drawn By	G. J. WOJCIECHOWSKI
			Checked By	J. TUCKER
			Date	03/01
			Bridge Design Supervisor	J. TUCKER
			Date	03/01
			PROJECT	CHESTER
			PROJECT NO.	BHO 1442 (32)
			I.G.C. Info. ...zj365104.dgn	
			D & K DWG NO.	11347
			Sheet 4 of 22	

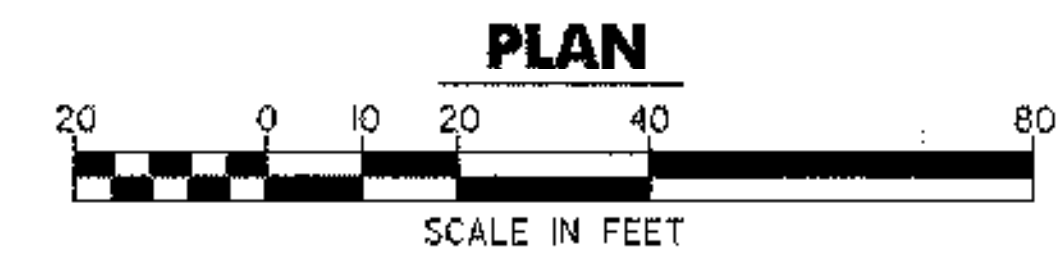


02/03/2004

FINAL GRADE RAISED 1" TO OBTAIN 1" MIN BEAM HAUNCH



APPROACH END DETAIL AT N.W. CORNER
SCALE: 1" = 5'



ANCHOR FOR STEEL BEAM GUARDRAIL

STA 1+86.70 LT STA 3+90.50 LT
STA 1+97.40 RT STA 3+95.80 RT

COLD PLANING-BITUMINOUS PAVEMENT

STA 1+70.00 TO STA 1+85.00
STA 3+95.00 TO STA 4+25.00

REMOVAL AND DISPOSAL OF GUARDRAIL

32' 1+80.00 STA 1+91.00 TO STA 2+12.00 LT
18' STA 3+34.00 TO STA 3+51.00 LT 3+52.00
40' STA 3+34.00 TO STA 3+70.00 RT 3+74.00

HEAVY DUTY STEEL BEAM GUARDRAIL (MOD. - W/8' POSTS)

STA 1+82.50 TO STA 2+07.50 LT
STA 1+89.75 TO STA 2+14.75 RT
STA 3+47.00 TO STA 3+97.00 LT
STA 3+46.00 TO STA 4+02.25 RT

4" WHITE LINES

STA 1+70.00 TO STA 4+25.00 SOLID RT & LT

4" YELLOW LINES (DOUBLE CENTERLINE)

STA 1+70.00 TO STA 4+25.00 SOLID RT & LT

STONE FILL, TYPE II

STA 2+15 TO STA 2+25 RT
STA 2+10 TO STA 2+28 LT
STA 3+24 TO STA 3+75 RT
STA 3+24 TO STA 3+50 LT

LEGEND	
EXISTING EDGE OF RIVER	---
EXISTING TRAVELED WAY	---
PROPOSED TRAVELED WAY	---
ROW	---
GUARD RAIL	○ ○
CULVERT	---
UTILITY POLE	□
TELEPHONE POLE	○
TREES	● *
TREE LINE	~~~~~

CURVE DATA

CURVE # 1	
Δ	0°57'50"
R	7639.50'
T	64.26'
L	128.52'
D	0°45'00"

NOTES:

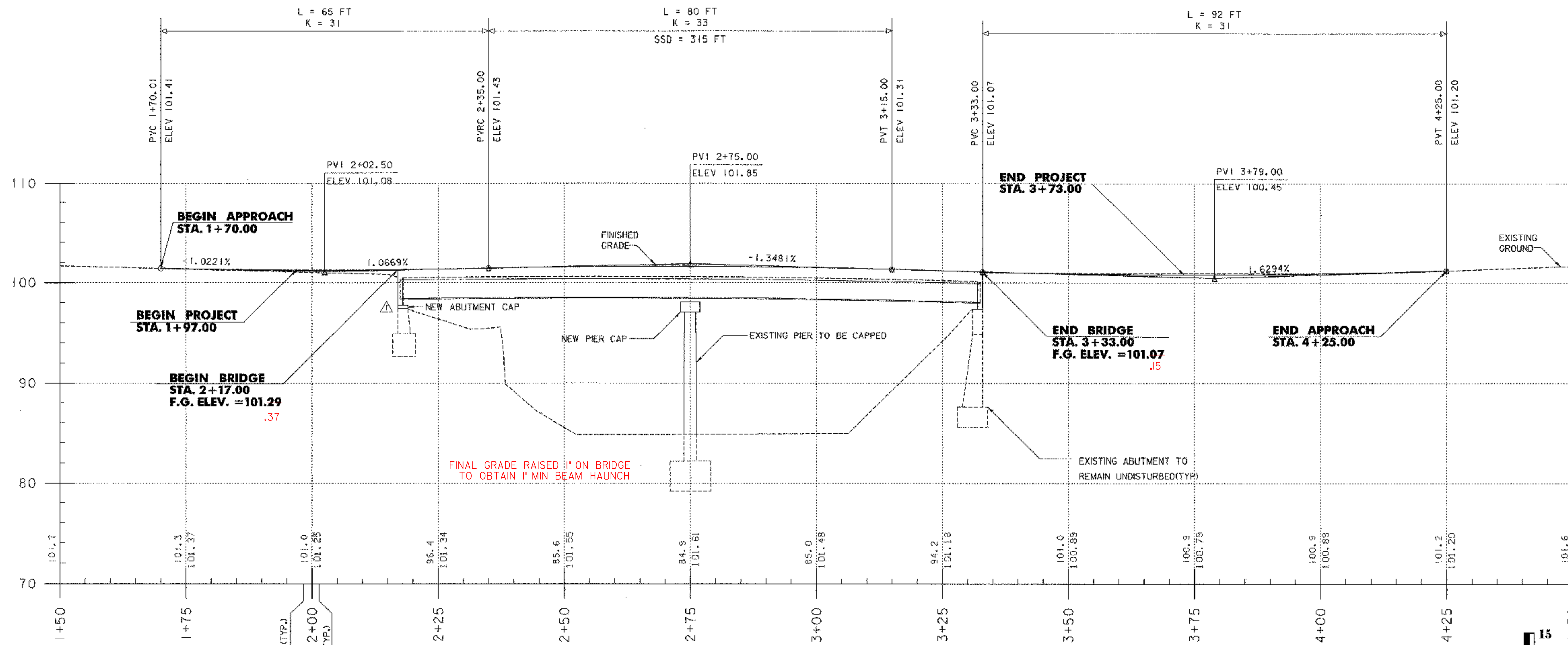
- THE EXISTING R.O.W. ON CHURCH STREET IS ASSUMED TO BE A 3 ROD (49.5 FEET), CENTERED ON THE EXISTING ROADWAY.
- THE EXISTING RAILROAD R.O.W IS ASSUMED TO BE 70 FEET, CENTERED ON THE TRACK.
- NEW STONE FILL TYPE II SHALL BE PLACED ON SIDE SLOPES AT LOCATIONS SHOWN. SEE DETAIL SHEET 22.
- THE UTILITY COMPANY SHALL BE CONTACTED IN ADVANCE OF EXCAVATING NEAR UTILITY POLE #28 TO ALLOW TEMPORARY STABILIZATION DURING THE PLACEMENT OF THE GUARDRAIL ANCHOR.

DATUM	
VERTICAL	LOCAL
HORIZONTAL	LOCAL

REVISIONS		
NO.	DESCRIPTION	BY & DATE

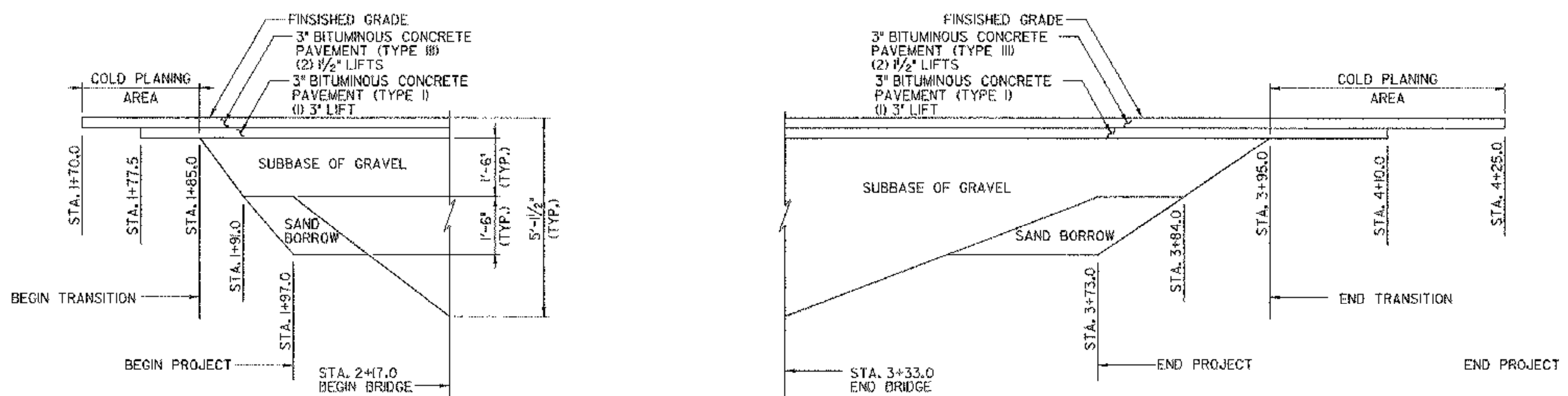
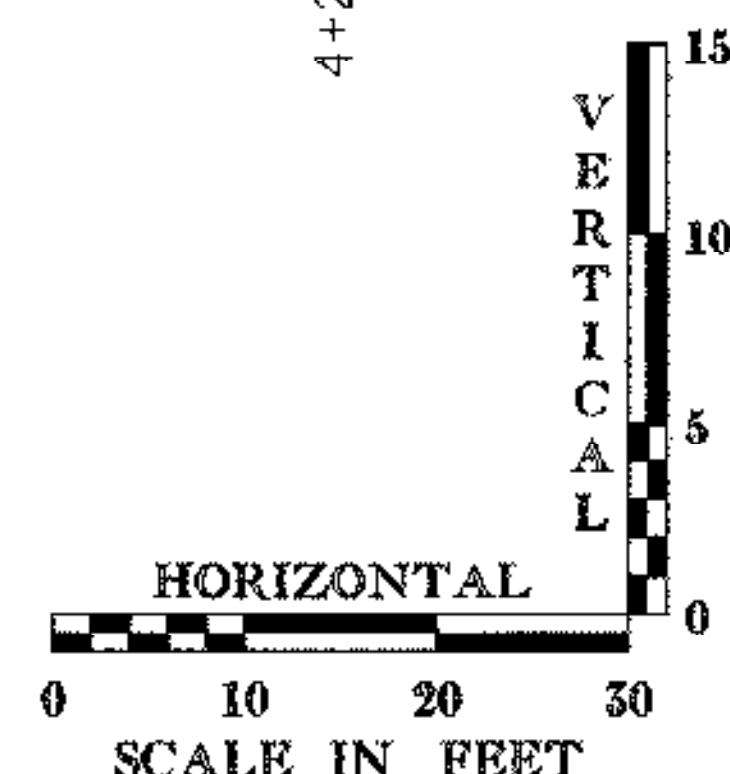
DuBois & King inc.	
engineering	planning
management	development

STATE OF VERMONT AGENCY OF TRANSPORTATION	
Town Of	CHESTER
Highway No.	TH 5
CHURCH STREET OVER WILLIAMS RIVER	
PLAN SHEET	
Designed By	B. C. AUSTIN
Checked By	J. TUCKER
Date	03/01
Drawn By	G. J. WOJCIECHOWSKI
Bridge Design Supervisor	J. TUCKER
Date	03/01
PROJECT	CHESTER
PROJECT NO.	BHO 1442 (32)
L.G.C. Info. ...ZJ365105.dgn	
D & K DWG NO.	11348
Sheet	5 of 22



PROFILE - CHURCH STREET

SCALE: HORIZ. 1" = 10'
VERT. 1" = 5'



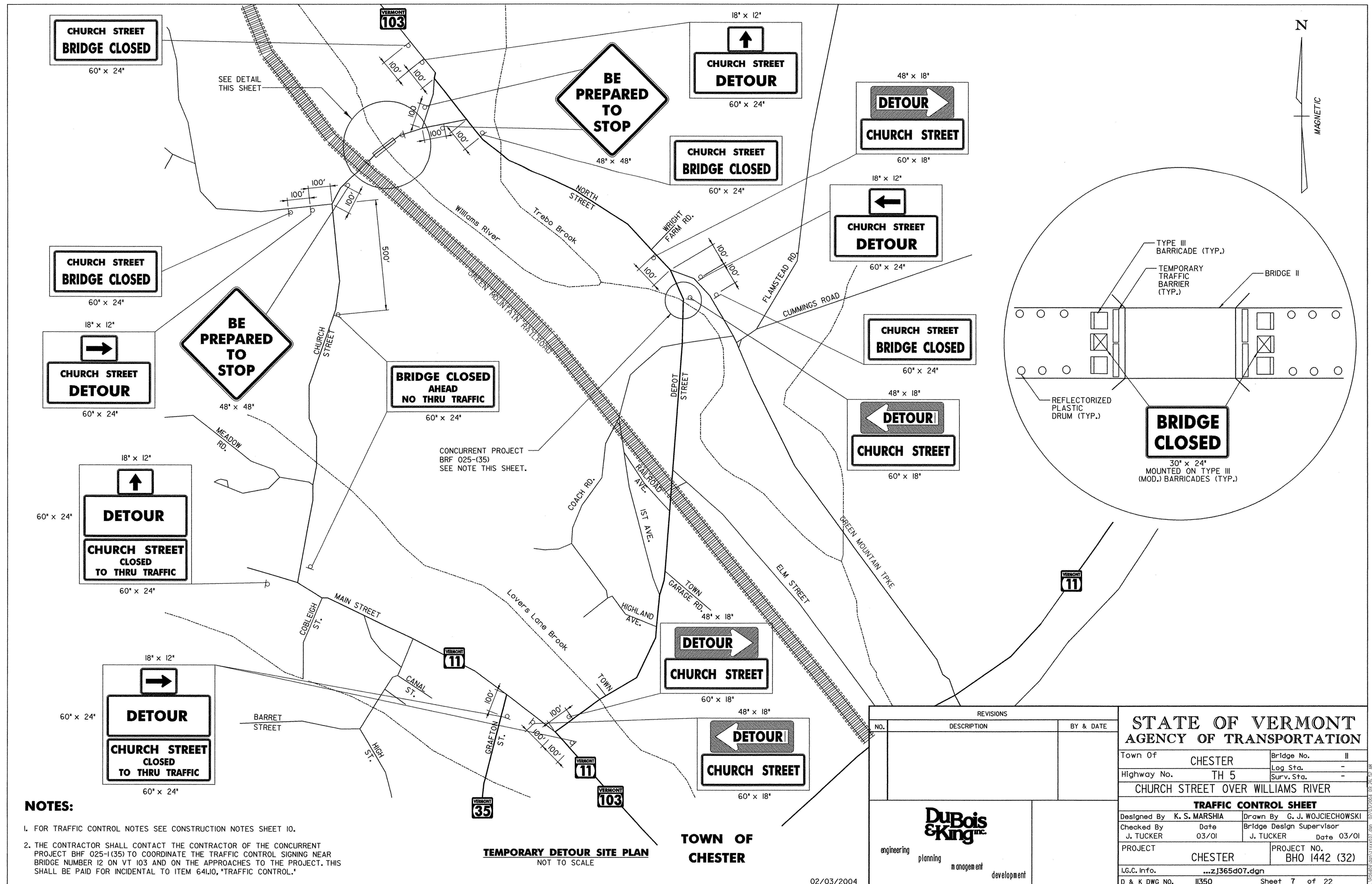
MATERIAL DEPTH TRANSITION DETAIL

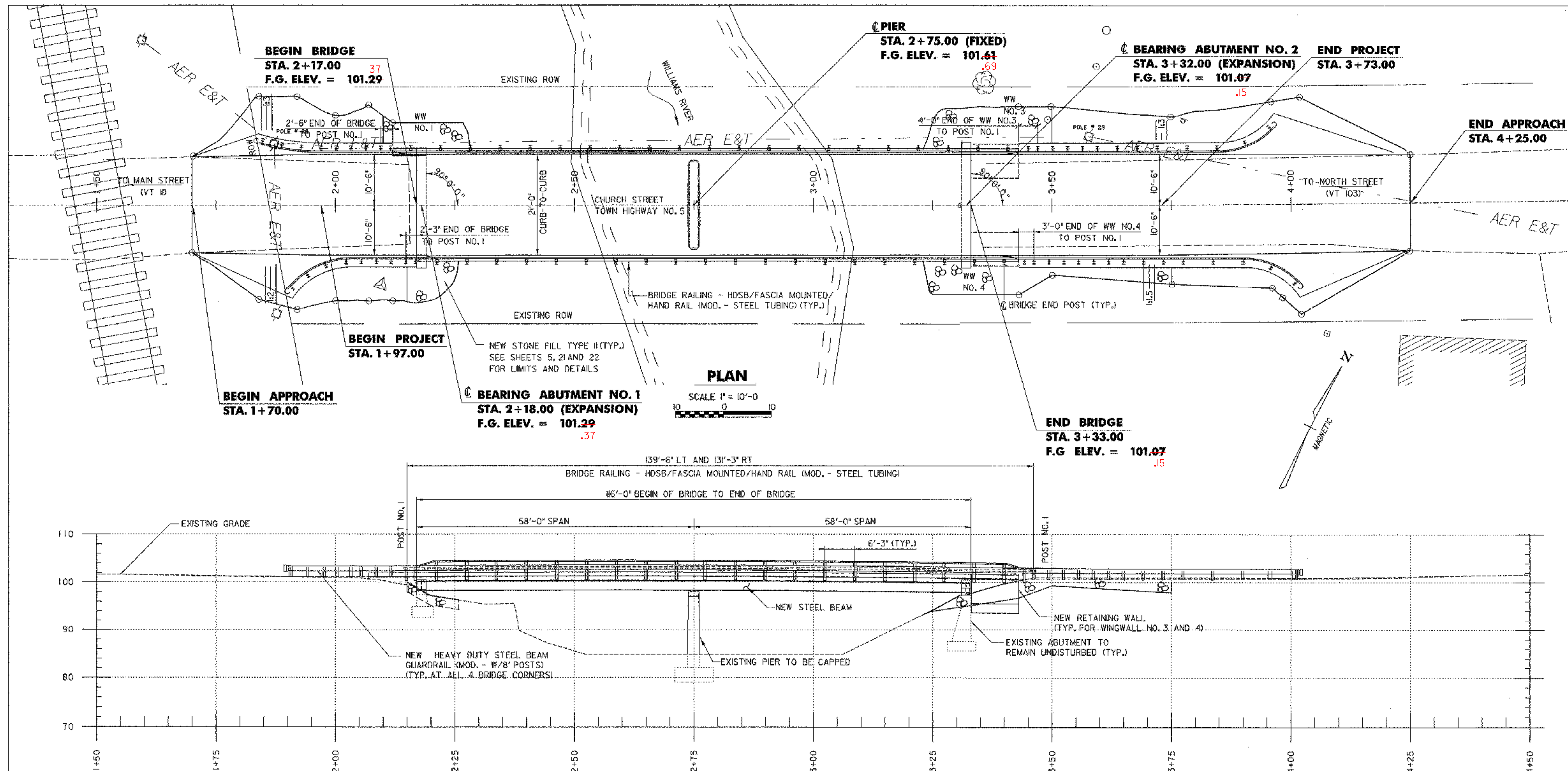
N.T.S.

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE	Town OF	Bridge No.
1	ADDED ABUTMENT CAP TO ABUTMENT NO. 11N PROFILE	APG 2/3/04	CHESTER	11
			Highway No.	TH 5
			CHURCH STREET OVER WILLIAMS RIVER	
			PROFILE	
			Designed By G. J. WOJCIECHOWSKI	Drawn By G. J. WOJCIECHOWSKI
			Checked By J. TUCKER	Bridge Design Supervisor J. TUCKER
			Date 03/01	Date 03/01
			PROJECT	CHESTER
			PROJECT NO.	BHO 1442 (32)
			I.G.C. Info.	...zj365pf6.dgn
			D & K DWG NO.	11349
			Sheet 6 of 22	

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engineering planning management development

02/10/2004





NOTES

1. THE EXISTING R.O.W. ON CHURCH STREET IS ASSUMED TO BE A 3 ROD (49.5 FEET), CENTERED ON THE EXISTING ROADWAY.
2. THE EXISTING RAILROAD R.O.W IS ASSUMED TO BE 10 FEET, CENTERED ON THE TRACK.

DATUM	
VERTICAL	LOCAL
HORIZONTAL	LOCAL

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
			Town Of	CHESTER
			Highway No.	TH 5
			CHURCH STREET OVER WILLIAMS RIVER	
			PLAN & ELEVATION	
			Designed By	B. C. AUSTIN
			Checked By	J. TUCKER
			Date	03/01
			Drawn By	G. J. WOJCIECHOWSKI
			Bridge Design Supervisor	J. TUCKER
			Date	03/01
			PROJECT	CHESTER
			PROJECT NO.	BHO 1442 (32)
			I.O.C. Info.	...zj365pe8.dgn
			D & K DWG NO.	11351
			Sheet 8 of 22	

02/03/2004

GENERAL NOTES:

- ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO STATE OF VERMONT AGENCY OF TRANSPORTATION'S STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, DATED 2001, AND ITS LATEST REVISIONS, AND THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 17TH EDITION, DATED 2002, AND ITS LATEST REVISIONS.
- DIMENSIONS, ANGLES, BEARINGS, AND ELEVATIONS OF THE EXISTING BRIDGE SHOWN ON THESE PLANS HAVE BEEN OBTAINED FROM PLANS OF THE EXISTING STRUCTURE AND LIMITED FIELD INVESTIGATION AND MAY NOT ACCURATELY REFLECT THE ACTUAL FIELD CONDITIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING FIELD MEASUREMENTS OF ALL EXISTING STRUCTURE COMPONENTS IMPACTED BY THE NEW WORK TO ASSURE CONSISTENCY WITH THE PROPOSED MODIFICATIONS. ANY DISCREPANCIES IN DIMENSIONS, CHARACTER OR EXTENT OF THE EXISTING FEATURES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE ADVANCING THE WORK.
- WORKING DRAWINGS REQUIRED FOR VARIOUS ITEMS OF WORK SHALL INDICATE THE ACTUAL FIELD MEASUREMENTS AND SHALL BE SO NOTED.
- ALL DIMENSIONS ARE HORIZONTAL OR VERTICAL, AND ARE GIVEN AT 68 DEGREES FAHRENHEIT, UNLESS NOTED OTHERWISE.
- SEE PIER DETAILS SHEET I9 FOR CONCRETE REPAIR NOTES.

CONSTRUCTION NOTES:

- THE CONTRACTOR SHALL USE CAUTION WHEN WORKING AROUND UNDERGROUND AND OVERHEAD UTILITIES.
- THE CONTRACTOR SHALL REVIEW AND UNDERSTAND ALL APPLICABLE ENVIRONMENTAL DOCUMENTS AND ENSURE THAT ALL CONSTRUCTION CONDITIONS ARE MET.
- A THOROUGH INSPECTION SHALL BE MADE OF SUBSTRUCTURE AREAS. AREAS OF CONCRETE FOUND TO BE SPALLED, DELAMINATED OR OTHERWISE UNSOUND SHALL BE REPAIRED. THE CONTRACTOR SHALL SUPPLY ANY MATERIALS REQUIRED FOR THE INSPECTION. ALL COSTS OF INSPECTION SHALL BE SUBSIDIARY TO ITEM 635.I0, MOBILIZATION.
- NO BACKFILL SHALL BE PLACED AGAINST ANY STRUCTURAL ELEMENTS UNTIL THE ENGINEER HAS APPROVED THIS WORK.
- THE CONTRACTOR SHALL SUBMIT A SITE SPECIFIC EROSION/SEDIMENT CONTROL PLAN WITH A SCHEDULE OF EVENTS. THIS WORK SHALL BE PAID FOR UNDER ITEM 652.I0, "EROSION AND SEDIMENT CONTROL PLAN"
- CONCRETE AND ALL OTHER CONSTRUCTION MATERIALS ARE PROHIBITED FROM BEING DISCHARGED INTO THE WATERS OF THE STATE.
- THE POST LENGTHS FOR RAILING TRANSITION POSTS AND POST #1 INCLUDED WITHIN THE BRIDGE RAIL PAY LIMITS, AND STEEL BEAM GUARDRAIL SHALL BE MODIFIED FROM 6 FEET TO 8 FEET. ALL COSTS SHALL BE INCLUDED IN ITEM 525.43, BRIDGE RAILING - HDSB/ FASCIA MOUNTED HAND RAIL (MOD.- STEEL TUBING) OR ITEM 621.21, HEAVY DUTY STEEL BEAM GUARDRAIL (MOD. - W/8" POSTS).

REMOVAL NOTES:

- ITEM 529.20, PARTIAL REMOVAL OF STRUCTURE SHALL INCLUDE THE REMOVAL OF:
 - EXISTING BRIDGE RAILING, CONCRETE CURBS AND POSTS, CONCRETE BRIDGE DECK AND CURTAIN WALLS.
 - STRUCTURAL STEEL BEAMS AND DIAPHRAGMS.
 - BEARING DEVICES
 - PORTIONS OF EXISTING WINGWALLS AND PIER AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER
 - ERECTION, MAINTENANCE, AND REMOVAL OF TEMPORARY STRUCTURES TO PREVENT DEBRIS FROM FALLING INTO THE WATERWAY
- REMOVAL OF EXISTING BRIDGE PAVEMENT SHALL BE PAID AS ITEM 529.I0, REMOVAL OF BRIDGE PAVEMENT.
- LIMITS OF WINGWALL AND PIER REMOVAL ARE APPROXIMATE. THE ENGINEER SHALL ESTABLISH ACTUAL LIMITS AFTER A COOPERATIVE INSPECTION BY THE CONTRACTOR AND THE ENGINEER. EXISTING ELEVATIONS SHALL BE FIELD VERIFIED TO ENSURE THAT THE REMOVAL LIMITS ARE ADEQUATE TO OBTAIN THE REQUIRED DIMENSIONS AND ELEVATIONS OF THE NEW CONSTRUCTION.
- SAWCUTS SHALL BE ONE-INCH DEEP ALONG ALL EXPOSED REMOVAL LINES WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE. ALL COSTS SHALL BE INCLUDED IN ITEM 529.20, PARTIAL REMOVAL OF STRUCTURE.
- EXISTING REINFORCING STEEL EXPOSED DURING REMOVAL OPERATIONS, WITHIN THE LIMITS OF THE NEW MASONRY, SHALL BE RETAINED AND INCORPORATED INTO THE NEW MASONRY. EXISTING REINFORCING STEEL TO BE RETAINED SHALL BE SAND BLASTED CLEAN OF ALL CONCRETE, DIRT, DETRIMENTAL SCALE, PAINT, OIL, AND OTHER FOREIGN SUBSTANCES. ALL COSTS SHALL BE INCLUDED IN ITEM 529.20, PARTIAL REMOVAL OF STRUCTURE.
- AFTER PIER CONCRETE REMOVAL, THE EXISTING REINFORCING STEEL TO BE RETAINED SHALL BE INSPECTED BY THE RESIDENT ENGINEER. DEPENDING UPON THE EXTENT OF CORROSION OR DAMAGE TO THE EXISTING REINFORCING STEEL, THE RESIDENT ENGINEER MAY ELECT TO INCREASE THE NUMBER OF DRILL AND CROUT BARS IF APPROXIMATELY 20 PERCENT OR MORE AREA LOSS HAS OCCURRED. COSTS TO INSPECT THE REINFORCING STEEL SHALL BE INCLUDED IN ITEM 529.20, PARTIAL REMOVAL OF STRUCTURE.
- ADDITIONAL DRILLING AND GROUTING, IF REQUIRED, SHALL BE PAID AT THE UNIT BID PRICE OF ITEM 507.J7, EPOXY COATED REINFORCING STEEL AND ITEM 507.I6, DRILLING AND GROUTING DOWELS.
- THE STRUCTURAL STEEL BEAMS AND DIAPHRAGMS SHALL REMAIN THE PROPERTY OF THE TOWN OF CHESTER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH THE TOWN OF CHESTER PRIOR TO THE REMOVAL OF THE STRUCTUAL STEEL FOR THE PROPER PROCEDURE IN SALVAGING THE BEAMS.

CONCRETE NOTES:

- CONCRETE PAYMENT AND CLASSIFICATION SHALL BE AS FOLLOWS:
ITEM 501.33, CONCRETE, HIGH PERFORMANCE CLASS A; DECK, CURTAIN WALLS AND CURB
ITEM 501.34, CONCRETE, HIGH PERFORMANCE CLASS B; WING WALLS AND ABUTMENT AND PIER CAPS
- ITEM 514.I0, WATER REPELLENT, SHALL BE APPLIED TO ALL EXPOSED CONCRETE ON BRIDGE SUPERSTRUCTURE EXCEPT THE BOTTOM OF THE DECK BETWEEN THE DRIP NOTCHES. WATER REPELLENT SHALL ALSO BE APPLIED TO THE EXPOSED CONCRETE ON THE NEW AND EXISTING SUBSTRUCTURES.

- ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 1 INCH BY 1 INCH, UNLESS OTHERWISE NOTED. A ONE-HALF INCH RADIUS SHALL BE USED ON THE TOP INSIDE CORNER OF THE CURBS.
- JOINTS AND SCORE MARKS IN CONCRETE SHALL BE CONSTRUCTED AS INDICATED ON THE PLANS OR AS DIRECTED BY THE ENGINEER.
- THE KEY IN CONCRETE CONSTRUCTION JOINTS SHALL BE MONOLITHIC AND CONTINUOUS FOR THE FULL LENGTH OF THE JOINT UNLESS OTHERWISE INDICATED. ANY UPWARD KEY SHALL BE PLACED INTEGRALLY WITH THE CONCRETE BELOW THE JOINT.
- SEE DECK DETAILS SHEET I1 FOR DECK POUR SEQUENCE NOTES.

BEARING NOTES:

- BEARINGS SHALL CONFORM TO APPLICABLE SUBSECTIONS OF SECTION 531 AND 731.
- PREFORMED BEARING PAD DESIGN CRITERIA:
 - ALLOWABLE BEARING PRESSURE ON CONCRETE = 1000 PSI
 - MINIMUM ALLOWABLE DESIGN ROTATION = 0.015 RADIAN
 - HORIZONTAL CAPACITY SHALL BE A MINIMUM OF 6% OF THE VERTICAL LOAD
 - DESIGN LOAD PER BEARING: ABUTMENTS = 90 KIPS & PIER = 150 KIPS
- SHOP DRAWINGS CONFORMING TO SUBSECTION 531.03 SHALL INCLUDE WELDING AND BONDING PROCEDURES.
- ALL STEEL FOR SOLE PLATES AND WASHERS SHALL BE AASHTO M 270/M270 , GRADE 36. BOLTS AND NUTS SHALL BE PER ASTM A 449. ALL HARDWARE SHALL BE GALVANIZED PER THE SPECIFICATION AASHTO M 111M/M 111.
- DRILL AND SET ANCHOR BOLTS WITH A MINIMUM OF 15 INCHES EMBEDDED INTO CONCRETE. HOLES FOR ANCHOR BOLTS SHALL BE 3 INCHES IN DIAMETER, IF ANCHOR BOLTS ARE TO BE GROUTED WITH MORTAR, TYPE IV AND 2-1/2 INCHES IN DIAMETER IF ANCHOR BOLTS ARE TO BE INSTALLED USING EPOXY MORTAR PER STANDARD SPECIFICATION 530. ALL COSTS SHALL BE INCLUDED IN ITEM 531.I0, BEARING DEVICE ASSEMBLY.
- ANCHOR BOLTS SHALL BE SWEDGED WITH 4 INCHES OF THREAD. EXPANSION BEARING NUTS ARE TO BE DRAWN UP FINGER TIGHT AND THEN BACKED OFF ONE-EIGHTH INCH. THREADS SHALL BE BURRED ABOVE NUT TO PREVENT NUT REMOVAL.
- BLANK
- EXISTING ANCHOR BOLTS THAT ARE NOT COVERED WITH A NEW BRIDGE SEAT CAP SHALL BE CUT ONE INCH MINIMUM BELOW EXISTING SUBSTRUCTURE SEATS, BLAST CLEANED AND FILLED WITH MORTAR TYPE IV. COSTS SHALL BE SUBSIDIARY TO ITEM 531.I0, "BEARING DEVICE ASSEMBLY."

STRUCTURAL STEEL NOTES:

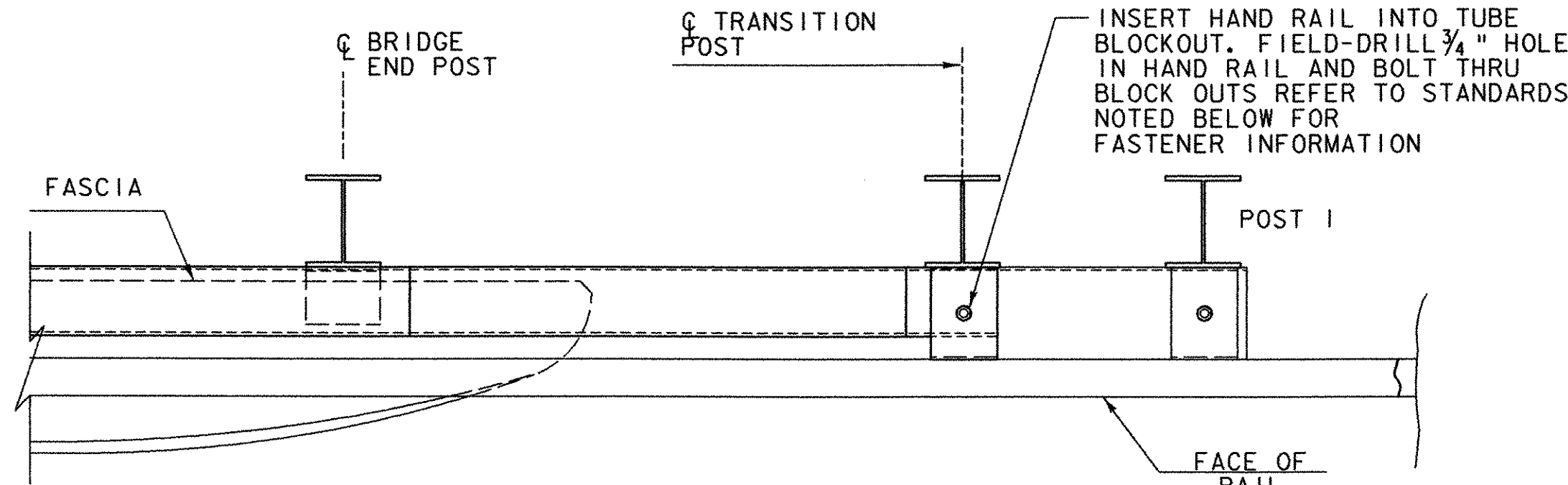
- ALL NEW STRUCTURAL STEEL SHALL BE AASHTO M270, GRADE 50W UNLESS OTHERWISE NOTED.
- BLANK
- ALL BOLTED FIELD CONNECTIONS SHALL BE MADE WITH 7/8-INCH DIAMETER HIGH STRENGTH BOLTS IN 15/16-INCH DIAMETER HOLES UNLESS OTHERWISE NOTED.
- CONNECTIONS NOT DESIGNED SHALL BE DETAILED BY THE FABRICATOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL.
- ALL WELDS SHALL HAVE CORROSION RESISTANCE AND WEATHERING APPEARANCE AS SPECIFIED FOR WEATHERING STEEL.
- SHEAR STUD CONNECTORS SHALL BE FIELD WELDED USING AUTOMATICALLY TIMED STUD WELDING EQUIPMENT AND SHALL BE PAID AS ITEM 508.I5, SHEAR CONNECTORS.
- AFTER THE SUPERSTRUCTURE STEEL HAS BEEN ERECTED, ELEVATIONS ALONG THE TOP OF THE BEAM SHALL BE TAKEN, AS DIRECTED BY THE ENGINEER, FOR USE IN DETERMINING THE FINAL GRADE AND HAUNCH DEPTHS.
- ANY HOLES IN FASCIA BEAM WEBS NOT OTHERWISE FILLED SHALL BE FILLED WITH BOLTS CONFORMING TO AASHTO M164 TYPE 3.
- THE ENDS OF THE BEAMS SHALL BE VERTICAL UNDER FULL DEAD LOAD DEFLECTION.

TRAFFIC CONTROL NOTES:

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON PROJECT CONSTRUCTION SIGNING, OFF PROJECT DETOUR SIGNING, AND BARRICADES. THE COSTS FOR REQUIRED ON PROJECT CONSTRUCTION SIGNING AND BARRICADES OTHER THAN TEMPORARY TRAFFIC BARRIERS SHALL BE INCIDENTAL TO OTHER ITEMS. THE COSTS FOR DETOUR SIGNING AND NECESSARY SIGN POSTS SHALL BE INCLUDED IN ITEM 641.I0, "TRAFFIC CONTROL." THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY CONCRETE TRAFFIC BARRIERS DURING THE CONSTRUCTION PERIOD TO PREVENT VEHICLES FROM DRIVING OFF THE BRIDGE OR APPROACHES. ALL COSTS SHALL BE INCLUDED IN ITEM 621.90, "TEMPORARY TRAFFIC BARRIER."
- ALL CONSTRUCTION AND DETOUR SIGNING AND SIGN POSTS SHALL REMAIN THE PROPERTY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL DRIVES AND SIDE ROADS DURING CONSTRUCTION. COSTS SHALL BE INCLUDED IN ITEM 641.I0, "TRAFFIC CONTROL."
- ALL DETOUR SIGNS SHALL BE ORANGE WITH BLACK LETTERING AND BLACK BORDERS.
- ALL DETAILS NOT SPECIFIED SHALL BE IN ACCORDANCE WITH THE MUTCD AND VAOT E SERIES STANDARDS.

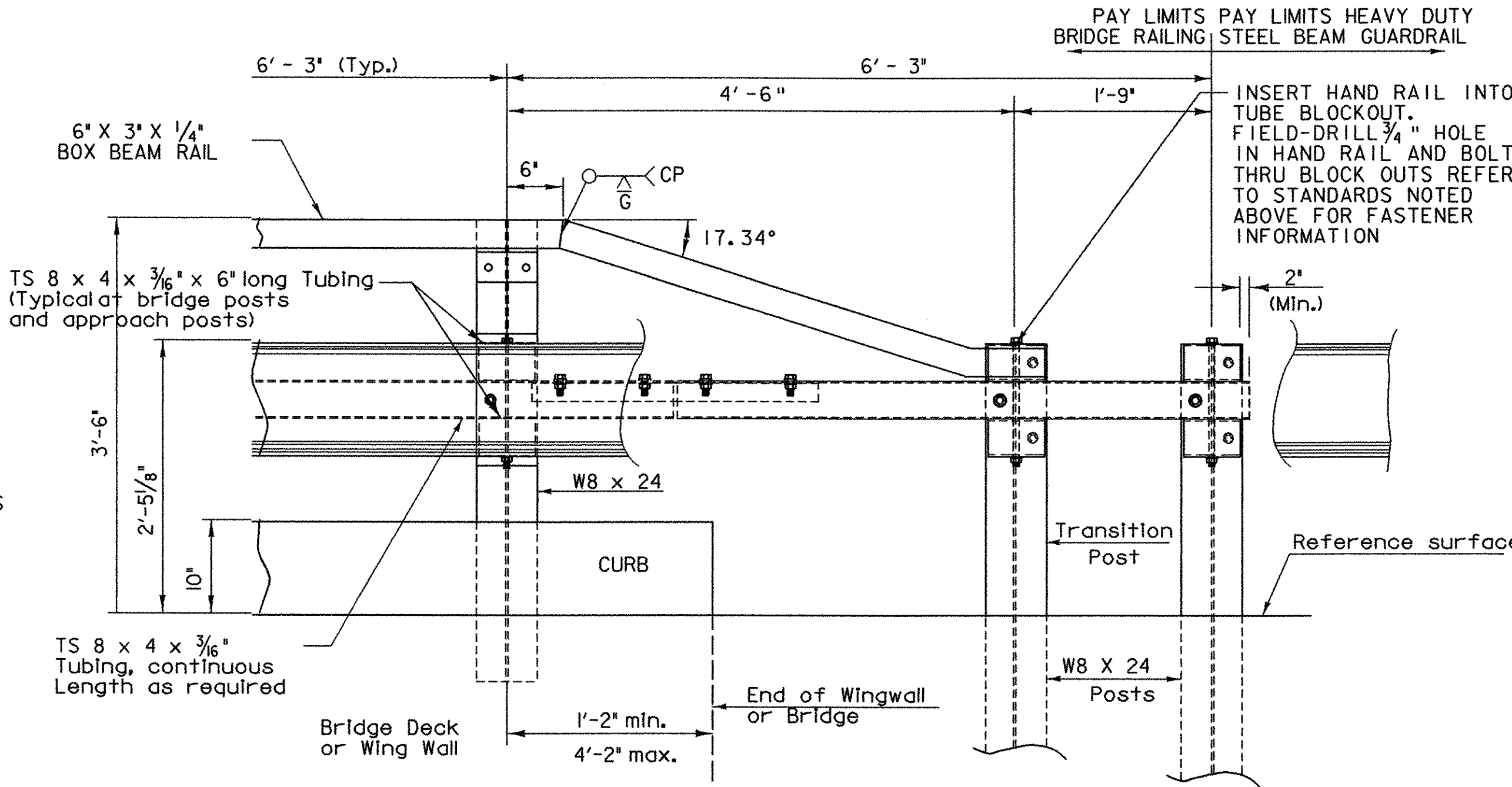
REINFORCING STEEL NOTES:

- REINFORCING STEEL IN THE DECK, CURTAIN WALL, AND BRUSH CURBS SHALL BE EPOXY COATED.
- EPOXY REINFORCING STEEL TO BE CUT IN THE FIELD SHALL BE SAW CUT AND THE EXPOSED ENDS TREATED. NO FLAME CUTTING SHALL BE PERMITTED.
- MINIMUM CLEAR COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:
ALONG BACK FACES OF WALLS AGAINST EARTH: 2.0 INCHES
ALONG TOP SURFACE OF DECK SLAB: 2.5 INCHES
ALONG BOTTOM SURFACE OF DECK SLAB: 1.5 INCHES
ELSEWHERE UNLESS OTHERWISE INDICATED: 3.0 INCHES
- REINFORCEMENT STEEL PLACEMENT TOLERANCES SHALL BE:
SPACING: +/- 1"
CLEARANCE: +/- 1/4".
- DRILLING AND GROUTING DOWELS SHALL BE PAID AS ITEM 507.I6, DRILLING AND GROUTING DOWELS. ALL DRILLED HOLES SHALL HAVE A MINIMUM OF 6-INCH CLEAR COVER.
- THE WELDED WIRE STEEL FABRIC FOR THE NEW CAP ON ABUTMENT NO. 1 SHALL BE PAID FOR INCIDENTAL TO ITEM 501.34, "CONCRETE, HIGH PERFORMANCE CLASS B."



RAIL PLAN

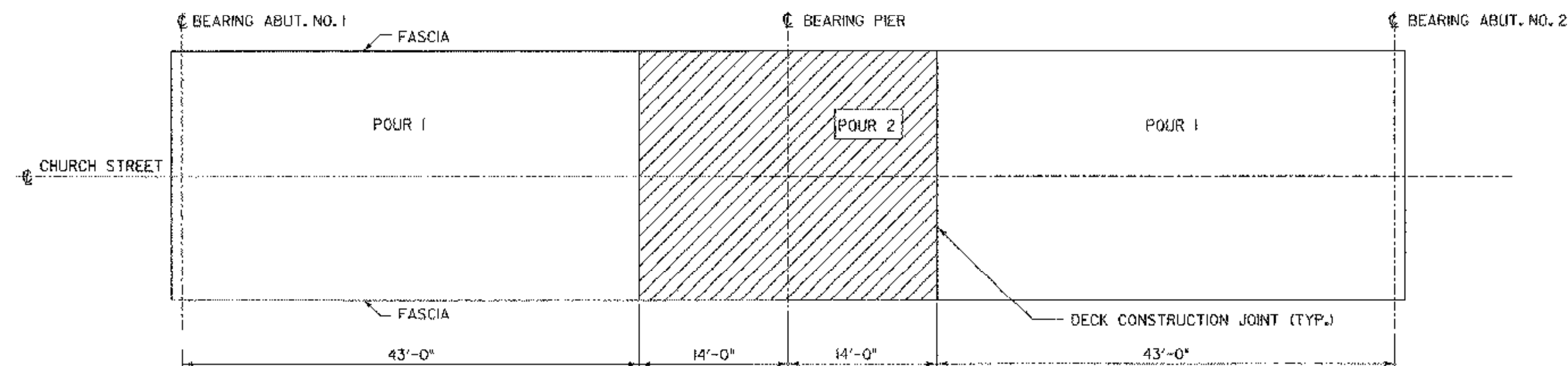
- NOTE:
- SEE VAOT STANDARD SHEET SB-R6-86 AND SB-R7-90 FOR ADDITIONAL DETAILS.
 - FIELD DRILLED HOLES IN HAND RAIL SHALL BE GALVANIZED PER ASTM A 760.



RAIL ELEVATION

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
	MODIFIED NOTES TO REFLECT REVISIONS TO PLANS	APG 2/3/04	Town Of CHESTER	
			Bridge No. 11	
			Log Sta. -	
			Highway No. TH 5	
			Surv. Sta. -	
			CHURCH STREET OVER WILLIAMS RIVER	
			CONSTRUCTION NOTES	
 engineering planning management development			Designed By B. C. AUSTIN	
			Drawn By G. J. WOJCIECHOWSKI	
			Checked By J. TUCKER	
			Date 03/01	
			Bridge Design Supervisor J. TUCKER	
			Date 03/01	
PROJECT CHESTER			PROJECT NO. BHO 1442 (32)	
I.G.C. Info. ...ZJ365gnl0.dgn				
D & K DWG NO. I1353			Sheet 10 of 22	

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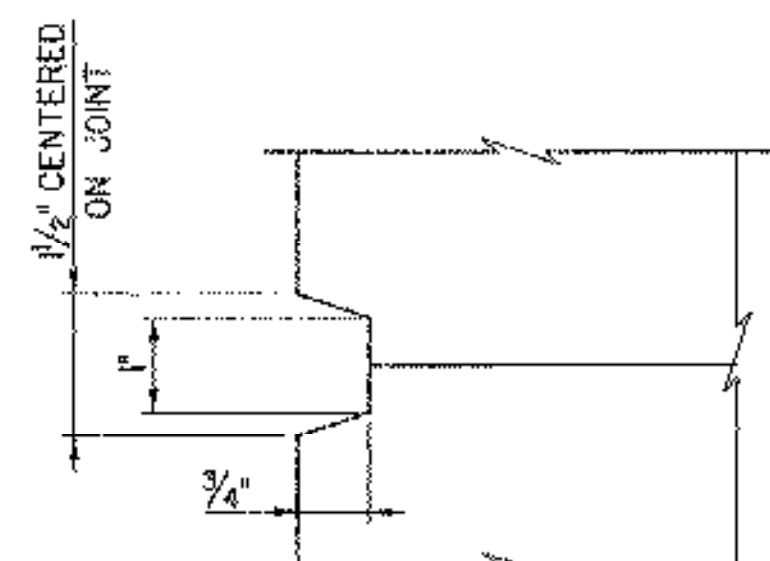


DECK POUR SEQUENCE NOTES:

1. EACH CONCRETE DECK POUR SHALL BE PLACED CONTINUOUSLY WITHIN ONE EIGHT HOUR WORKING DAY. THERE SHALL BE A MINIMUM DELAY PERIOD OF 96 HOURS AFTER COMPLETION OF EACH POUR BEFORE BEGINNING ANOTHER POUR. INDIVIDUAL POUR NUMBERS AS SHOWN MAY BE COMBINED INTO A SINGLE POUR IF APPROVED BY THE ENGINEER.

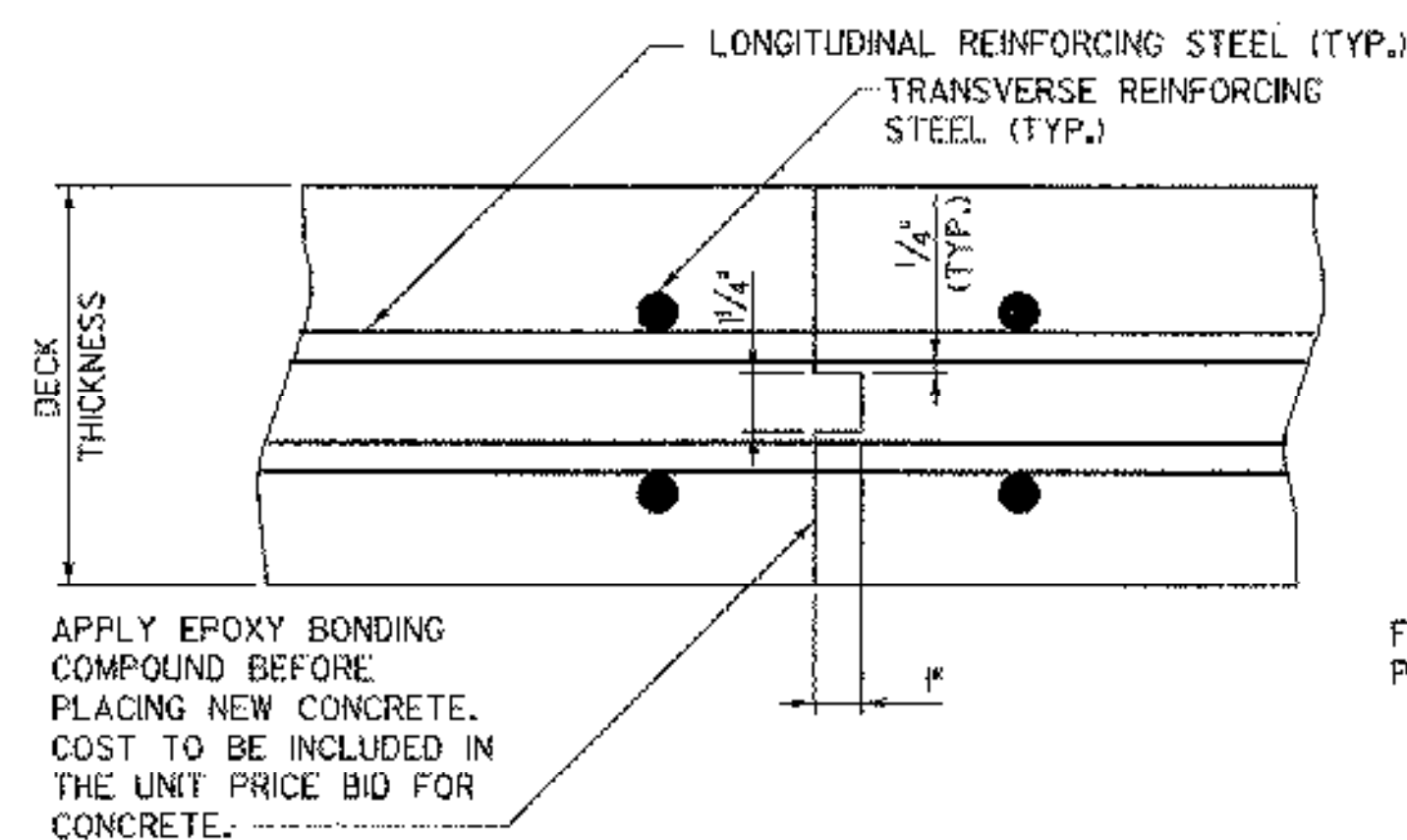
DECK POUR SEQUENCE

NTS



SCORE MARK DETAIL

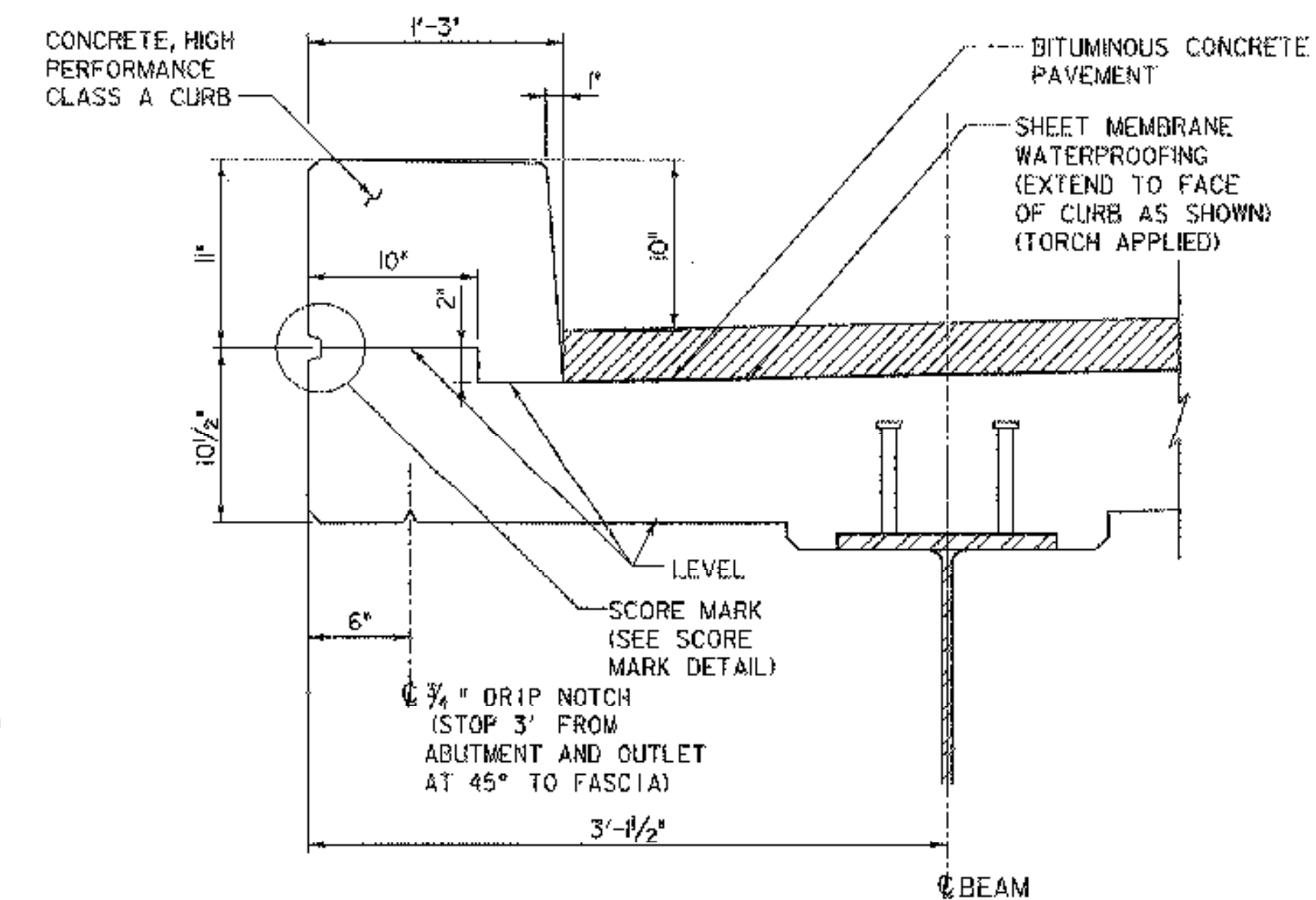
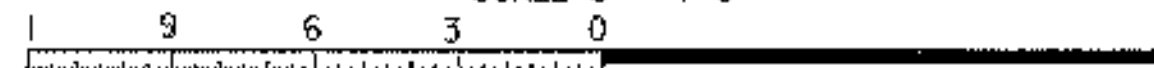
NOT TO SCALE



APPLY EPOXY BONDING COMPOUND BEFORE PLACING NEW CONCRETE. COST TO BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE.

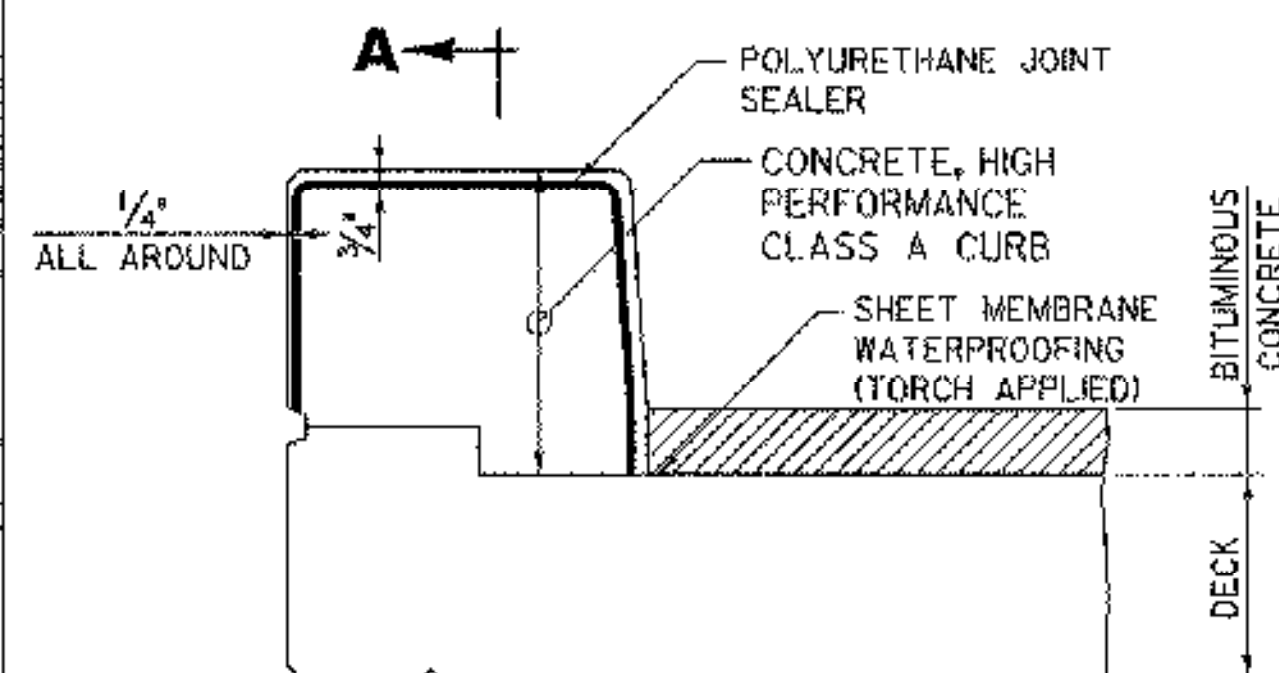
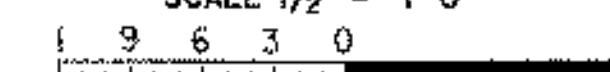
TRANSVERSE DECK CONSTRUCTION JOINT DETAIL

SCALE 3" = 1'-0"

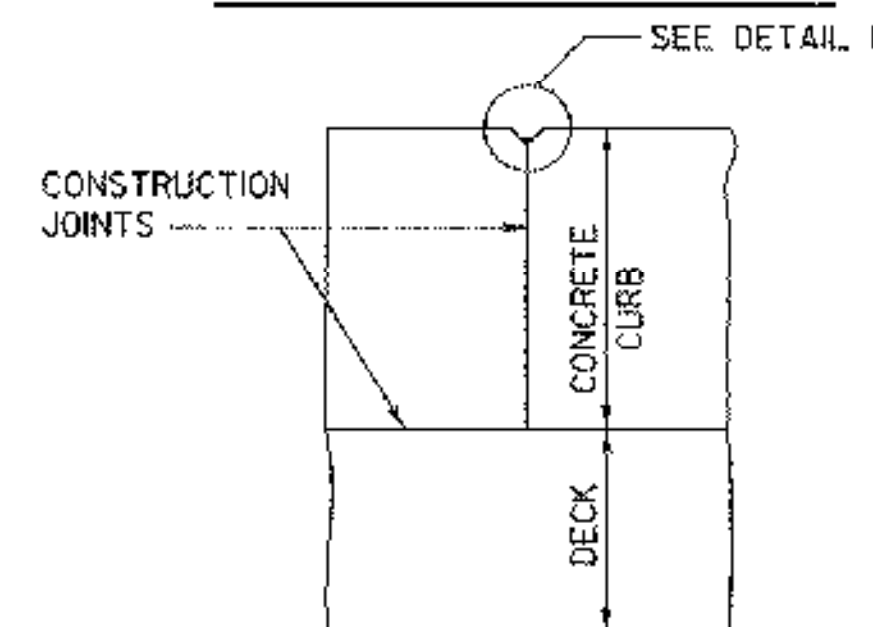


TYPICAL CURB DETAIL

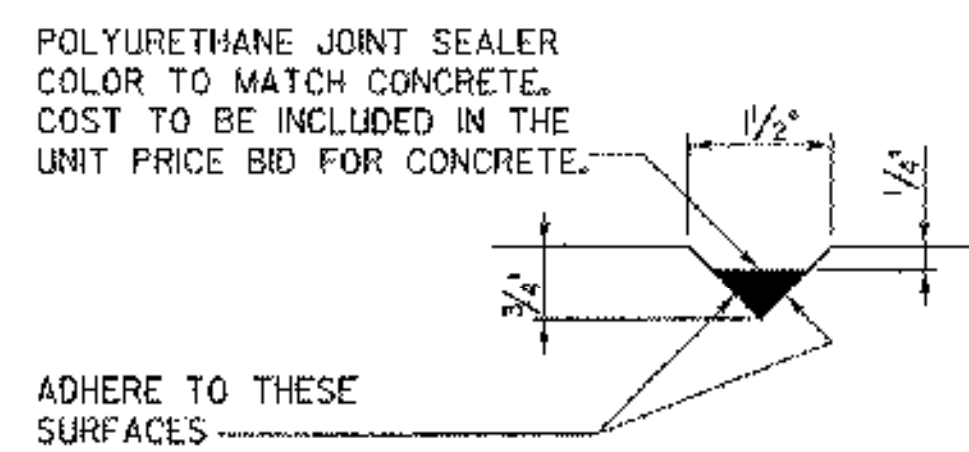
SCALE 1/2" = 1'-0"



TYPICAL SECTION



SECTION A-A



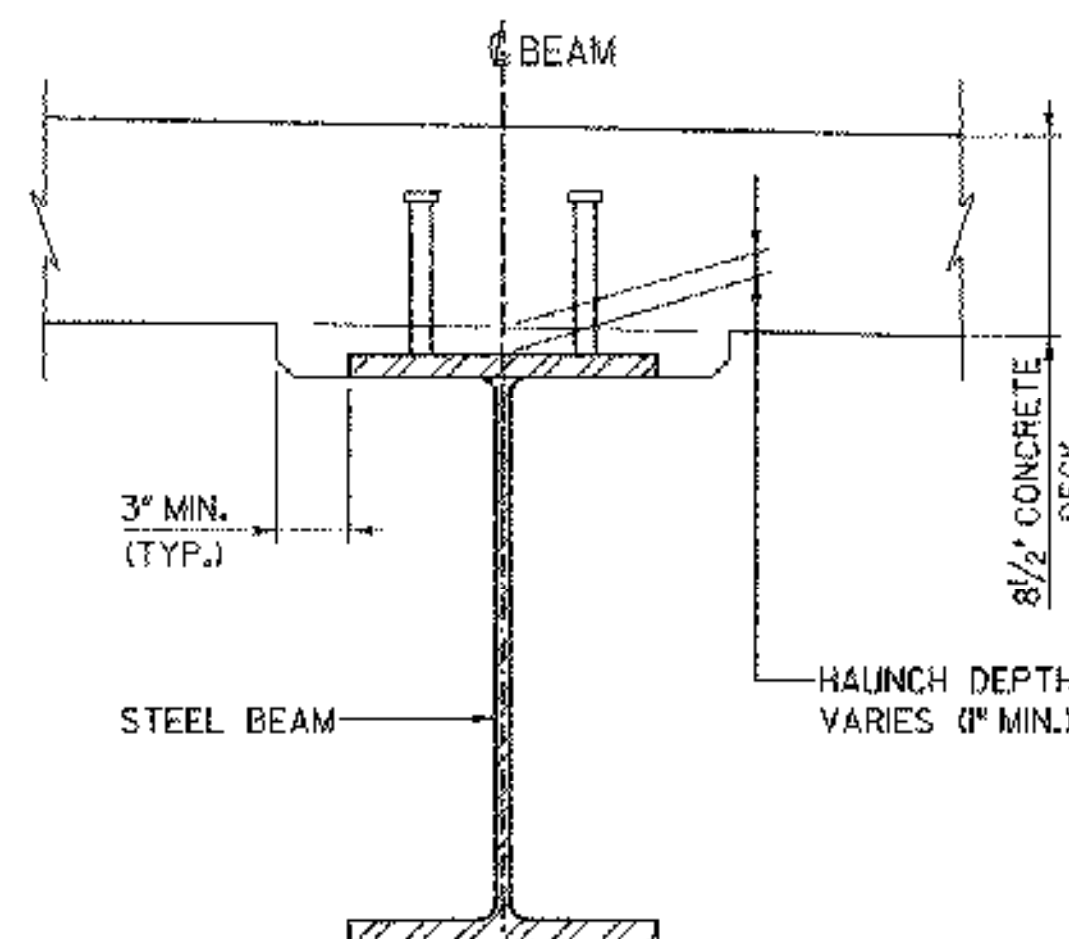
DETAIL 1

CONCRETE CURB CONSTRUCTION JOINT

NOT TO SCALE

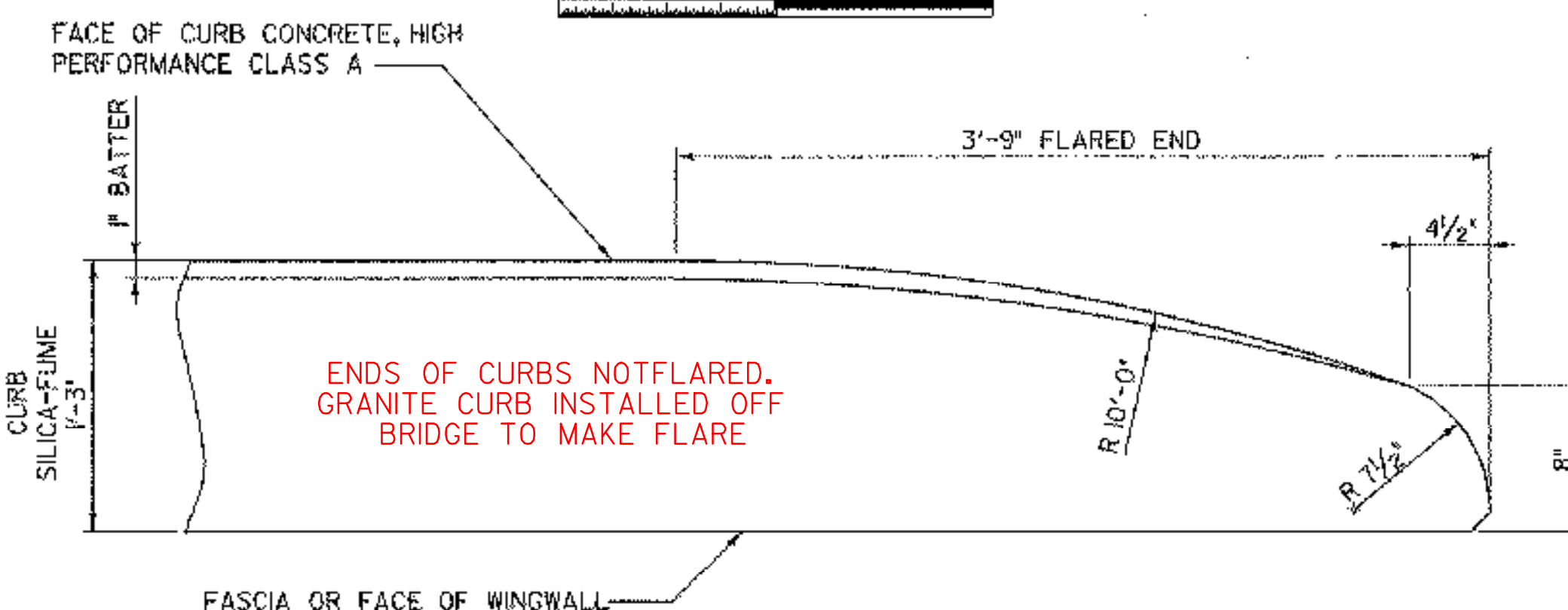
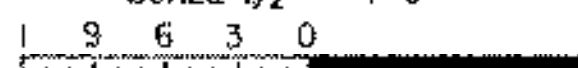
CURB CONSTRUCTION JOINT NOTES:

1. POLYURETHANE MEMBRANE AND BLAST CLEANING SHALL BE INCLUDED IN THE UNIT PRICE BID FOR SHEET MEMBRANE WATERPROOFING.
2. JOINTS SHALL BE A SPACED MAXIMUM OF 15'-0" CENTER TO CENTER AND SHALL BE 1'-6" MINIMUM FROM THE CENTER OF THE NEAREST BRIDGE RAIL POST. CONCRETE SHALL BE PLACED IN ALTERNATING SECTIONS WITH A MINIMUM OF 48 HOURS DELAY BETWEEN ADJACENT POURS.
3. LONGITUDINAL REINFORCING SHALL PASS THROUGH CONCRETE CURB CONSTRUCTION JOINTS.



HAUNCH DETAIL

SCALE 1/2" = 1'-0"



FLARED END DETAIL FOR 1'-3" CURB

CURB REINFORCING STIRRUP BARS SHALL BE TURNED AS REQUIRED TO FIT FLARED ENDS.

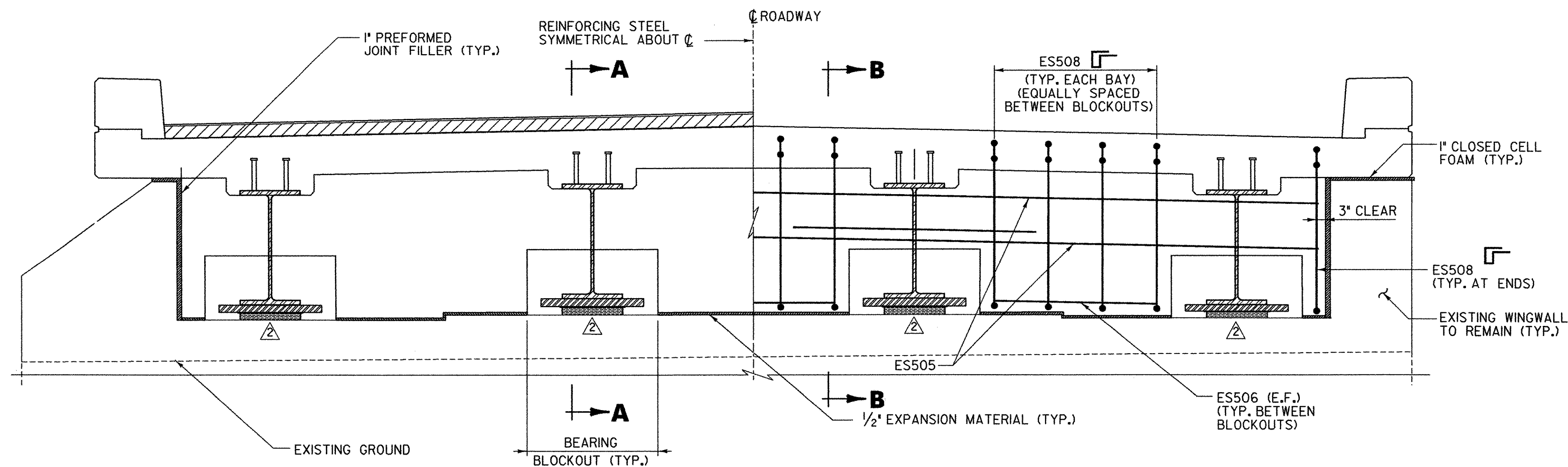
NOTES:

1. SEE STANDARD SB-R6-82, SB-R7-90 AND SHEET 10 FOR BRIDGE RAILING DETAILS.
2. BLAST CLEAN 3 FEET FROM FACE OF CURB AND 3 INCHES UP CURB FACE PRIOR TO PLACING MEMBRANE.

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
			Town Of	CHESTER
			Bridge No.	11
			Highway No.	TH 5
			Log Sta.	-
			Surv. Sta.	-
			CHURCH STREET OVER WILLIAMS RIVER	
			DECK DETAILS	
			Designed By	B. C. AUSTIN
			Drawn By	G. J. WOJCIECHOWSKI
			Checked By	J. TUCKER
			Date	03/01
			Bridge Design Supervisor	J. TUCKER
			Date	03/01
			PROJECT	CHESTER
			PROJECT NO.	BHO 1442 (32)
			I.G.C. Info.	...zj365dl.dgn
			D & K DWG NO.	11354
			Sheet	11 of 22

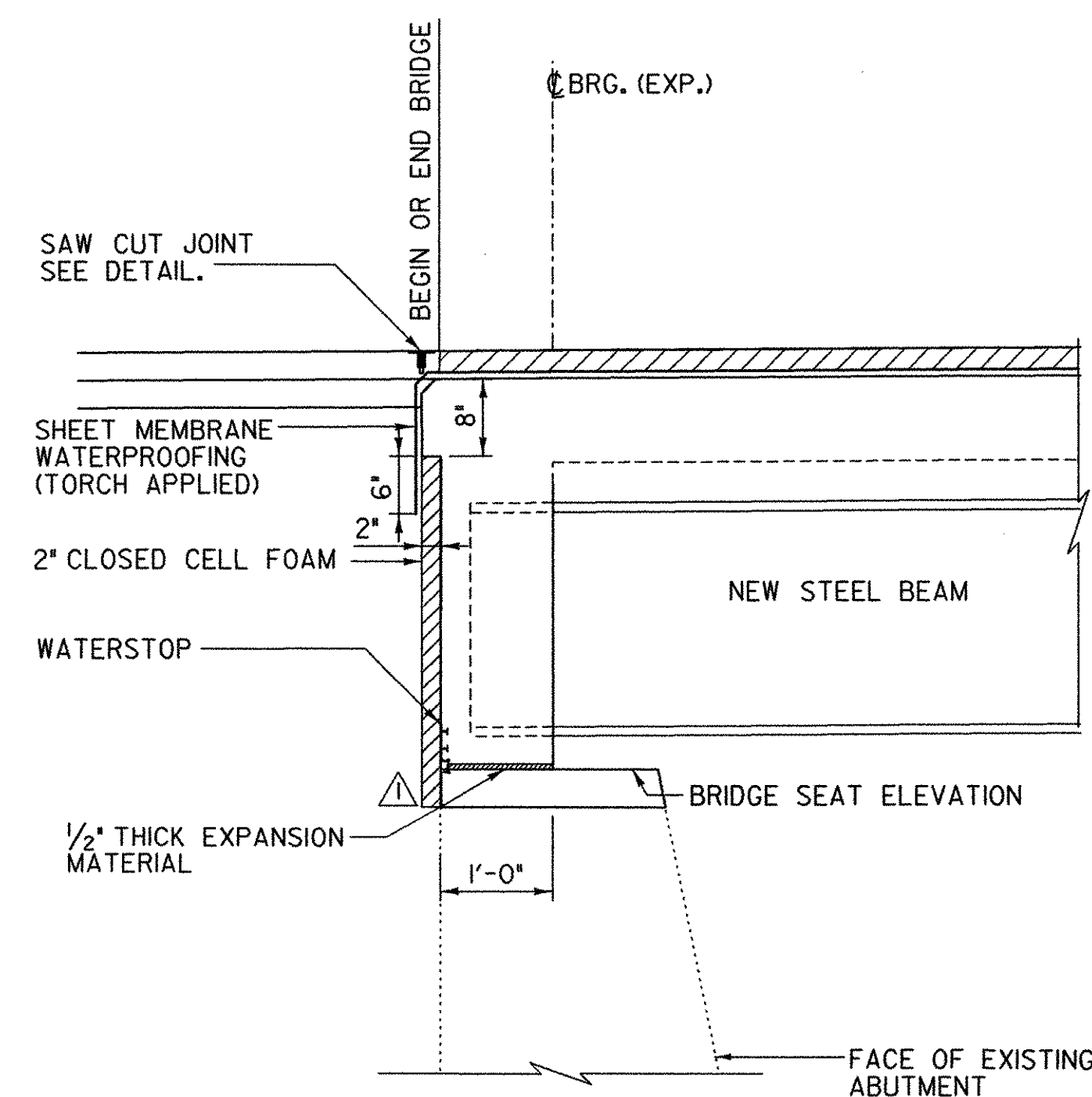
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02/03/2004



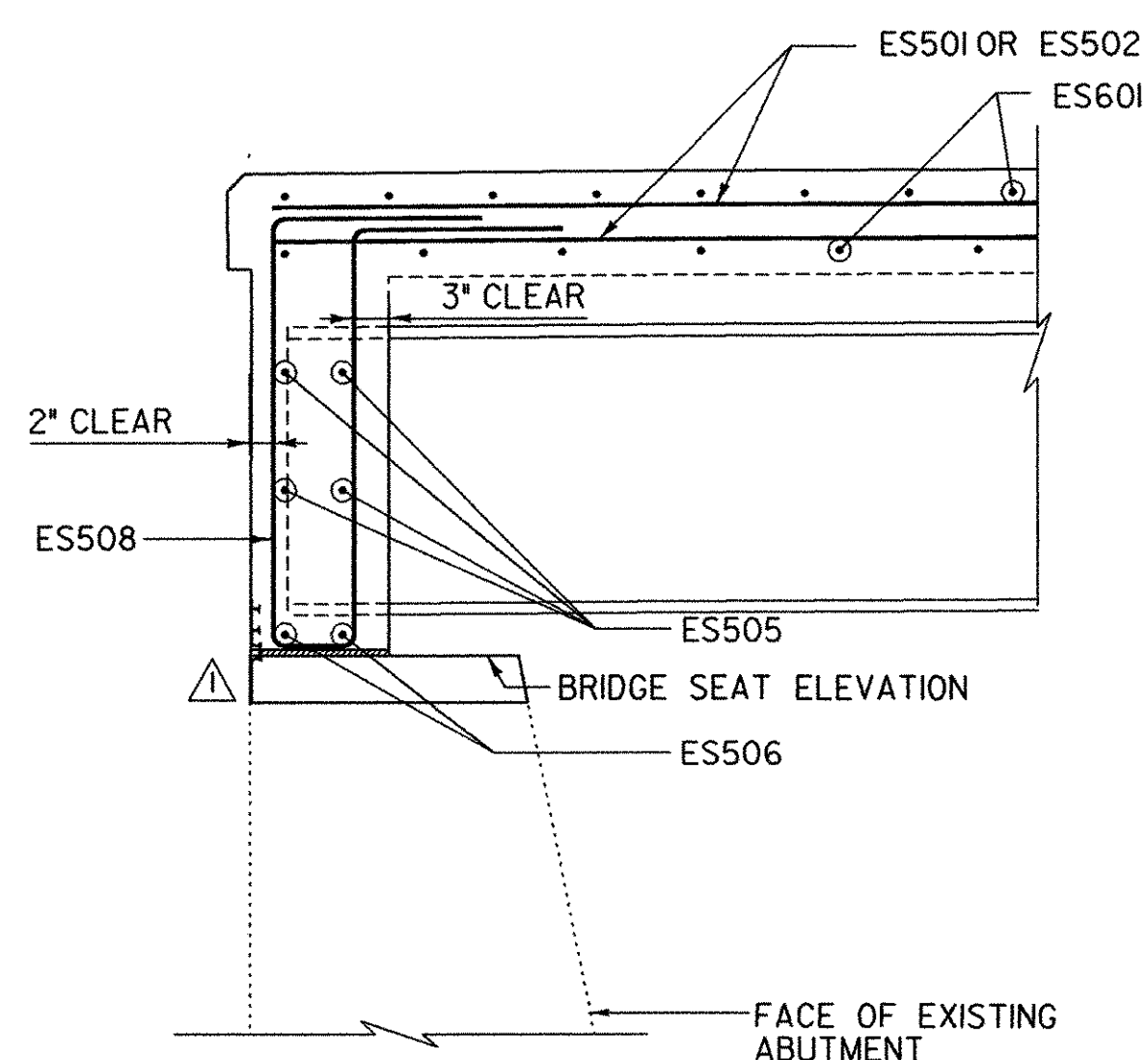
CURTAIN WALL ELEVATION

SCALE $\frac{3}{4}" = 1'-0"$



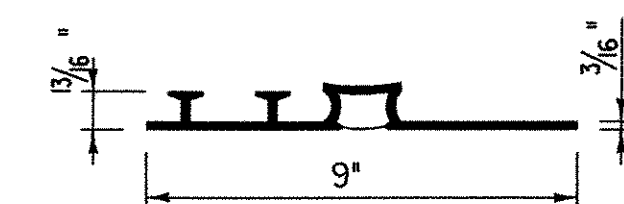
SECTION A-A

SCALE $\frac{3}{4}" = 1'-0"$



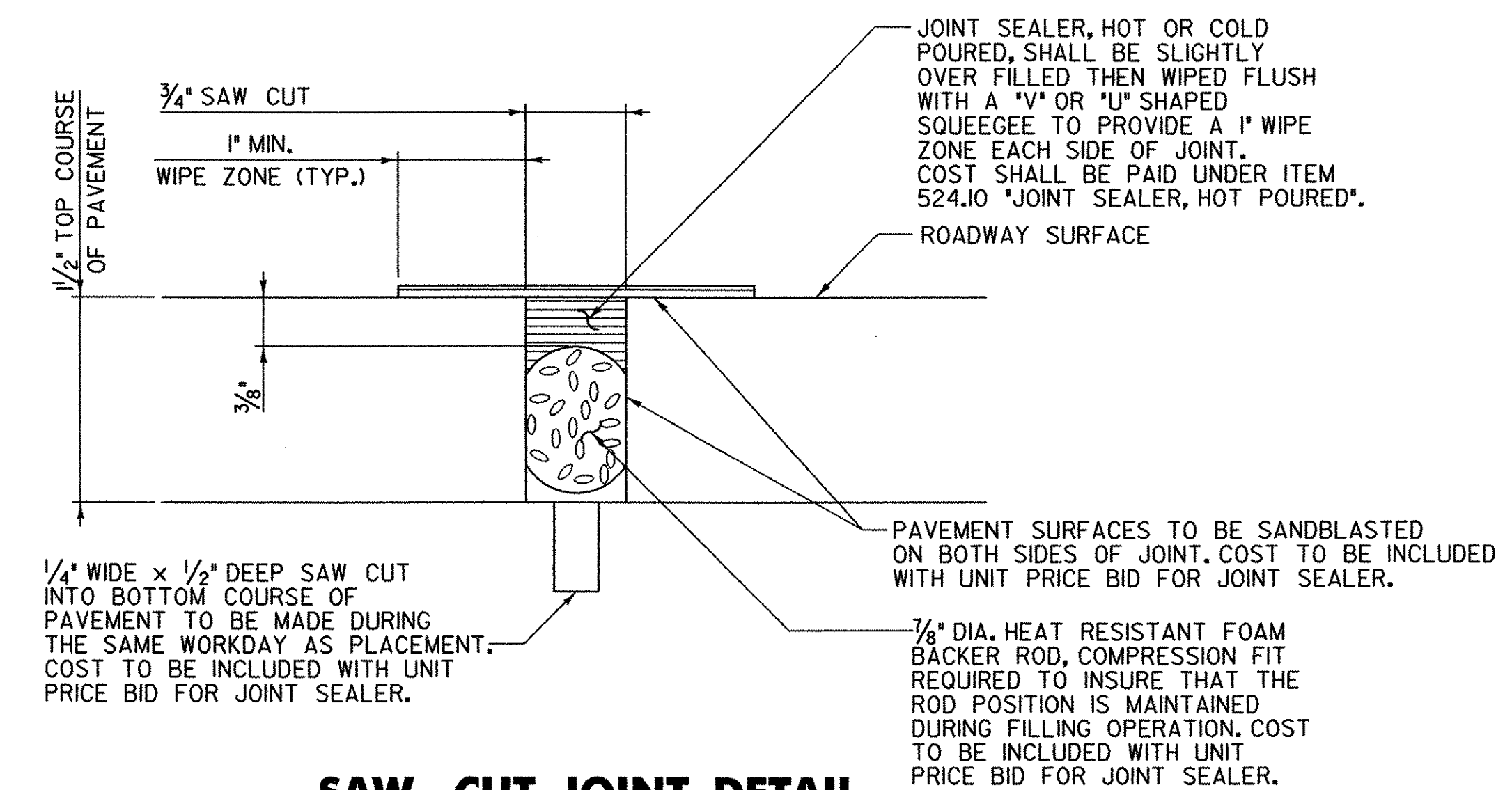
SECTION B-B

SCALE $\frac{3}{4}" = 1'-0"$



P. V. C. WATERSTOP FOR EXPANSION END

NOTE: FOR INSTALLATION, REMOVE BOTTOM HOOKS. THE COSTS FOR P. V. C. WATERSTOP SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE.



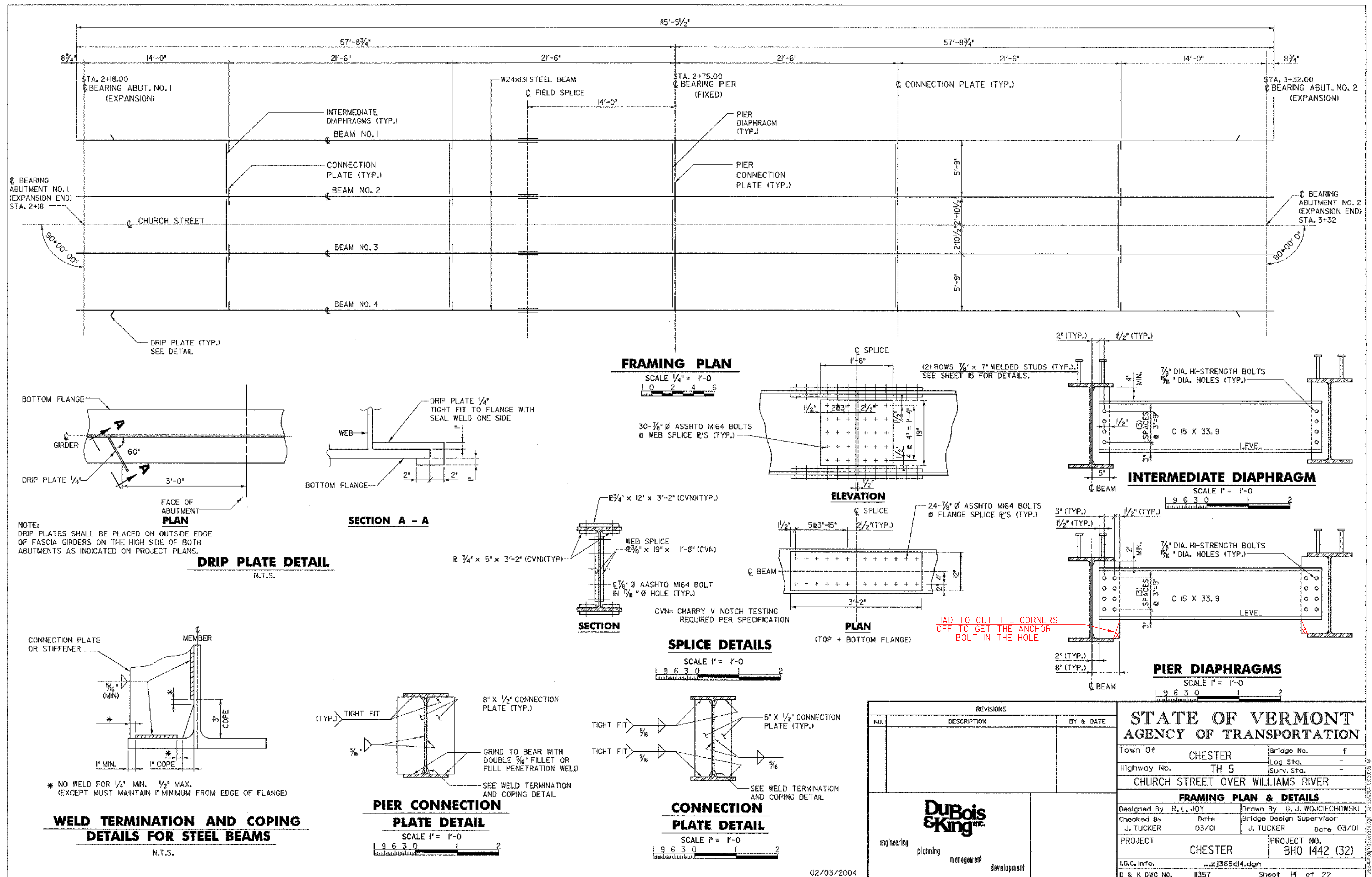
SAW CUT JOINT DETAIL

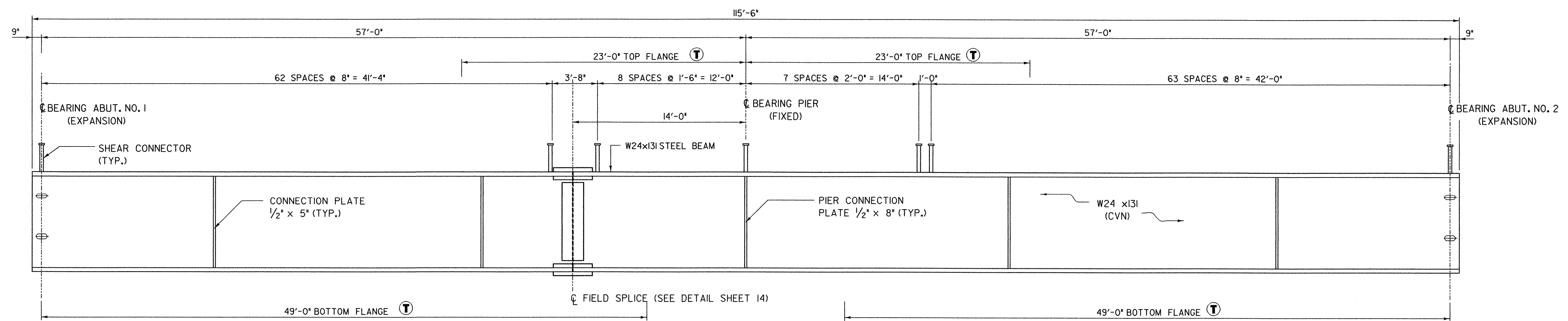
N.T.S.

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
1	ADDED ABUTMENT CAP TO ABUTMENT SECTIONS	APG 2/3/04	Town Of CHESTER Bridge No. 11	
2	REMOVED SHIM PLATE FROM BEARING ASSEMBLY IN CURTAIN WALL ELEVATION	APG 2/3/04		
			Highway No. TH 5	Log Sta. -
			Surv. Sta. -	
			CHURCH STREET OVER WILLIAMS RIVER	
			CURTAIN WALL DETAILS	
			Designed By G. J. WOJCIECHOWSKI	Drawn By G. J. WOJCIECHOWSKI
			Checked By J. TUCKER Date 03/01	Bridge Design Supervisor J. TUCKER Date 03/01
			PROJECT CHESTER	PROJECT NO. BHO 1442 (32)
			I.G.C. Info. ...zj365dl3.dgn	
			D & K DWG NO. 11356	Sheet 13 of 22

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engineering planning management development

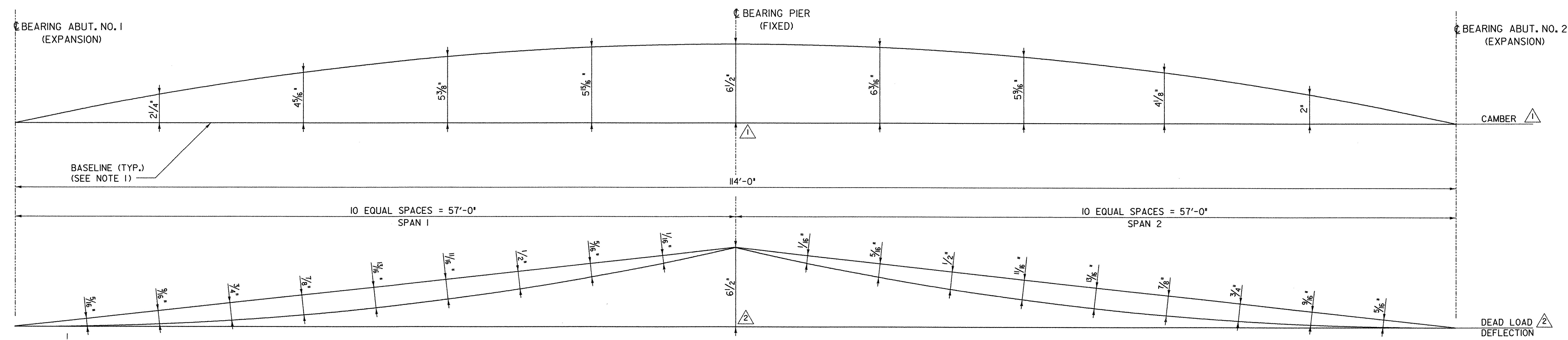




Ⓜ INDICATES RANGE OF TENSION ZONE
CVN = CHARPY V NOTCH TESTING REQUIRED PER SPECIFICATION

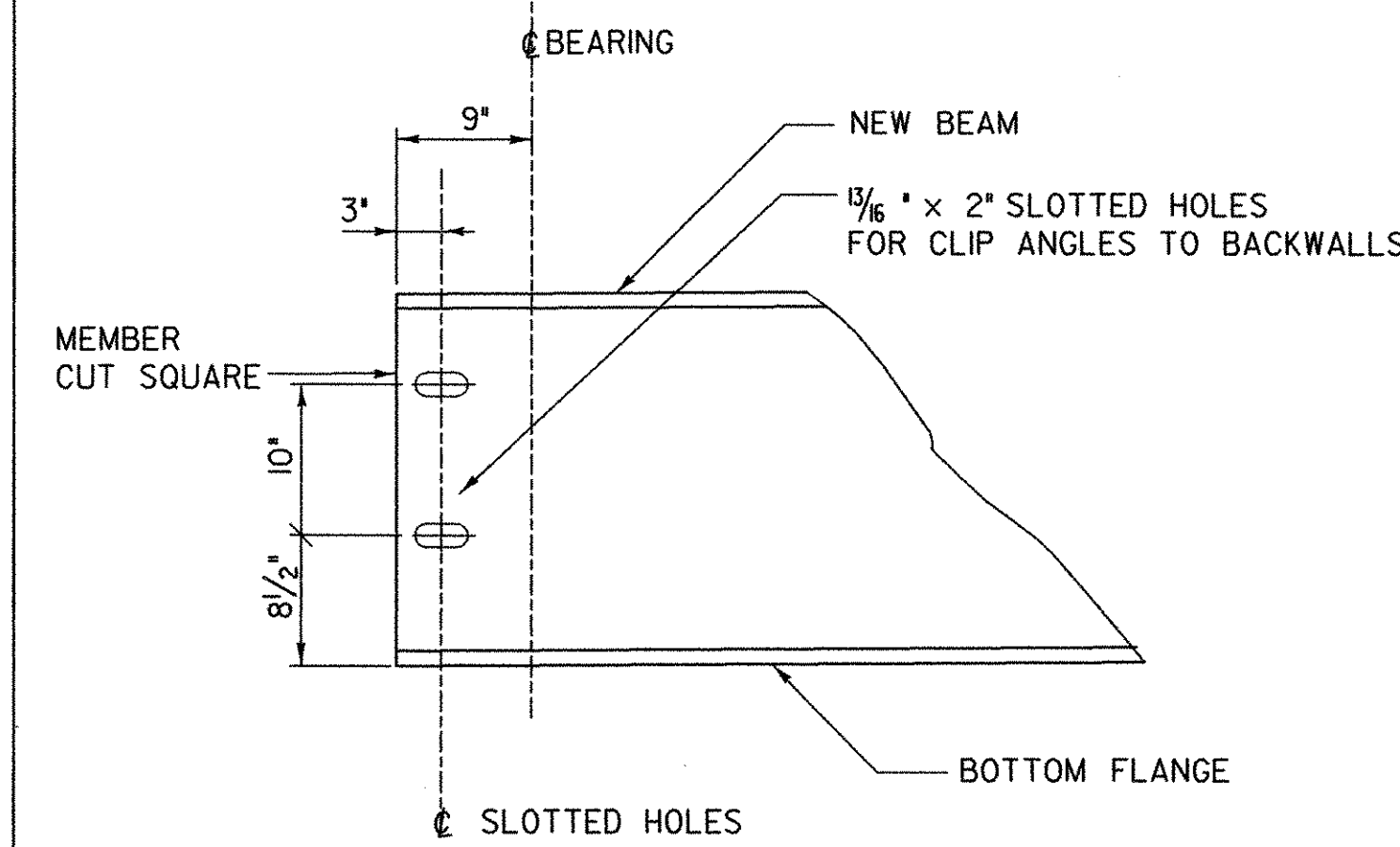
BEAM ELEVATION

N.T.S.

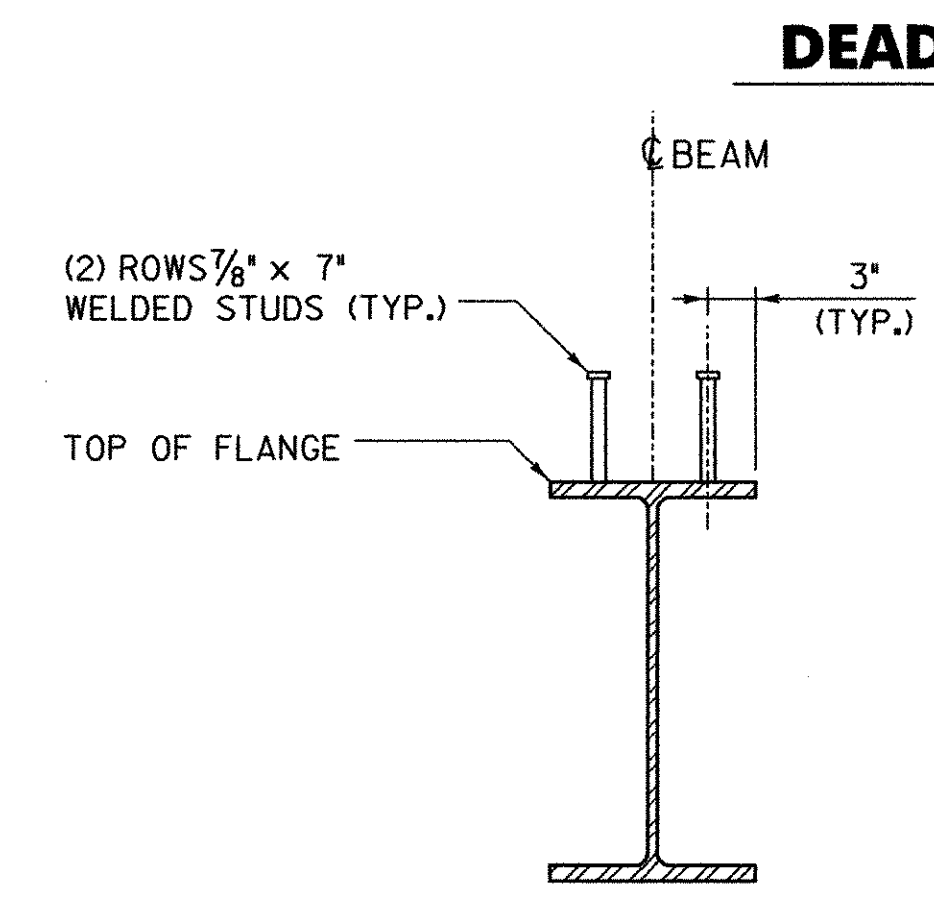


DEAD LOAD DEFLECTION AND CAMBER DIAGRAM

N.T.S.



ELEVATION VIEW END OF STEEL BEAM AT ABUTMENT
SCALE 1" = 1'-0"
1 9 6 3 0 2

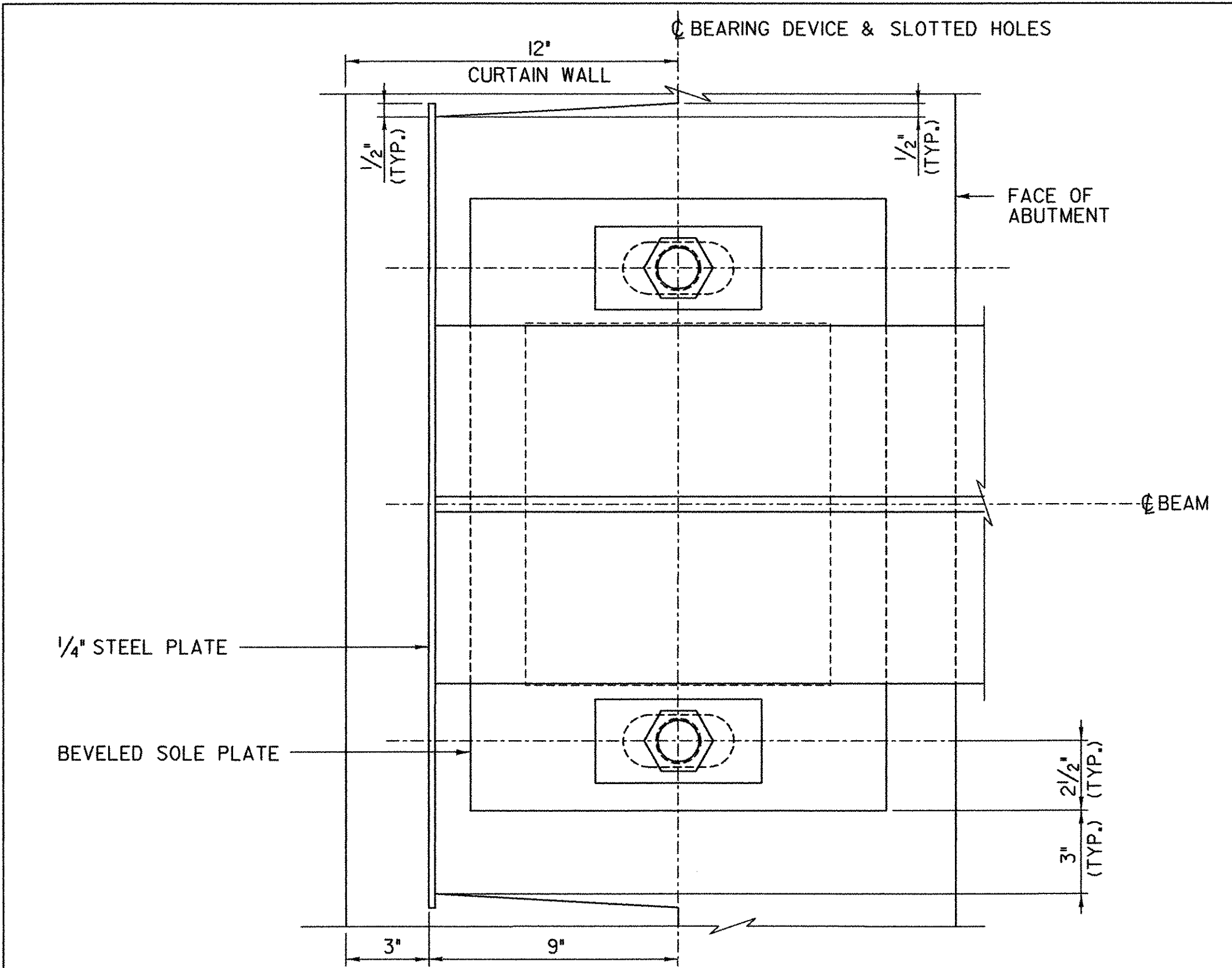


SHEAR CONNECTOR DETAIL
SCALE 1" = 1'-0"
1 9 6 3 0 2

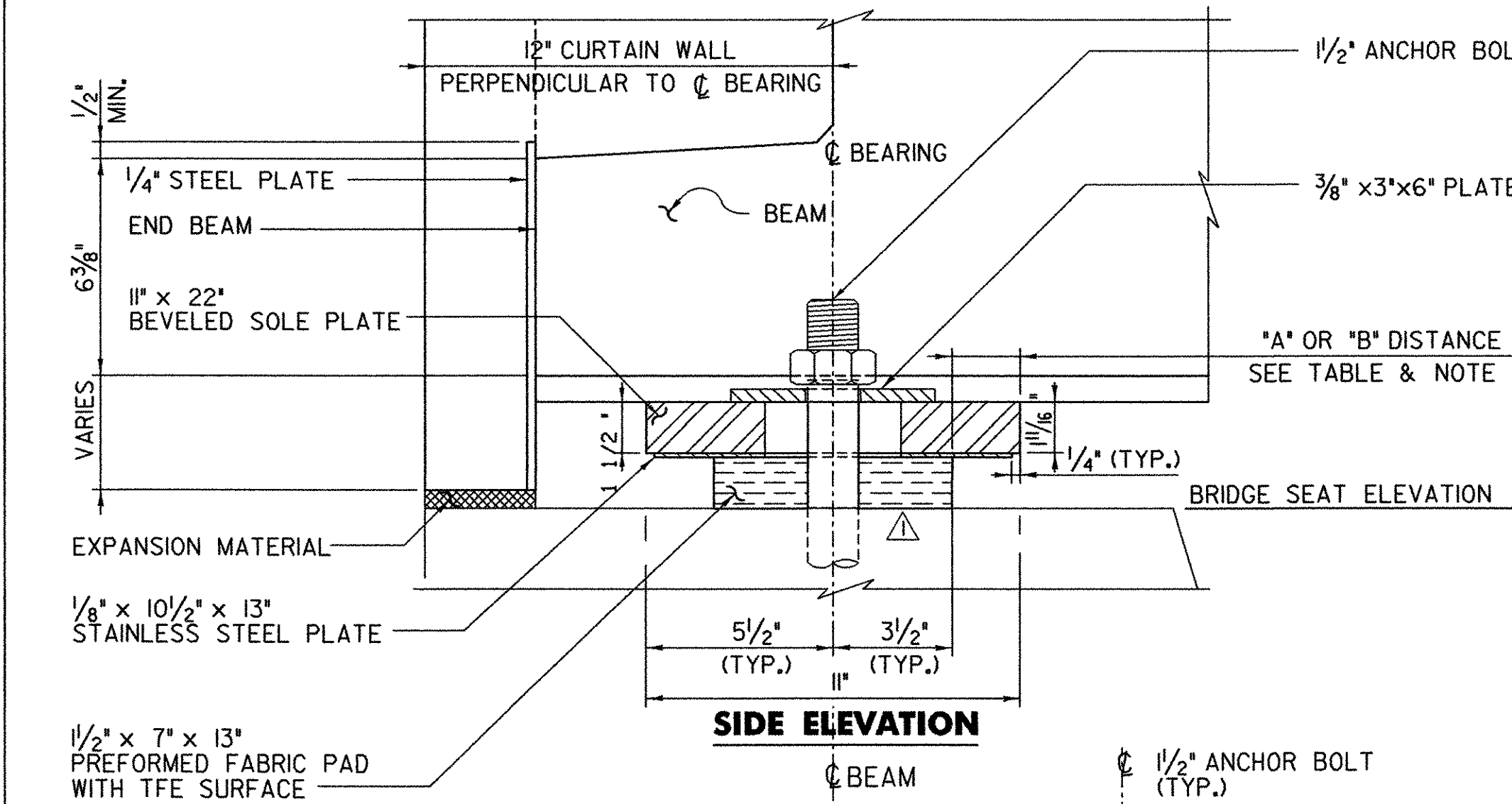
- NOTES:**
1. THE BASELINE IS NOT HORIZONTAL BUT IS FROM BOTTOM OF BEAM TO BOTTOM OF BEAM BETWEEN ABUTMENT SUPPORTS. THE SLOPE OF BASELINE DIFFERS FOR SPAN 1 AND SPAN 2.
 2. THE DEAD LOAD DEFLECTION DIAGRAM INCLUDES BEAM SELF WEIGHT.

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
1	REVISED CAMBER DIAGRAM	APG 2/3/04	Town Of	CHESTER
2	REVISED DEAD LOAD DEFLECTION DIAGRAM	APG 2/3/04	Highway No.	TH 5
			CHURCH STREET OVER WILLIAMS RIVER	
			BEAM ELEVATION & DETAILS	
			Designed By	R. L. JOY
			Checked By	J. TUCKER
			Date	03/01
			Bridge Design Supervisor	J. TUCKER
			Date	03/01
			PROJECT	CHESTER
			PROJECT NO.	BHO 1442 (32)
			I.G.C. Info.	...zj365dl5 .dgn
			D & K DWG NO.	11358
			Sheet	15 of 22

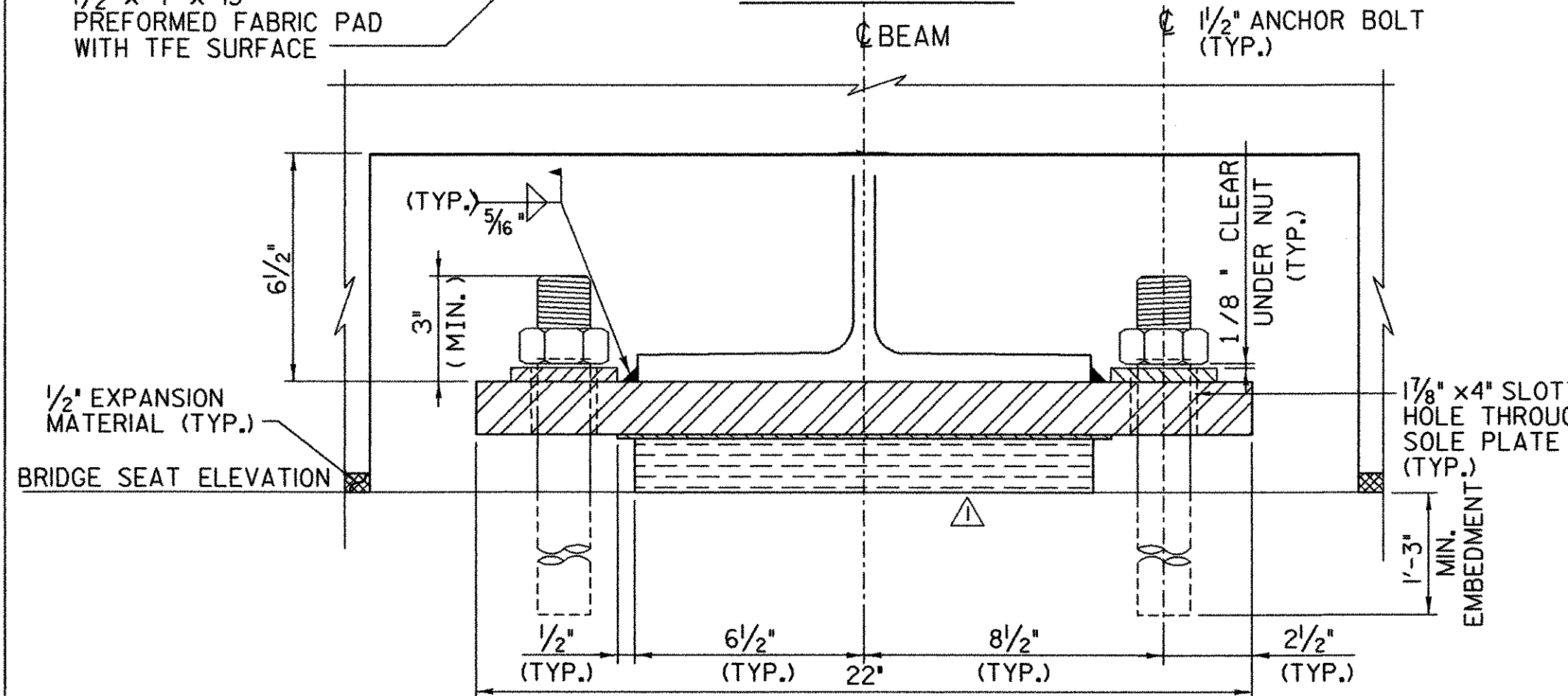
02/10/2004



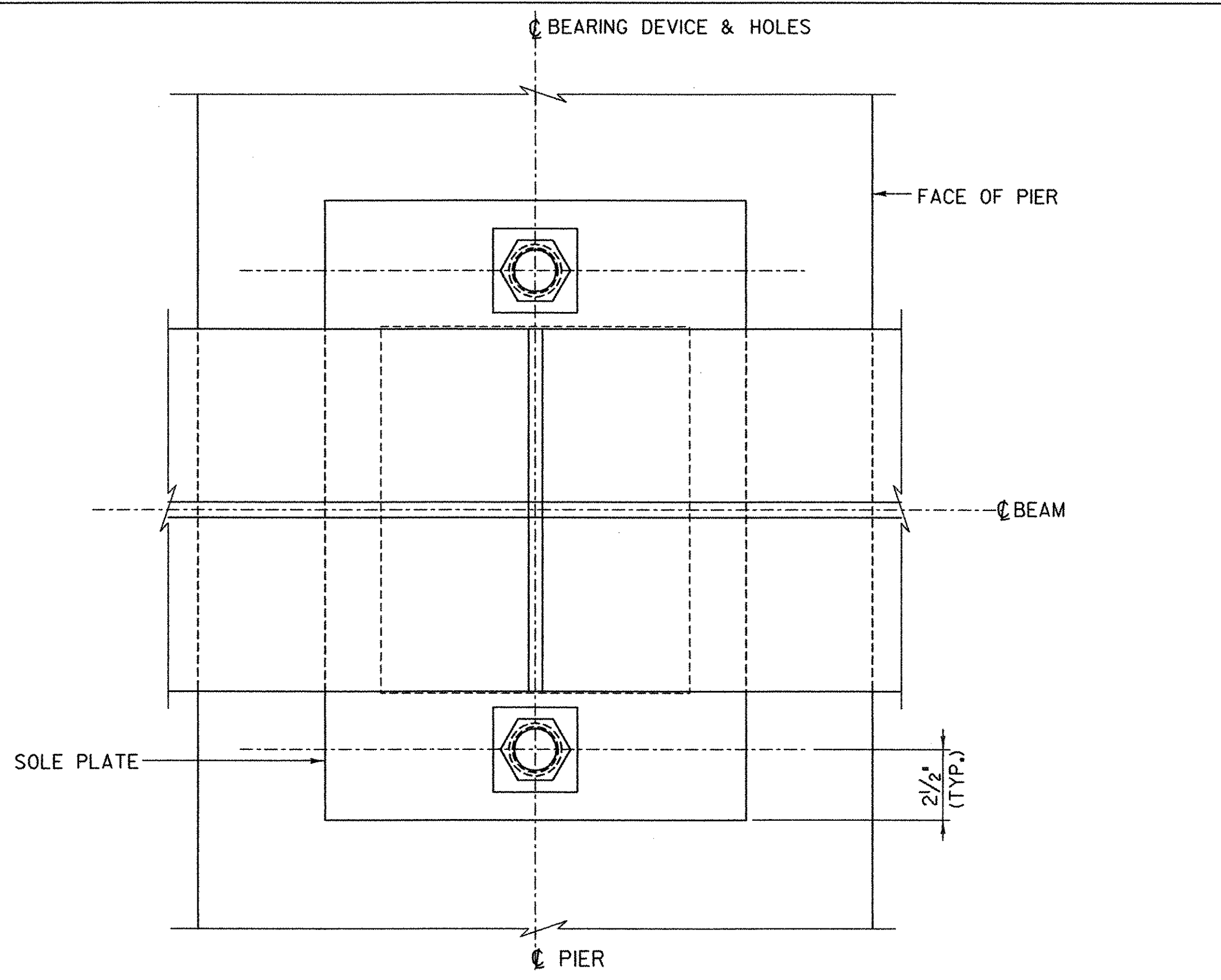
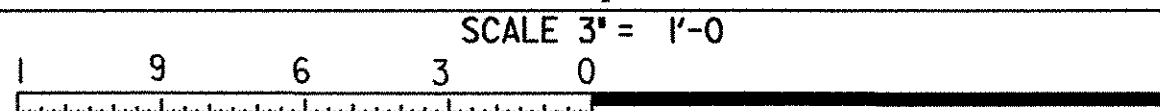
PLAN



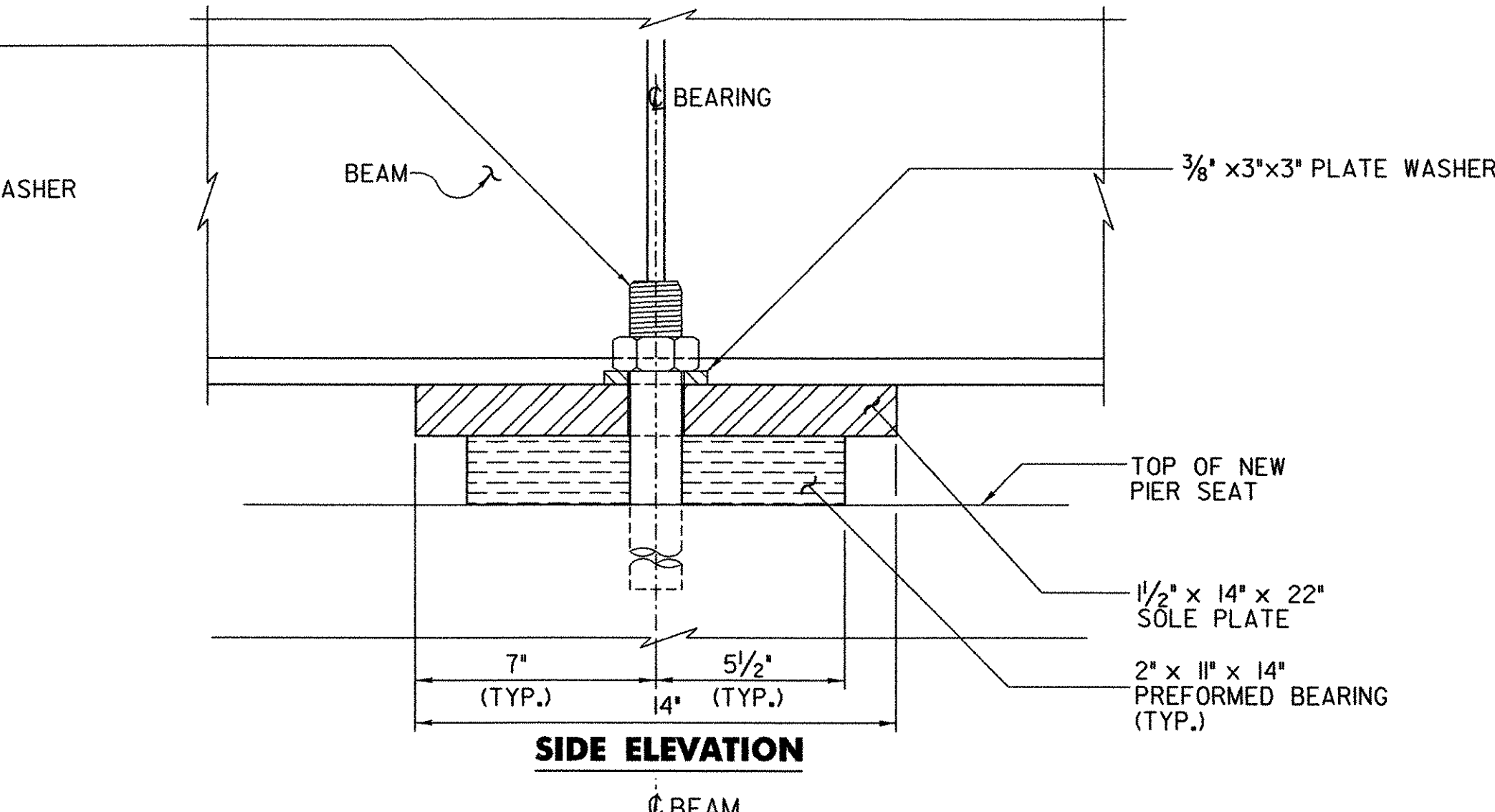
SIDE ELEVATION



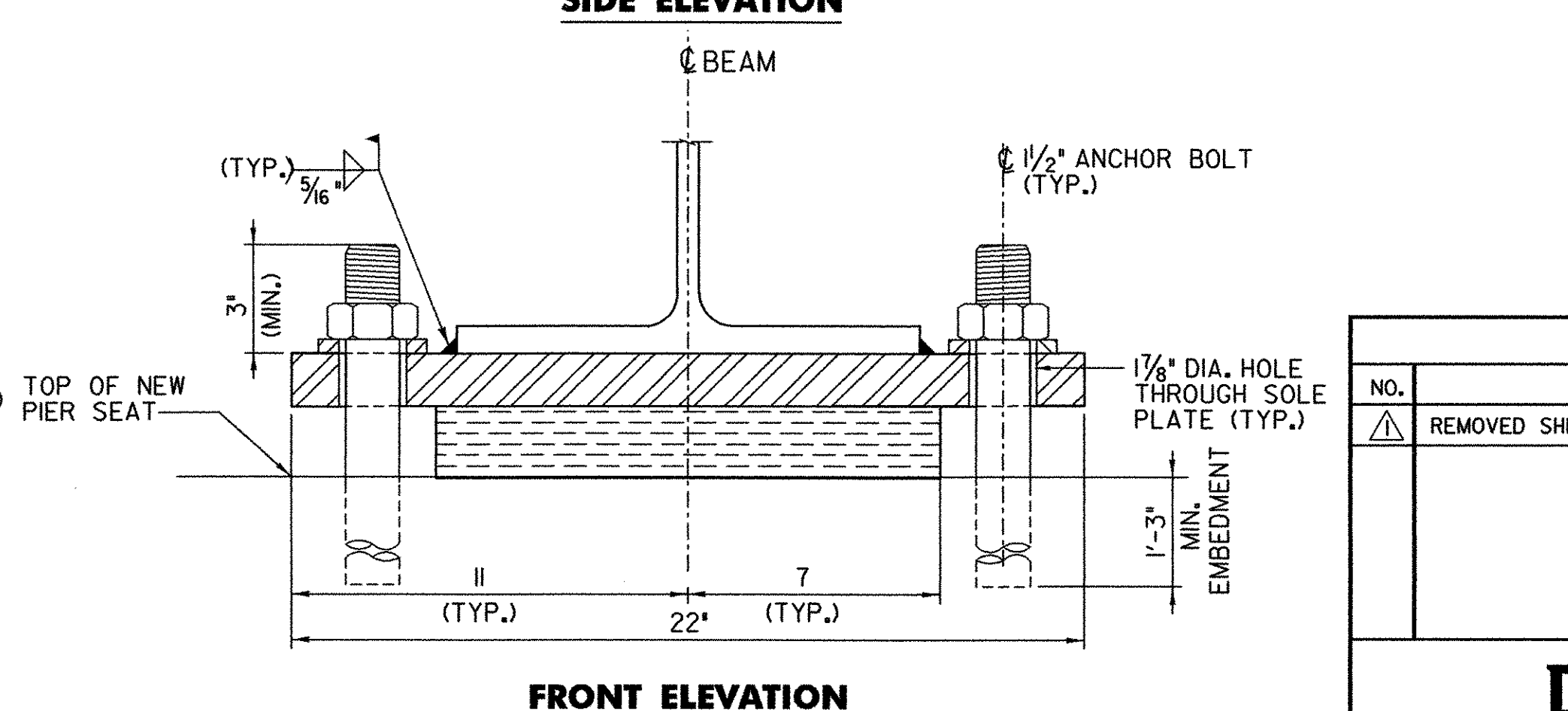
FRONT ELEVATION
EXPANSION BEARING DETAILS
(ABUTMENT NO. 1 SHOWN, ABUTMENT NO. 2 SIMILAR)



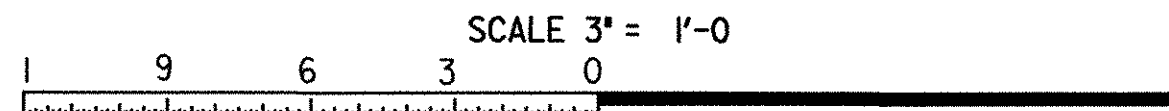
PLAN



SIDE ELEVATION



FRONT ELEVATION
FIXED BEARING DETAILS
(PIER)



TEMPERATURE ADJUSTMENT TABLE		
TEMP ° F	*A* DIST. (INCHES)	*B* DIST. (INCHES)
120	1 5/8	1 3/4
105	1 3/4	1 7/8
90	1 3/4	1 7/8
75	1 7/8	2
60	1 7/8	2
45	2	2 1/8
30	2 1/8	2 1/8
15	2 1/8	2 1/4
0	2 1/4	2 1/4
-15	2 1/4	2 3/8
-30	2 3/8	2 3/8

NOTE:

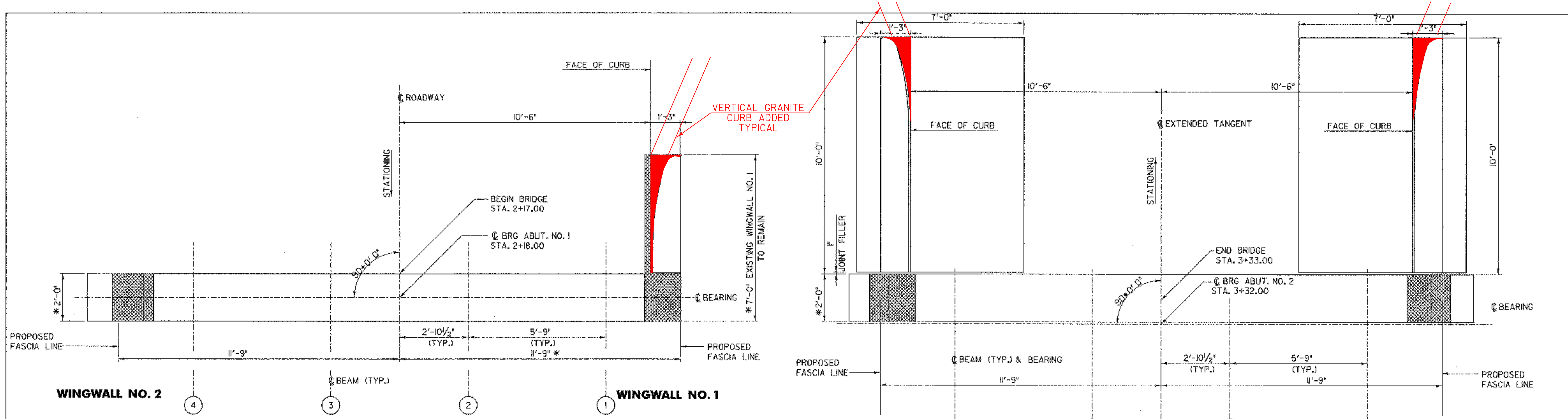
1. THE "A" DISTANCE IS THE SOLE PLATE ADJUSTMENT TO BE USED AFTER THE DEAD LOAD IS ADDED TO THE BEAM SELF WEIGHT. THE "B" DISTANCE IS THE SOLE PLATE ADJUSTMENT TO BE USED AFTER BEAM ERECTION AND BEFORE DECK IS POURED. THE DIFFERENCE IS THE ELONGATION OF THE BOTTOM FLANGE DUE TO DEAD LOAD DEFLECTION FROM THE SLAB, BRUSH CURB, BRIDGE RAIL, AND PAVEMENT.
2. SEE SHEET 10 FOR ADDITIONAL BEARING NOTES.

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
1	REMOVED SHIM PLATE FROM BEARING ASSEMBLY	APG 2/3/04	Town Of CHESTER	
			Highway No. TH 5	Bridge No. 11
			Log Sta. -	Surv. Sta. -
			CHURCH STREET OVER WILLIAMS RIVER	
			BEARING DETAILS	
			Designed By R. L. JOY	Drawn By G. J. WOJCIECHOWSKI
			Checked By J. TUCKER	Date 03/01
			Bridge Design Supervisor J. TUCKER	Date 03/01
			PROJECT CHESTER	PROJECT NO. BHO 1442 (32)
			I.G.C. Info. ...zj365dl6.dgn	
			D & K DWG NO. 11359	Sheet 16 of 22

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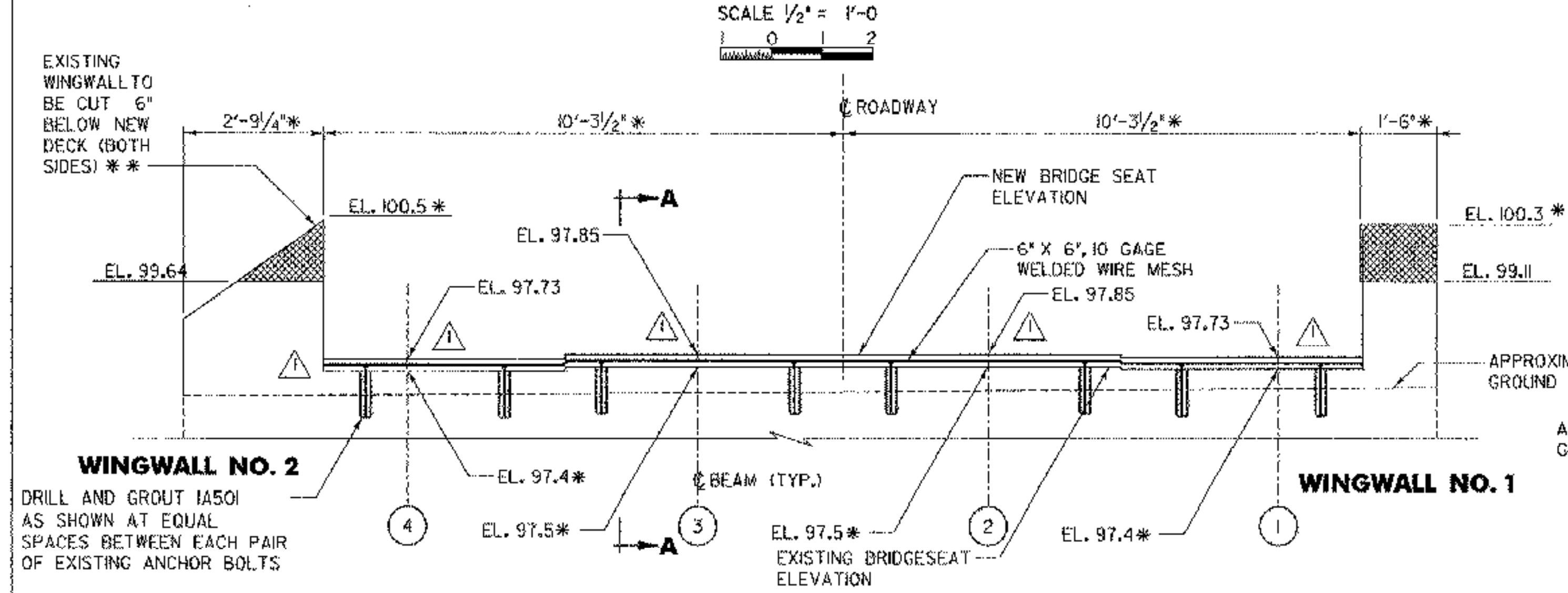
engineering planning management development

02/10/2004

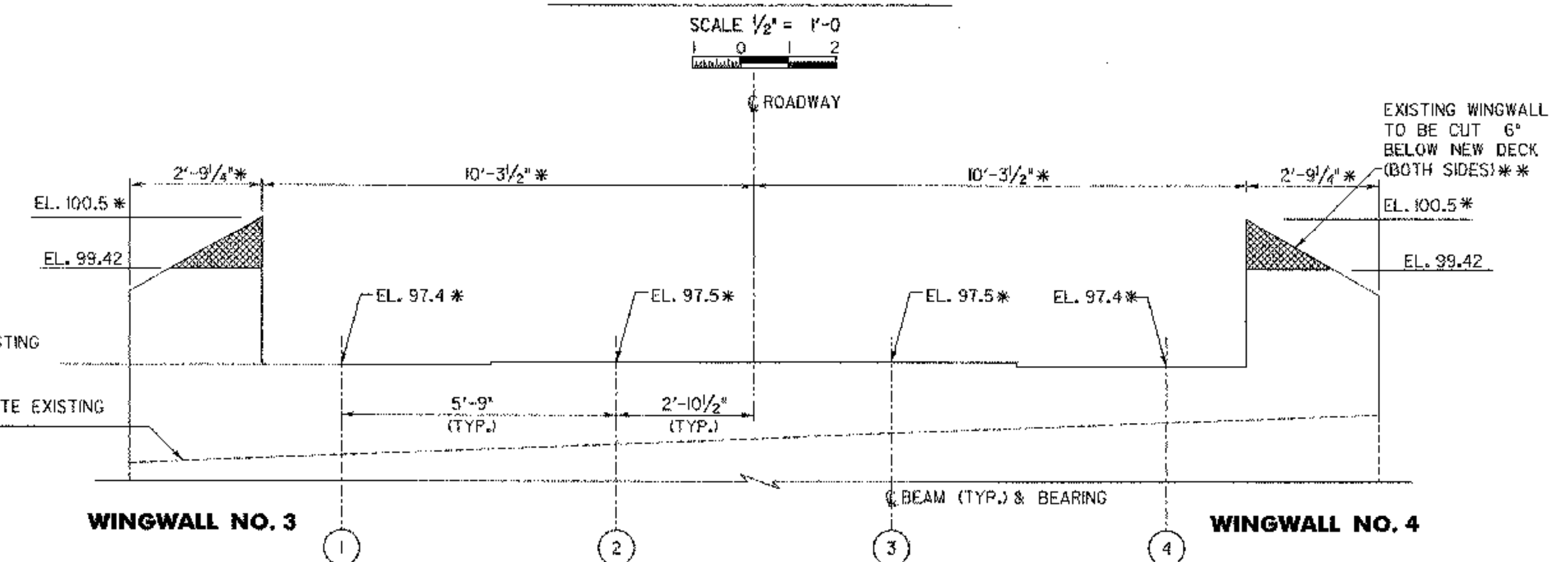


ABUTMENT NO. 1 - PLAN

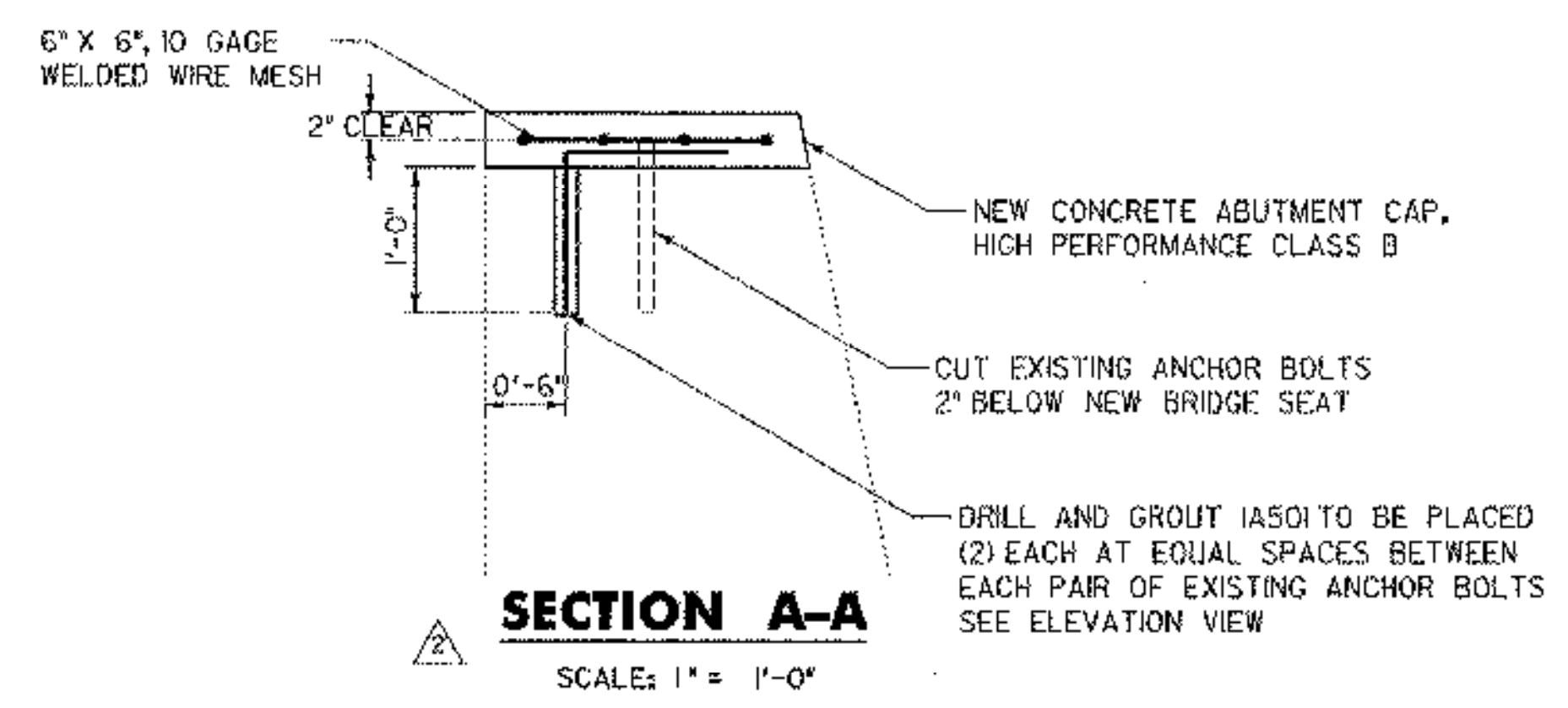
ABUTMENT NO. 2 - PLAN



ABUTMENT NO. 1 - ELEVATION



ABUTMENT NO. 2 - ELEVATION



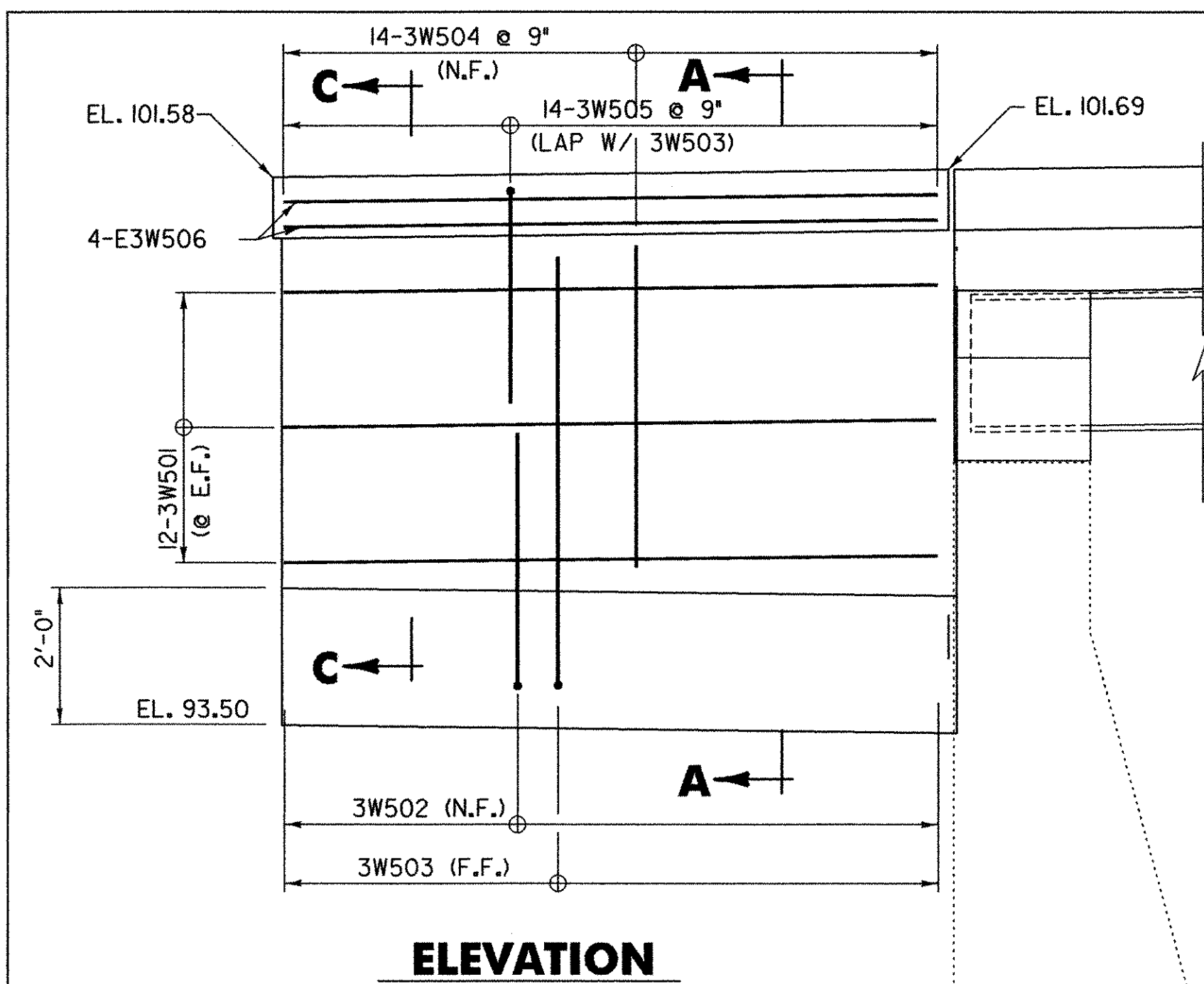
SECTION A-A

- DENOTES LIMITS OF REMOVAL
- * APPROXIMATE EXISTING ELEVATION OR DIMENSION
- ** TOPS OF EXISTING WINGWALLS SHALL BE REMOVED TO 6" MIN. BELOW THE ANTICIPATED BOTTOM OF NEW DECK. CONCRETE, HIGH PERFORMANCE CLASS B SHALL BE PLACED TO BRING THE TOP OF THE WINGWALL TO INCH BELOW THE COMPUTED BOTTOM OF DECK. RETAIN EXISTING REINFORCING, SEE REMOVAL NOTES SHEET 10. 1" INCH CLOSED CELL FOAM WILL BE PLACED ON TOP OF THE NEW WINGWALL CONCRETE BEFORE THE NEW DECK IS POURED.

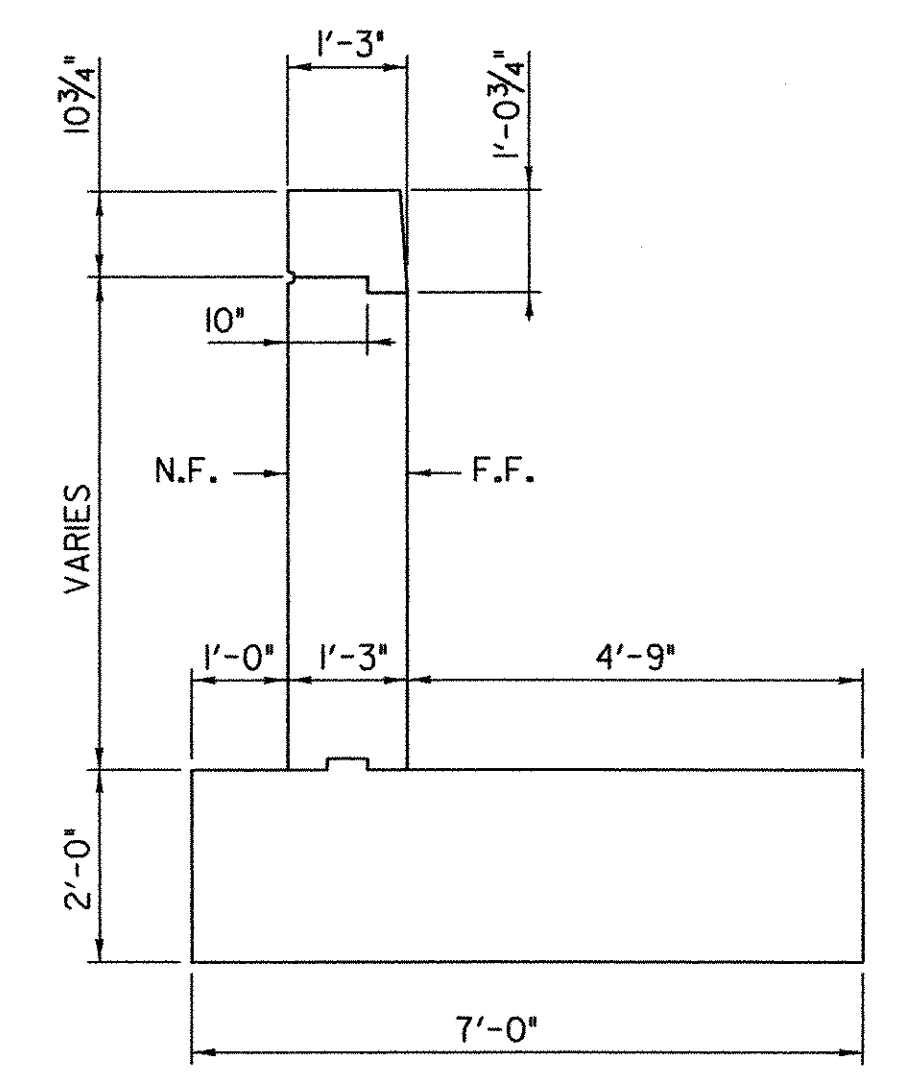
REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE	Town Of	Bridge No.
1	ADDED ABUTMENT CAP AND MODIFIED BRIDGE SEAT ELEVATIONS OF ABUTMENT NO. 1 ELEVATION	APG 2/3/04	CHESTER	11
2	ADDED SECTION A-A	APG 2/3/04	Highway No. TH 5	Log Sta. -
			CHURCH STREET OVER WILLIAMS RIVER	Surv. Sta. -
ABUTMENTS DETAILS				
Designed By J. W. KAMB		Drawn By G. J. WOJCIECHOWSKI		
Checked By J. TUCKER		Bridge Design Supervisor		
Date 03/01		Date 03/01		
PROJECT CHESTER		PROJECT NO. BHO 1442 (32)		
I.G.C. Info. ...zj365dl7.dgn				
D & K DWG NO. R360		Sheet 17 of 22		

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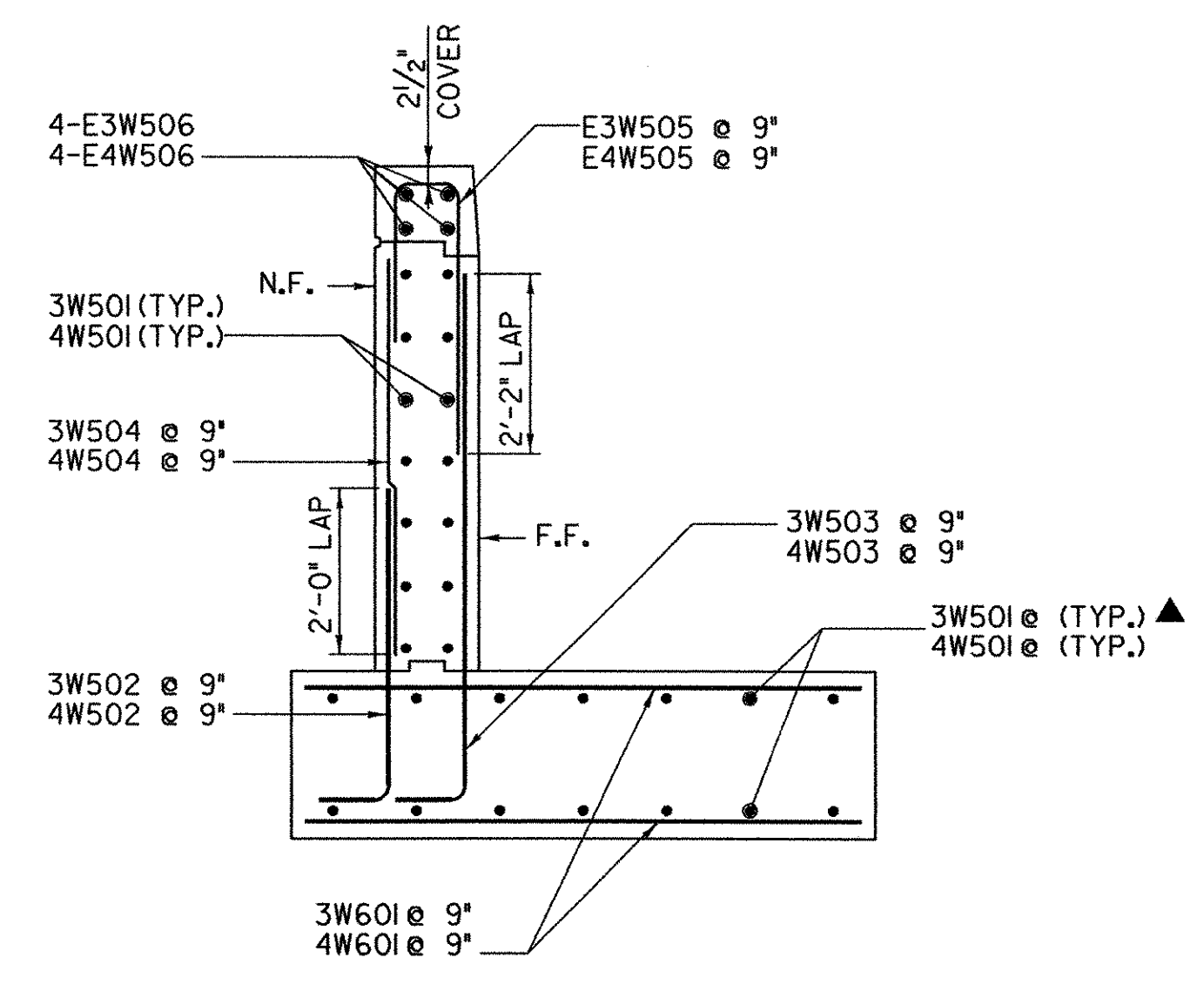
02/13/2004



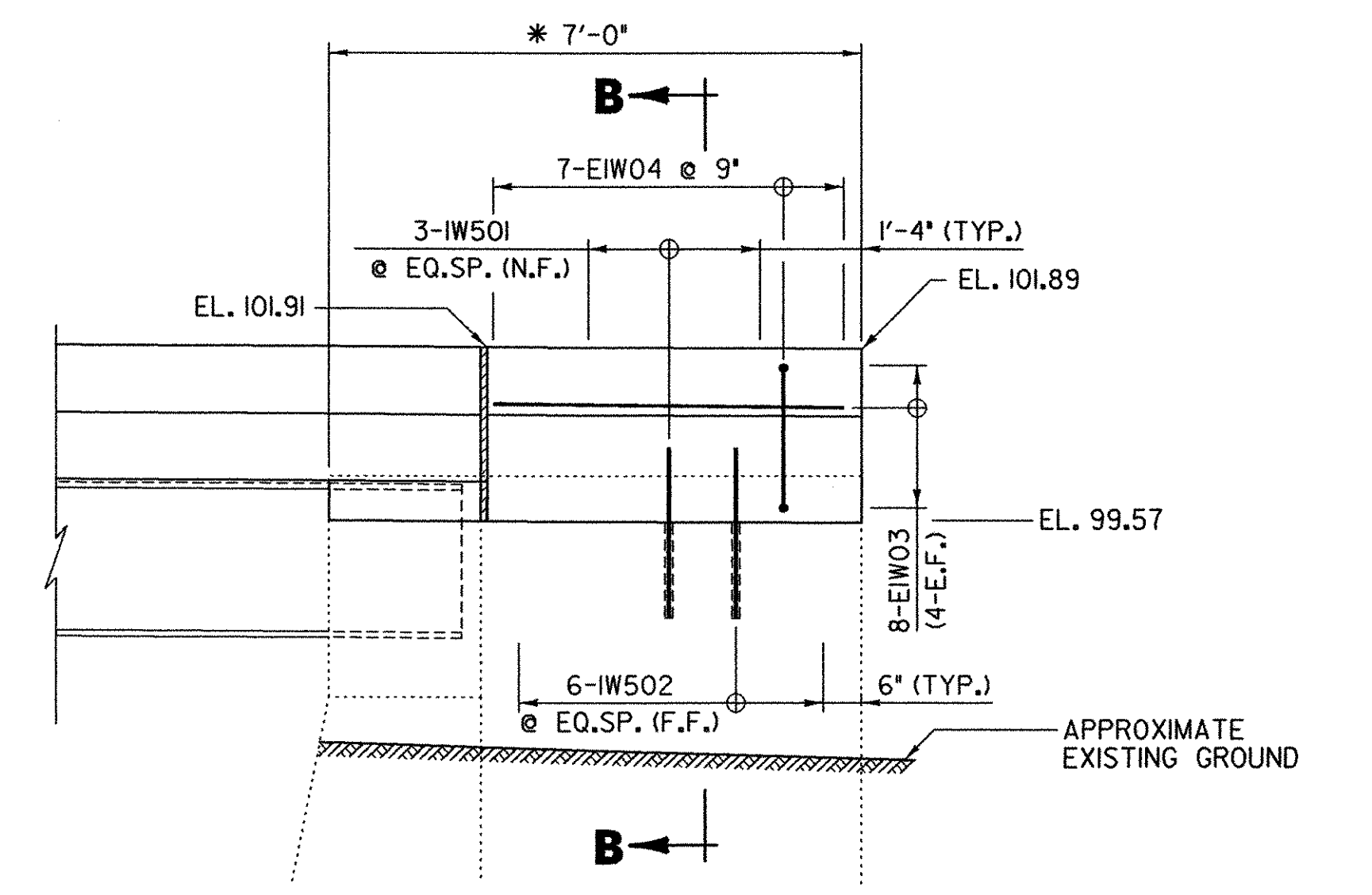
**ELEVATION
WINGWALL NO. 3**
SCALE 1/2" = 1'-0"
0 1 2



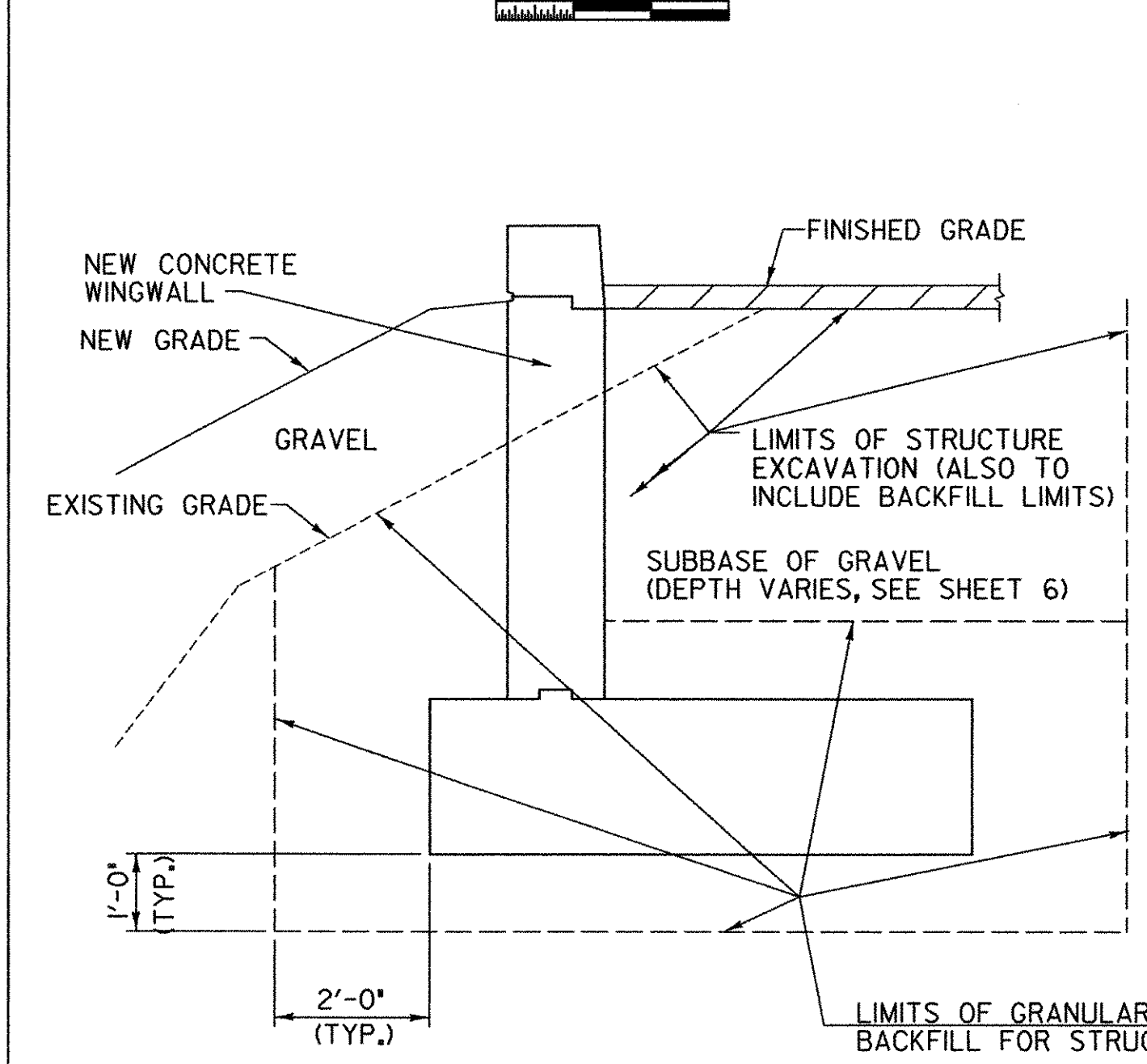
SECTION A-A
SCALE 1/2" = 1'-0"
0 1 2



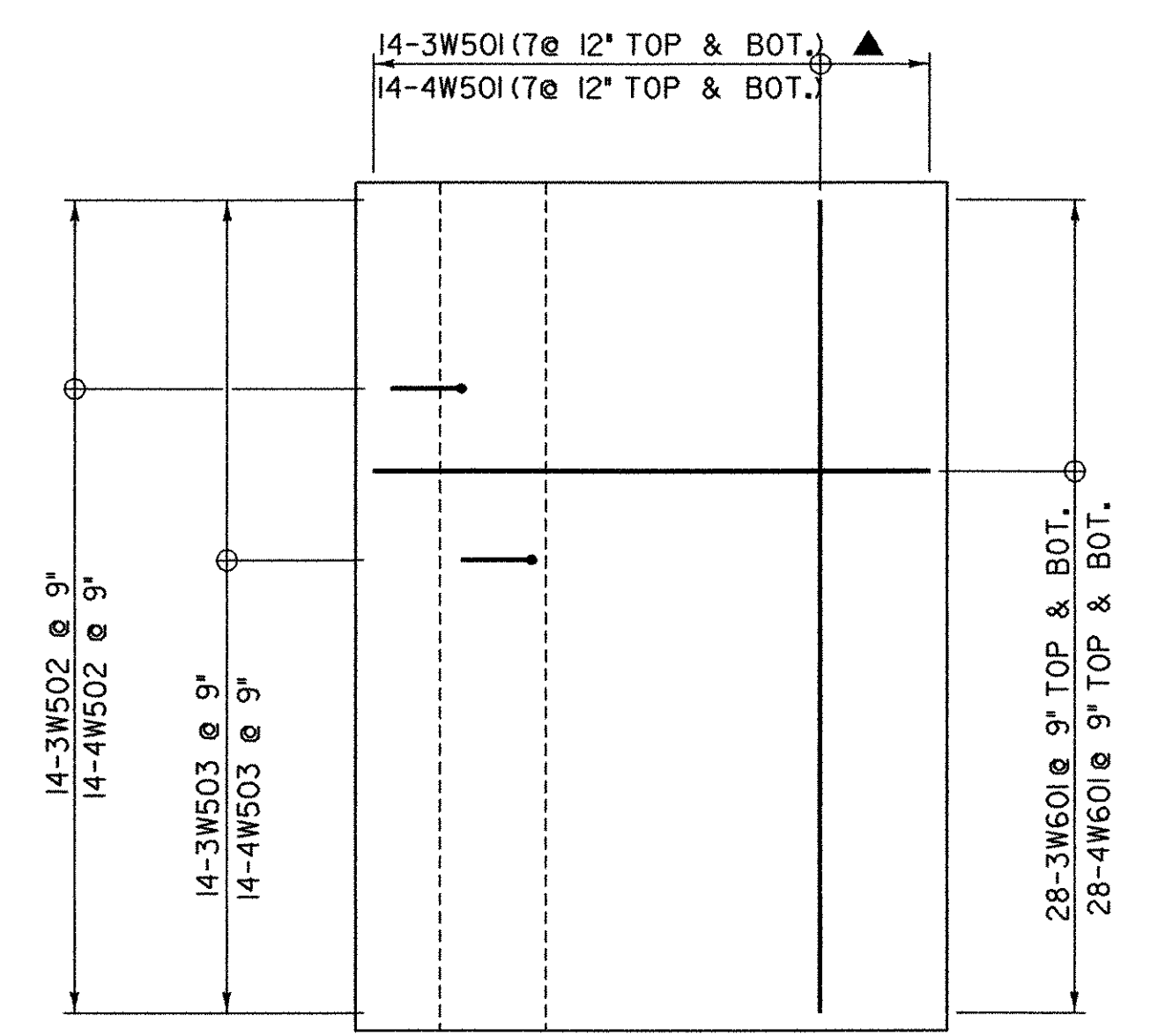
SECTION A-A
SCALE 1/2" = 1'-0"
0 1 2



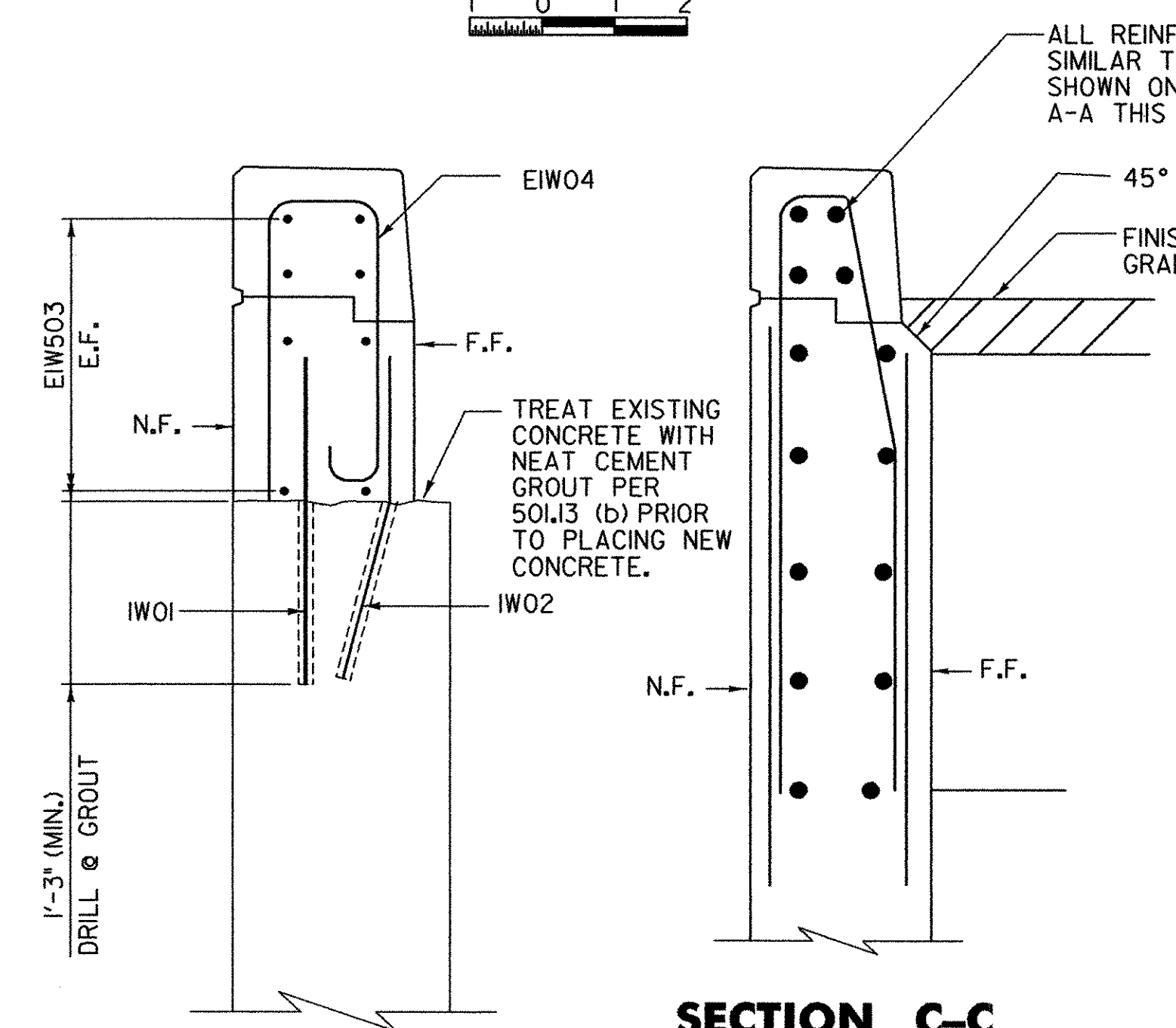
**ELEVATION
WINGWALL NO. 1**
SCALE 1/2" = 1'-0"
0 1 2



TYPICAL WINGWALL EARTHWORK SECTION
SCALE 1/2" = 1'-0"
0 1 2

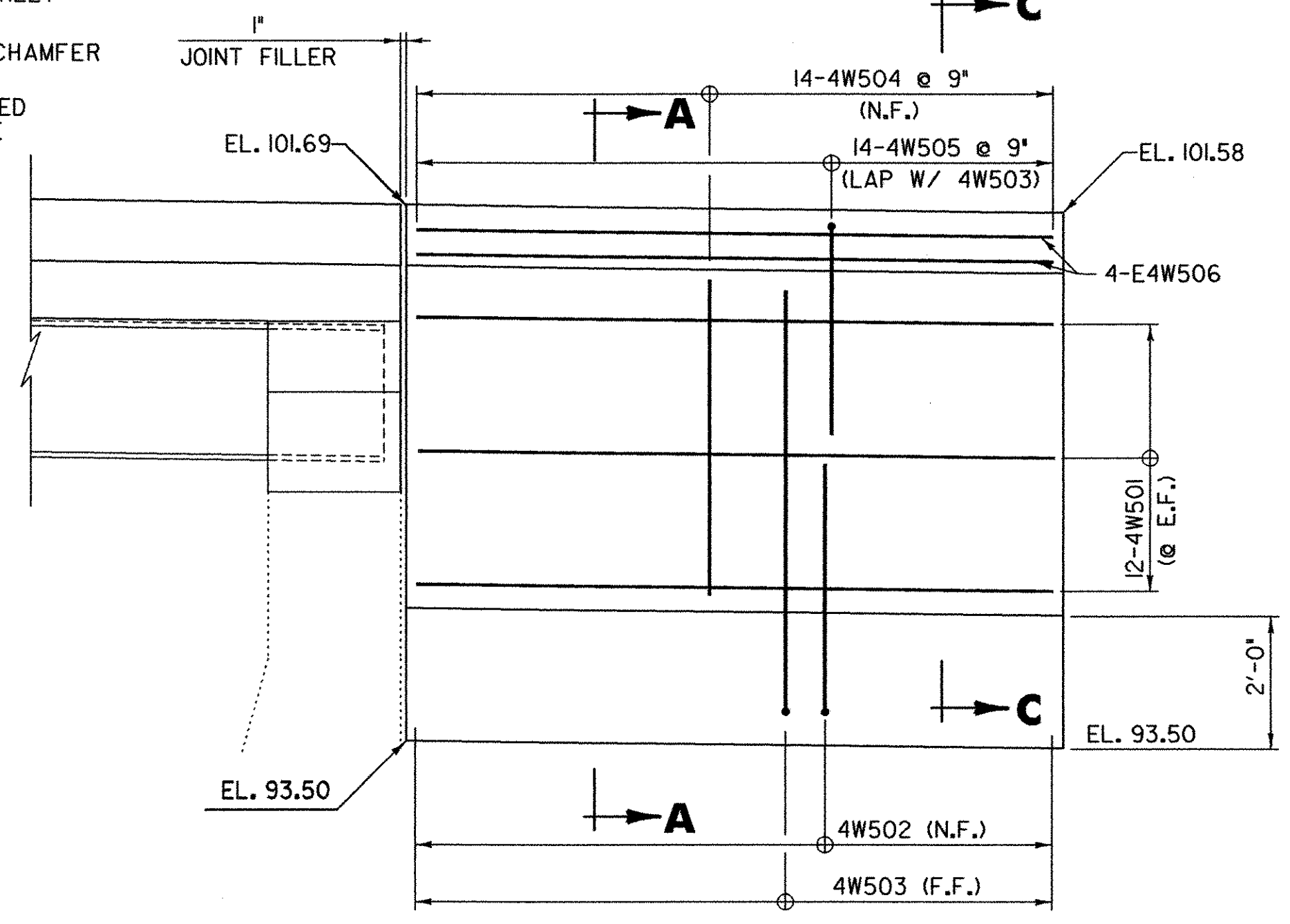


FOOTING PLAN
SCALE 1/2" = 1'-0"
0 1 2

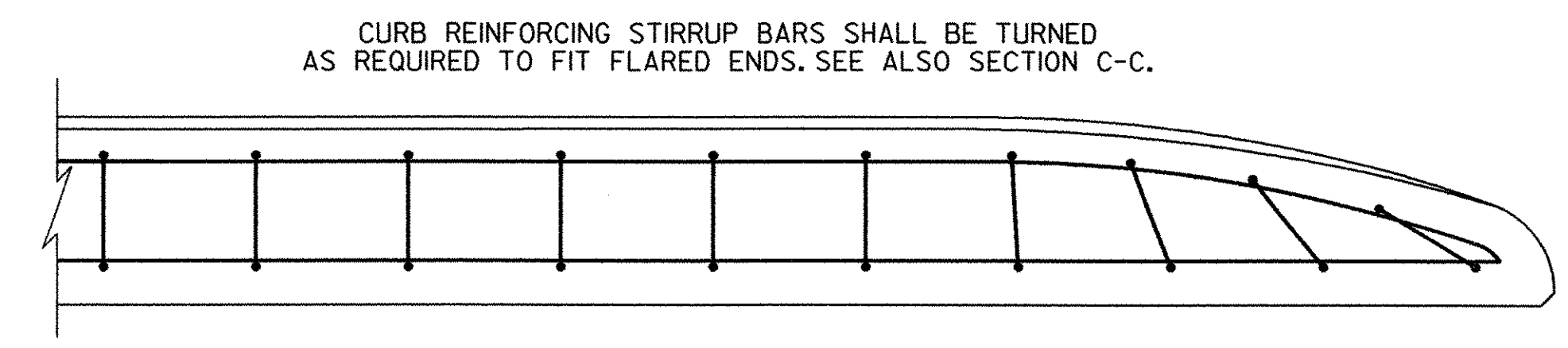


SECTION B-B
SCALE 1" = 1'-0"
0 1 2

SECTION C-C
SCALE 1" = 1'-0"
0 1 2



**ELEVATION
WINGWALL NO. 4**
SCALE 1/2" = 1'-0"
0 1 2

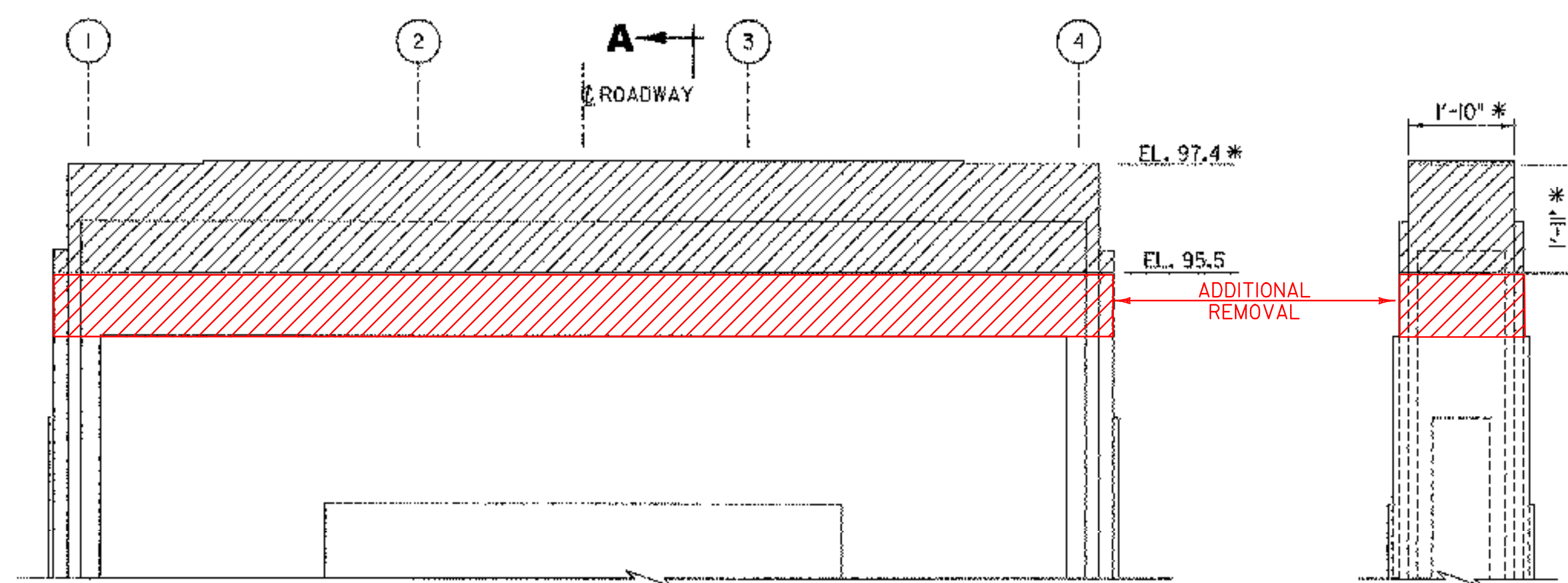


FLARED END DETAIL FOR 1'-3" CURB
SCALE 1" = 1'-0"
0 1 2

* - APPROXIMATE EXISTING ELEVATION OR DIMENSION
F.F. - FAR FACE
N.F. - NEAR FACE
EQ.SP. - EQUAL SPACE

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION Town Of CHESTER Bridge No. 11 Highway No. TH 5 Log Sta. - Surv. Sta. - CHURCH STREET OVER WILLIAMS RIVER WINGWALL DETAILS Designed By B. C. AUSTIN Drawn By G. J. WOJCIECHOWSKI Checked By J. TUCKER Date 03/01 Bridge Design Supervisor J. TUCKER Date 03/01 PROJECT CHESTER PROJECT NO. BHO 1442 (32) I.G.C. Info. ...ZJ365dl8.dgn D & K DWG NO. 11361 Sheet 18 of 22
NO.	DESCRIPTION	BY & DATE	

02/03/2004

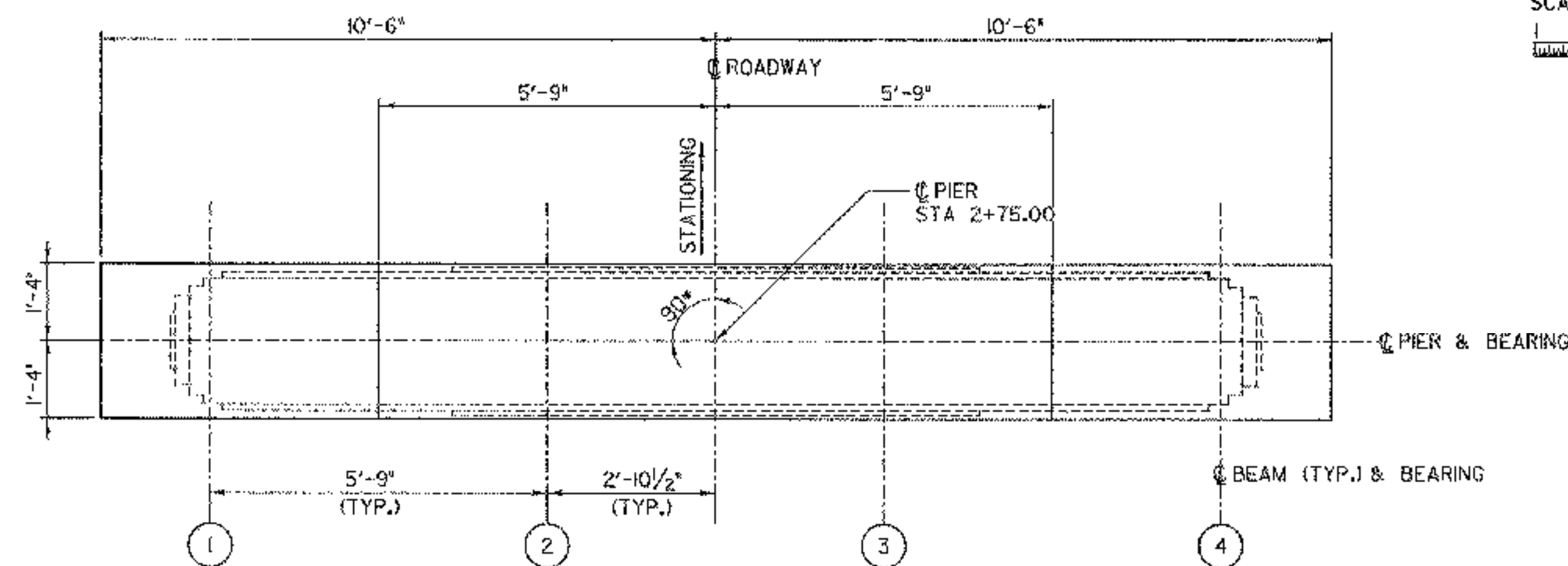


ELEVATION - REMOVAL

SCALE 1/2" = 1'-0"

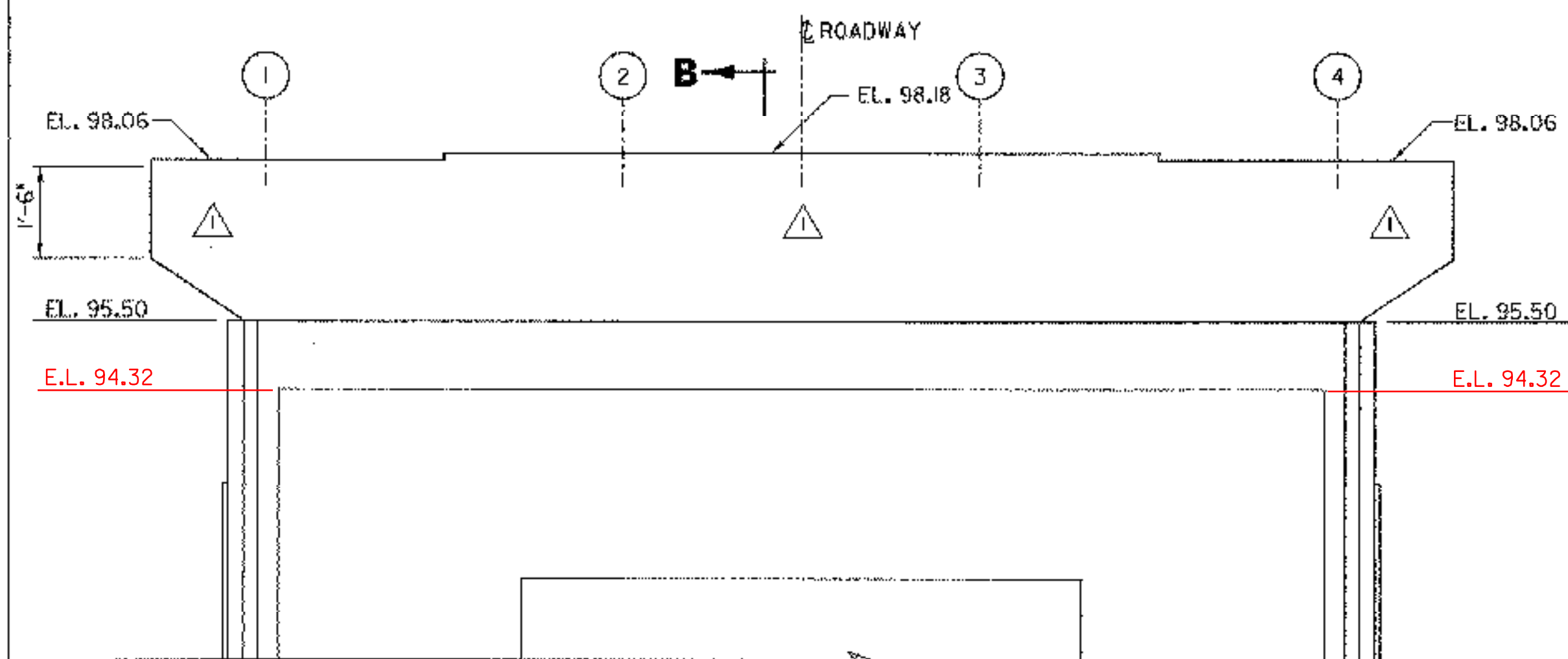
SECTION A-A

SCALE 1/2" = 1'-0"



PLAN - MASONRY

SCALE 1/2" = 1'-0"

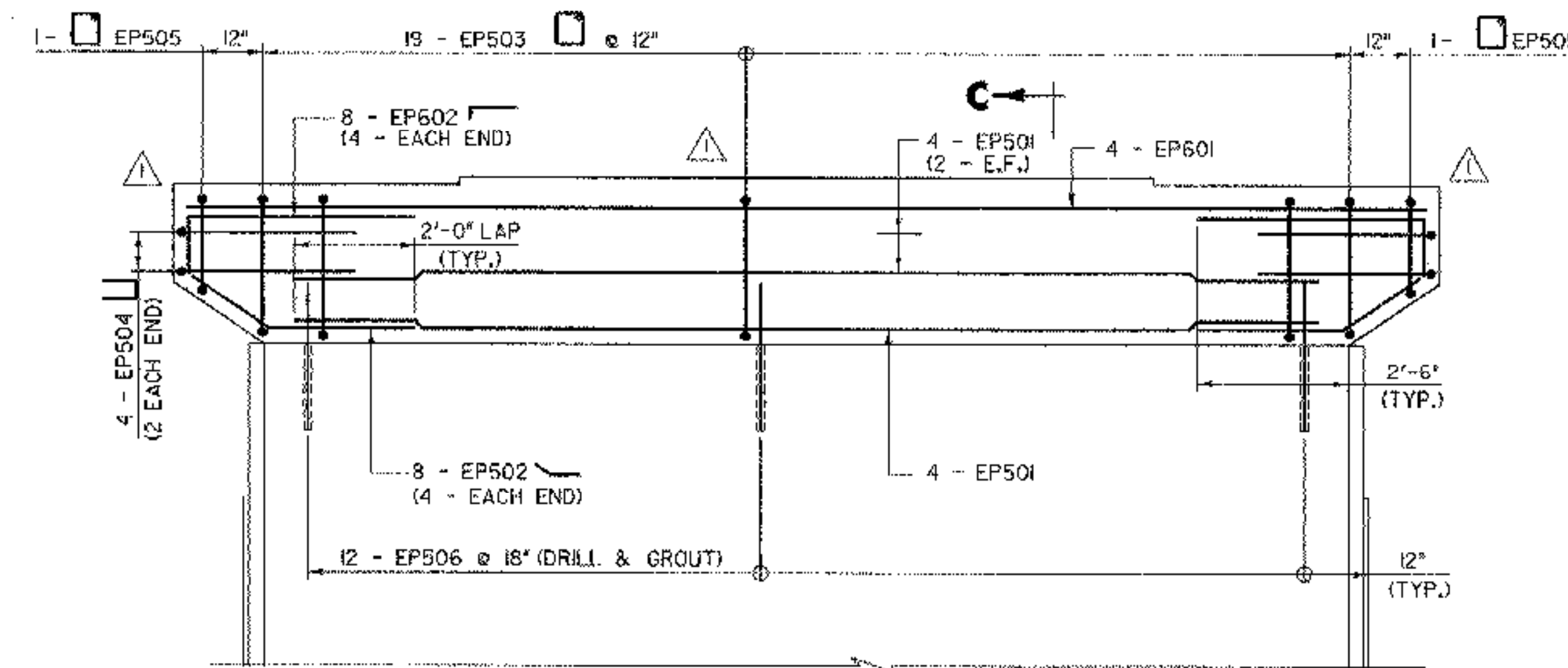


ELEVATION - MASONRY

SCALE 1/2" = 1'-0"

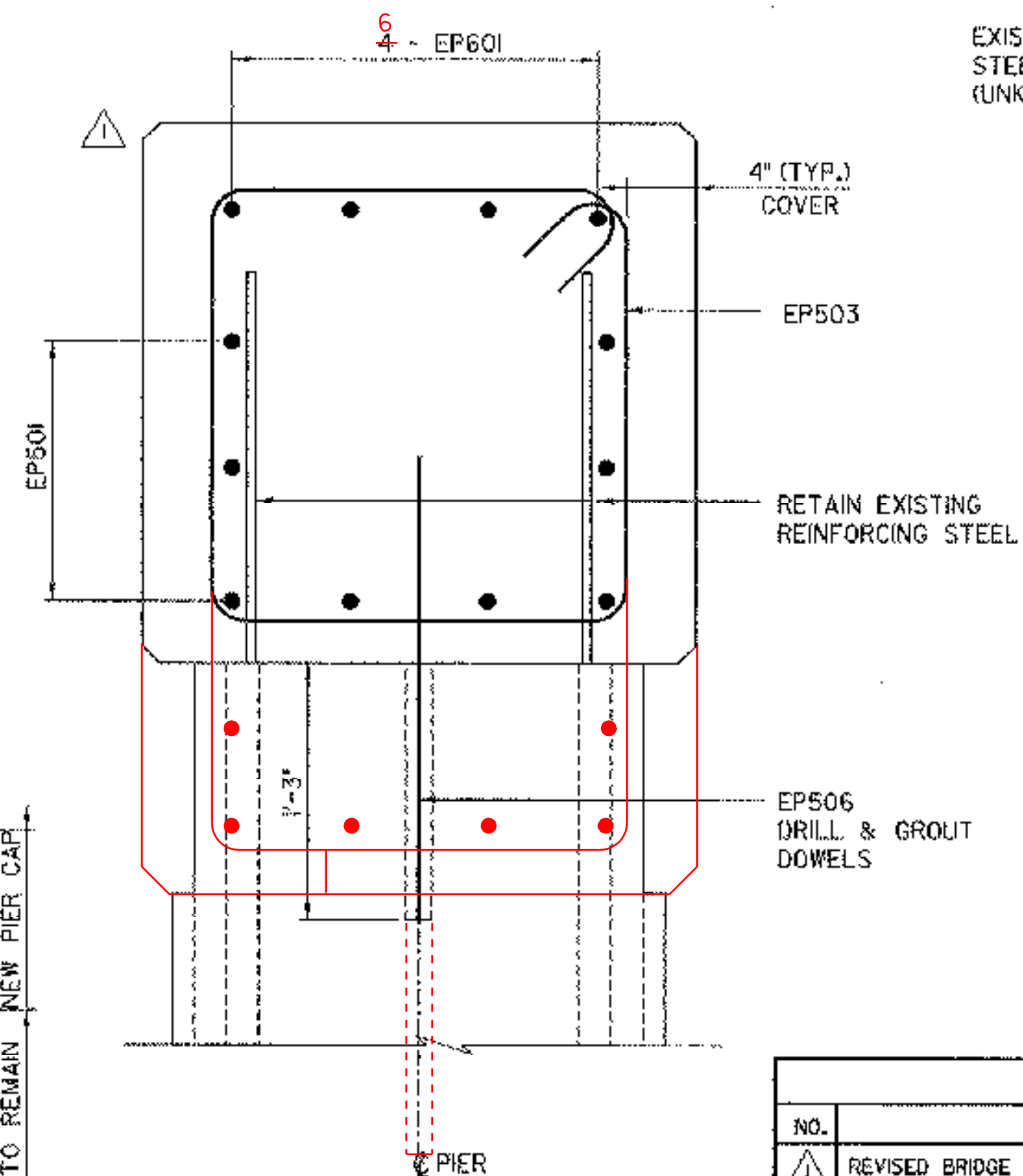
SECTION B-B

SCALE 1/2" = 1'-0"



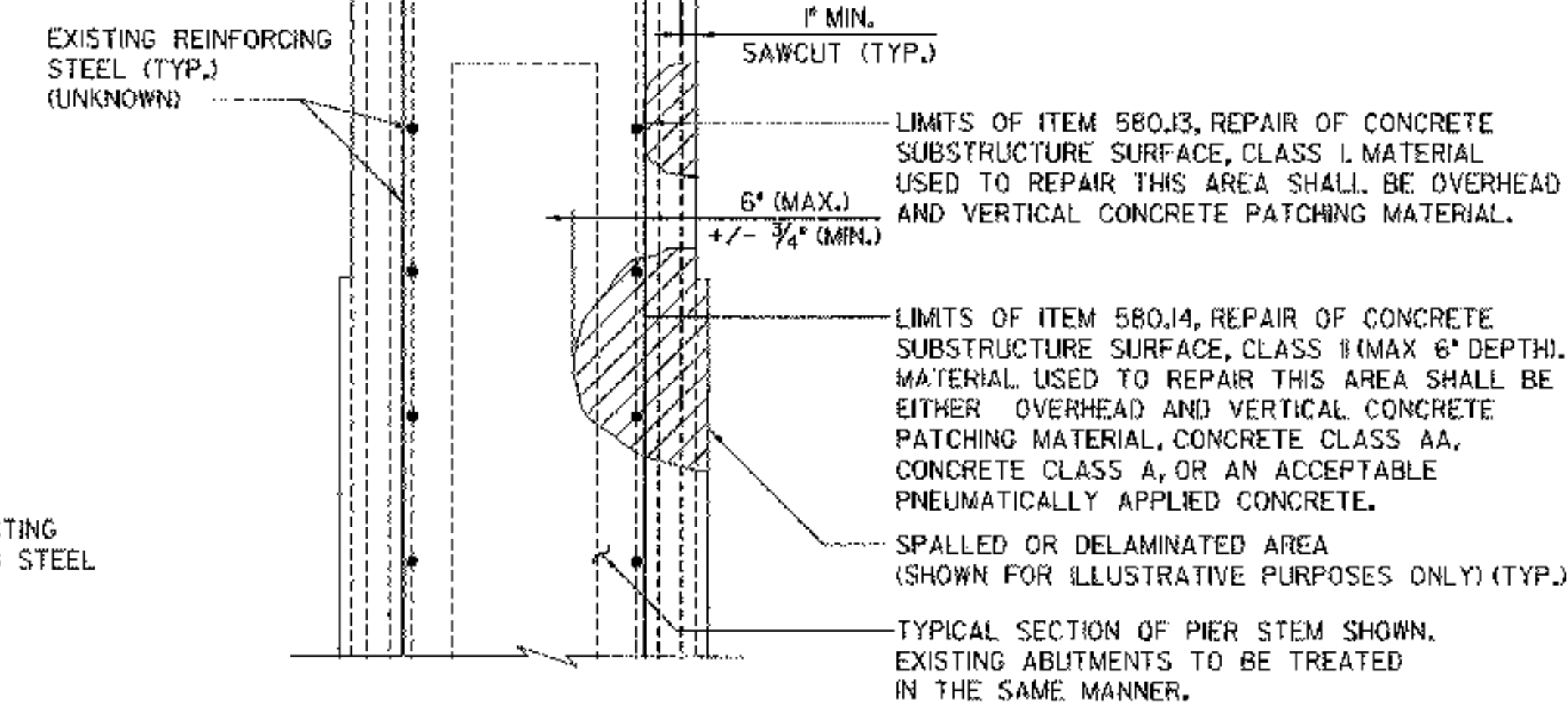
ELEVATION - REINFORCEMENT

SCALE 1/2" = 1'-0"



SECTION C-C

SCALE 1/2" = 1'-0"



**REPAIR OF CONCRETE SUBSTRUCTURE SURFACE
CLASS I OR CLASS II**

N.T.S.

DENOTES LIMITS OF REMOVAL

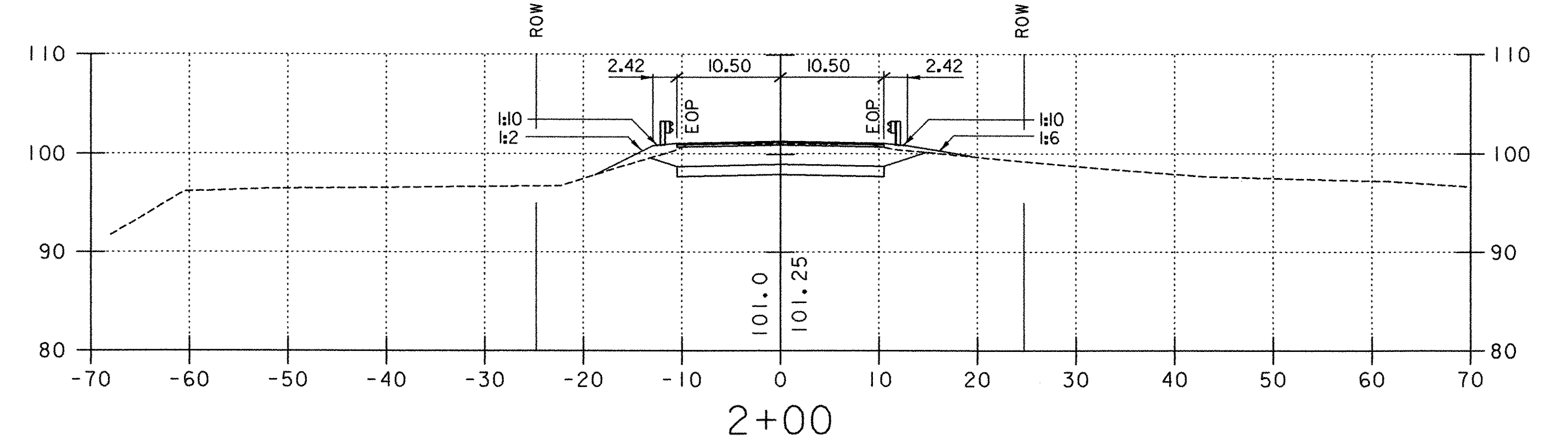
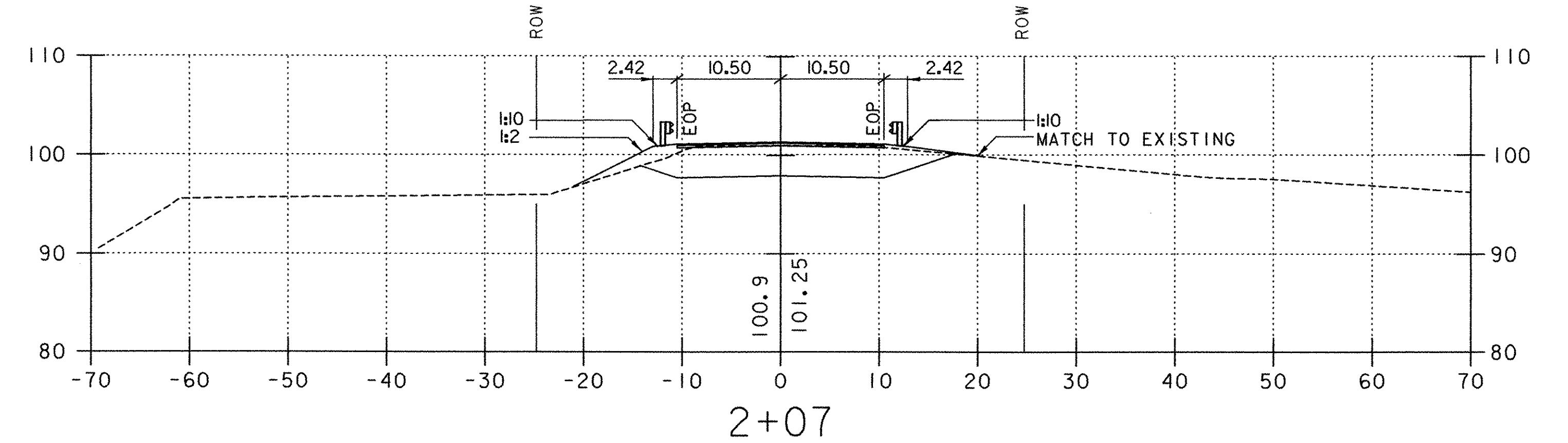
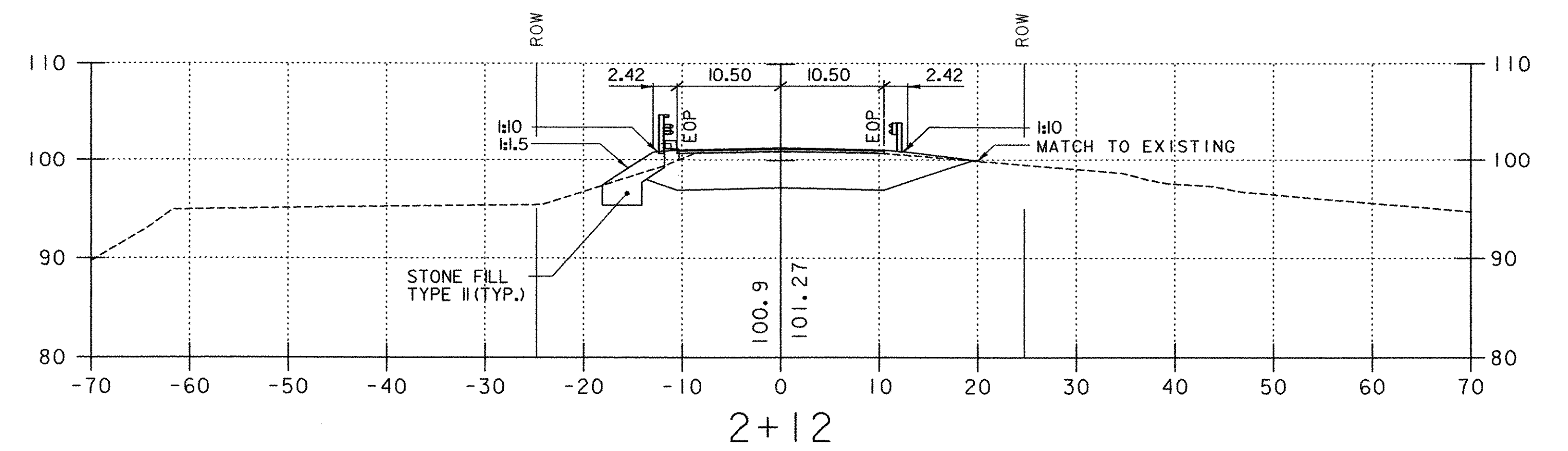
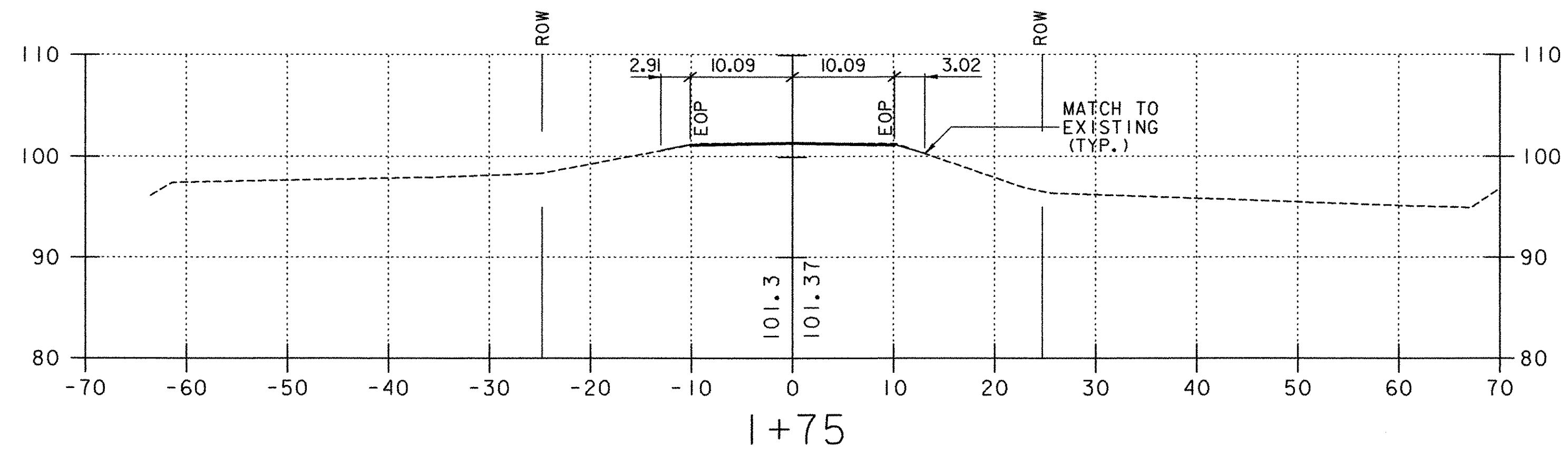
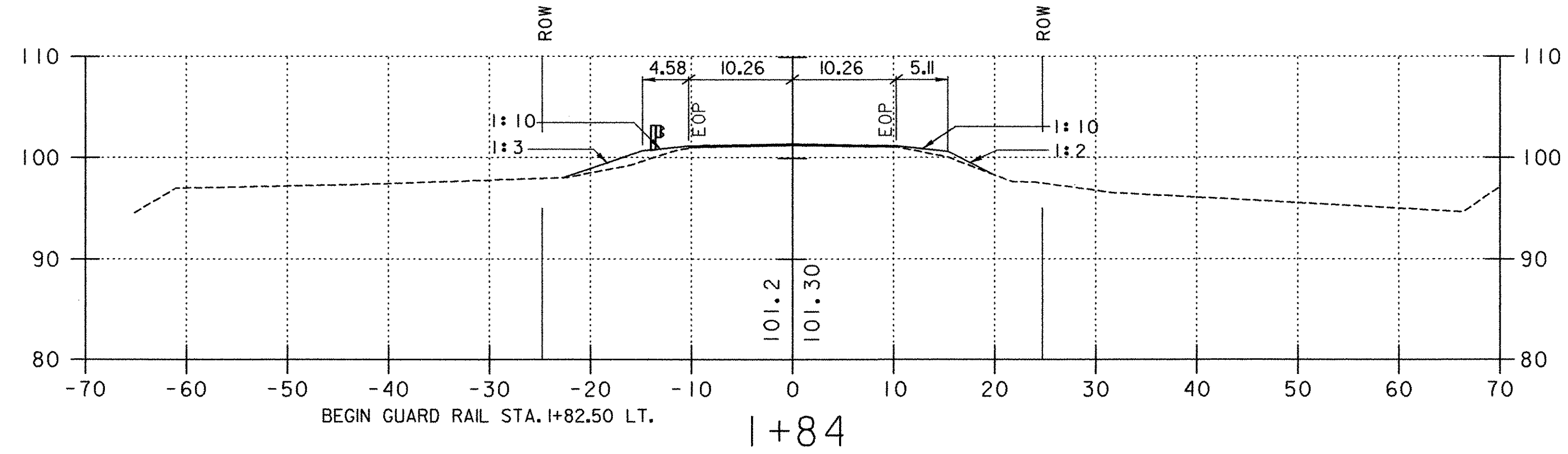
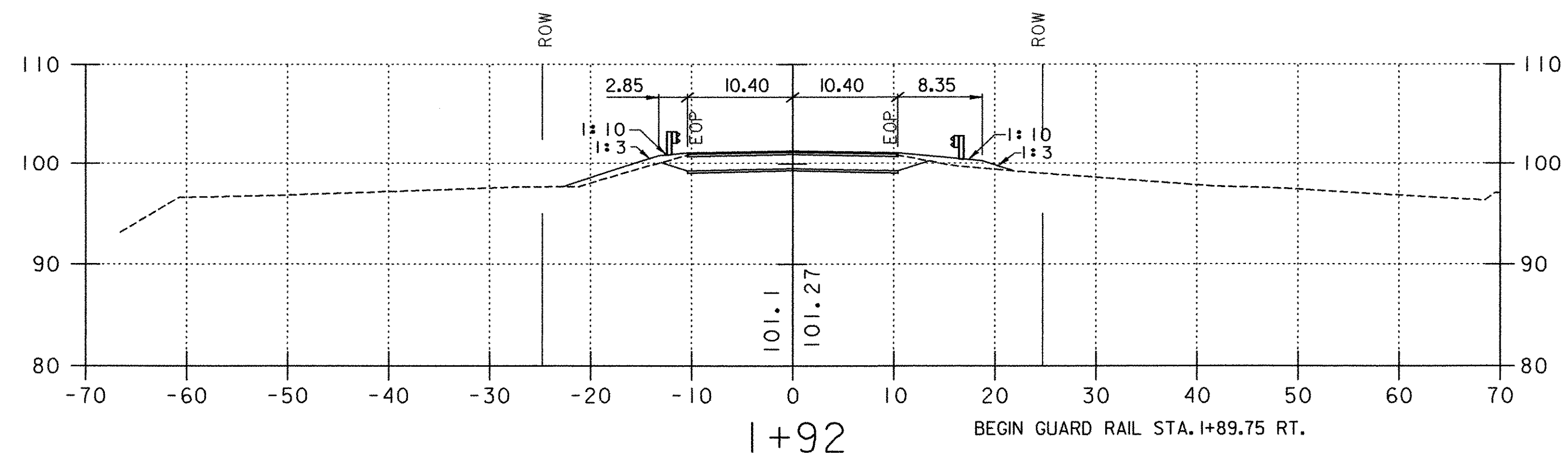
* APPROXIMATE EXISTING ELEVATION

REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION		
NO.	DESCRIPTION	BY & DATE			
1	REVISED BRIDGE SEAT ELEVATIONS	APG 2/3/04	Town Of CHESTER		
			Highway No. TH 5		
			CHURCH STREET OVER WILLIAMS RIVER		
			PIER DETAILS		
			Designed By J. W. KAMB	Drawn By G. J. WOJCIECHOWSKI	
			Checked By J. TUCKER	Bridge Design Supervisor J. TUCKER	Date 03/01
			PROJECT CHESTER	PROJECT NO. BHO 1442 (32)	
			I.G.C. Info. ...zj365dts.dgn		
			D & K DWG NO. H362 Sheet 19 of 22		

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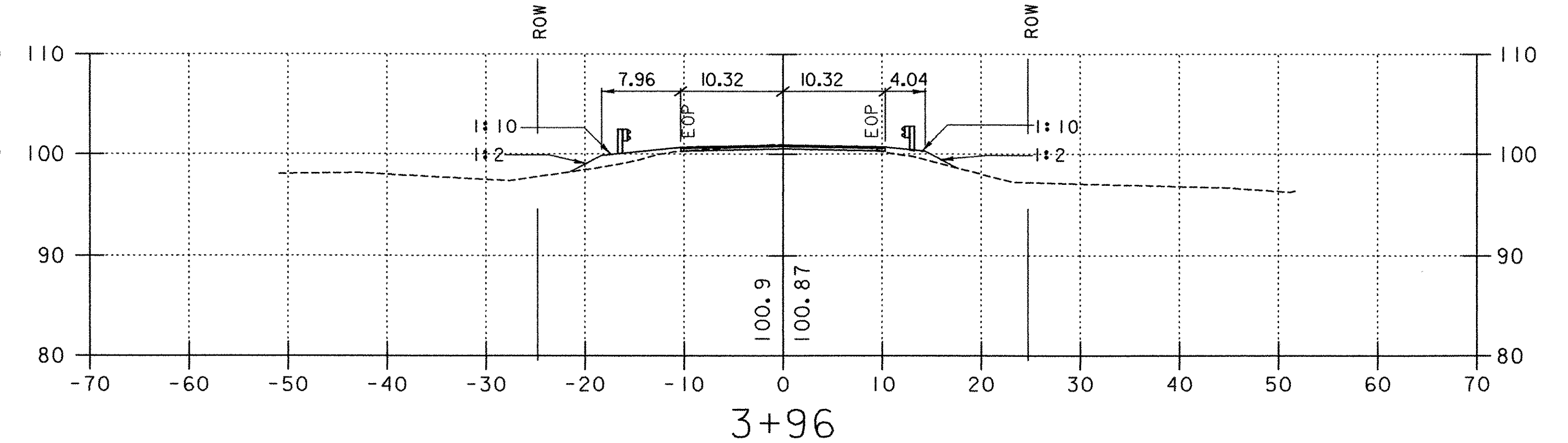
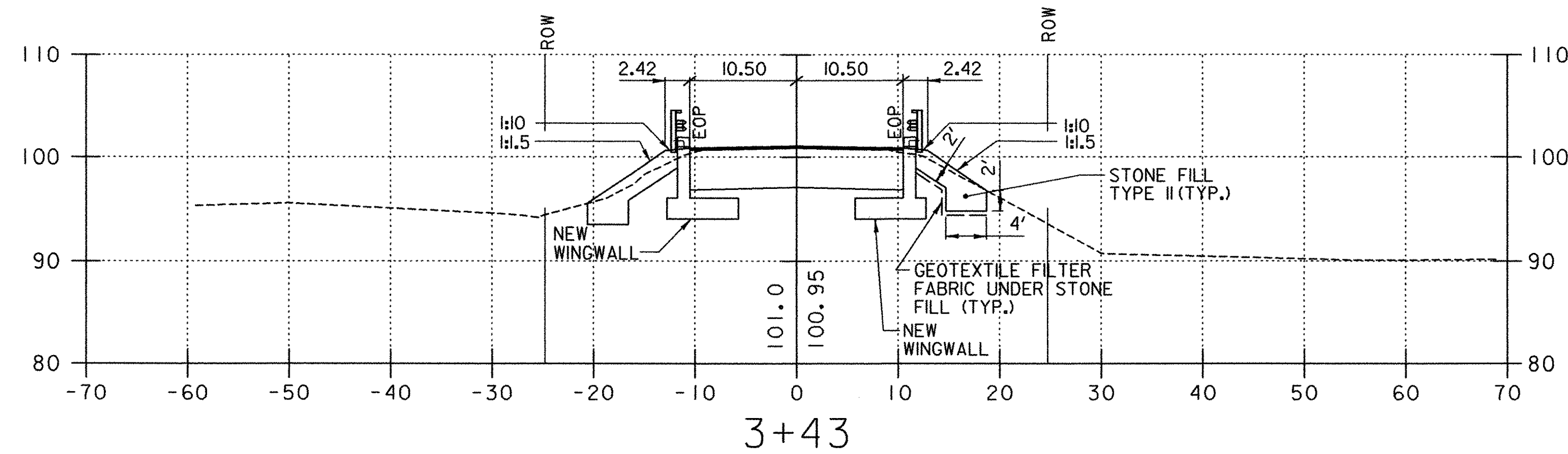
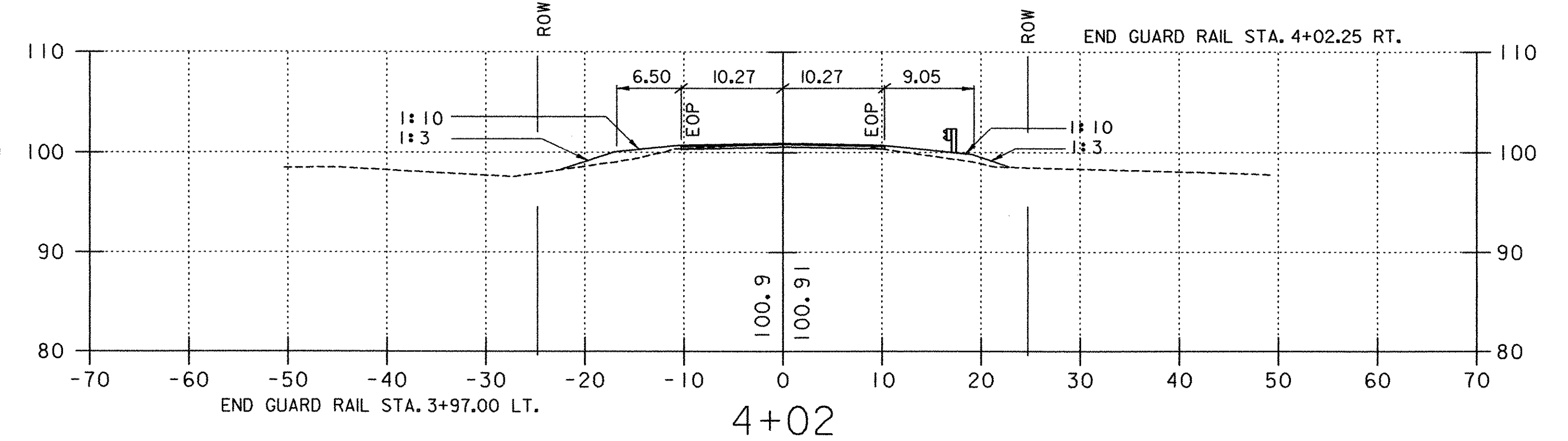
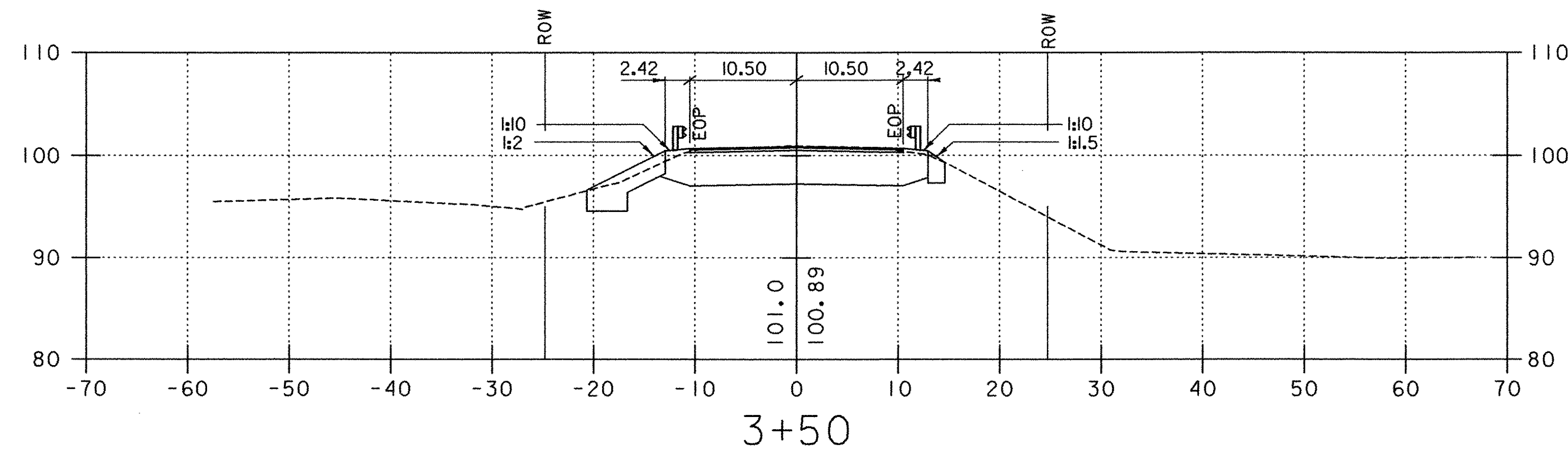
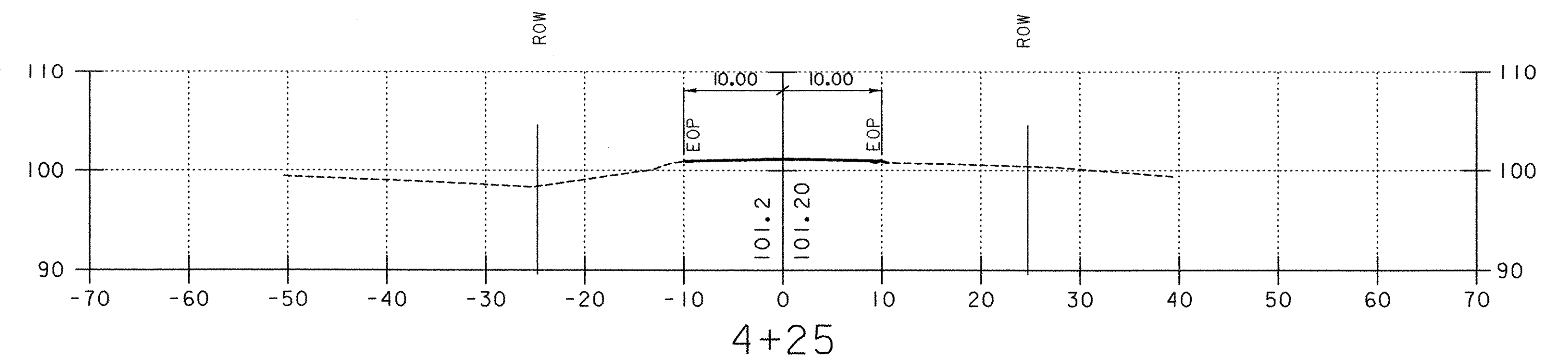
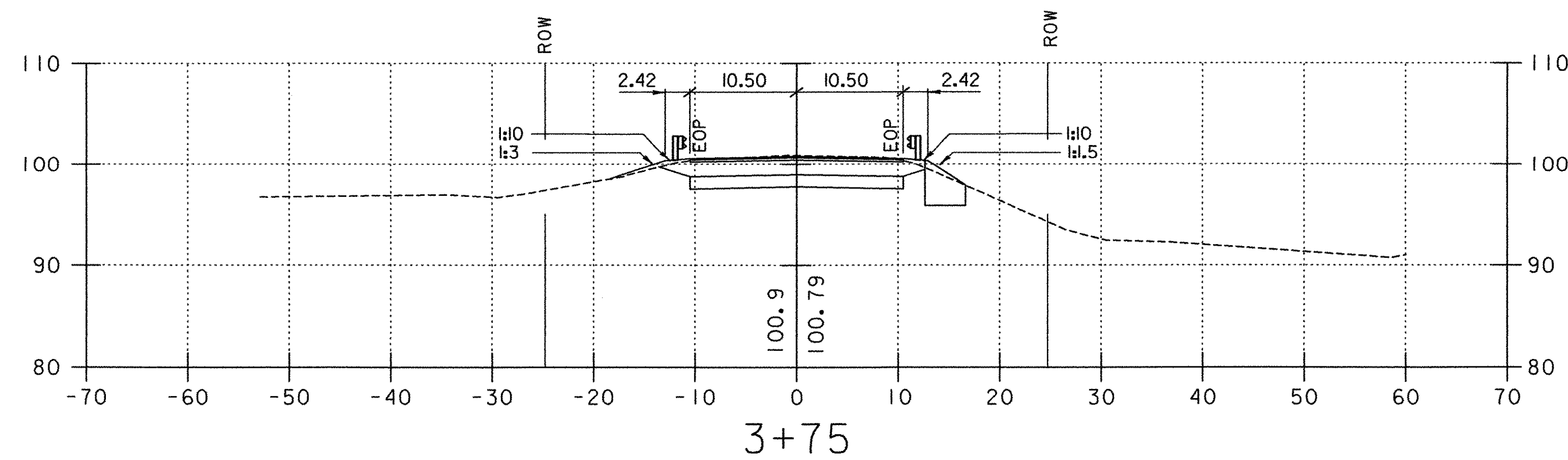
02/10/2004



REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
			Town Of CHESTER	Bridge No. 11
			Highway No. TH 5	Log Sta. -
				Surv. Sta. -
			CHURCH STREET OVER WILLIAMS RIVER	
			ROADWAY CROSS SECTIONS	
			Designed By G. J. WOJCIECHOWSKI	Drawn By G. J. WOJCIECHOWSKI
			Checked By J. TUCKER	Bridge Design Supervisor J. TUCKER
			Date 03/01	Date 03/01
			PROJECT CHESTER	PROJECT NO. BHO 1442 (32)
			I.G.C. Info. ...zj36521.dgn	
			D & K DWG NO. 11364	Sheet 21 of 22

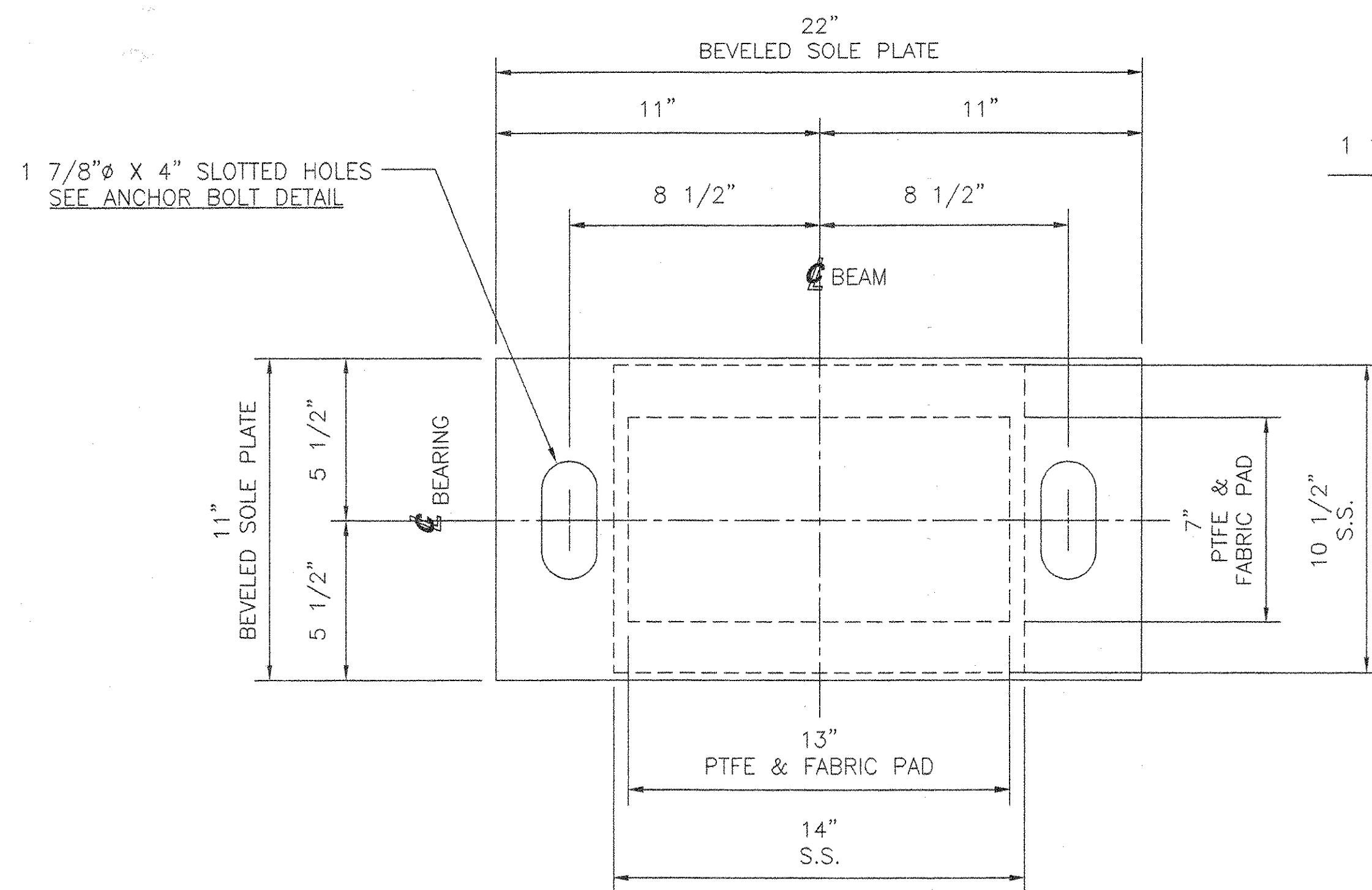


02/03/2004

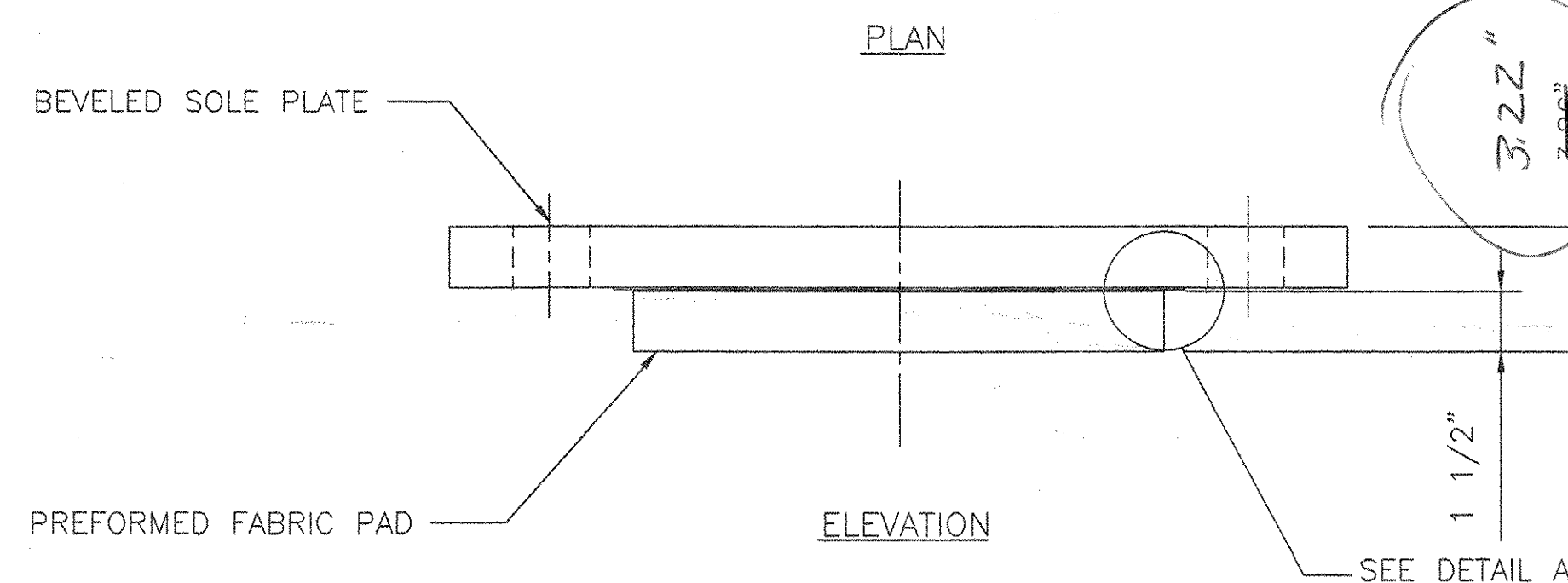


REVISIONS			STATE OF VERMONT AGENCY OF TRANSPORTATION	
NO.	DESCRIPTION	BY & DATE		
 engineering planning management development			Town Of	CHESTER
			Bridge No.	11
			Log Sta.	-
			Surv. Sta.	-
			CHURCH STREET OVER WILLIAMS RIVER	
			ROADWAY CROSS SECTIONS	
			Designed By	G. J. WOJCIECHOWSKI
			Checked By	J. TUCKER
			Date	03/01
			Bridge Design Supervisor	J. TUCKER
			Date	03/01
			PROJECT	CHESTER
			PROJECT NO.	BHO 1442 (32)
			I.G.C. Info.	...zj36521.dgn
			D & K DWG NO.	11365
			Sheet	22 of 22

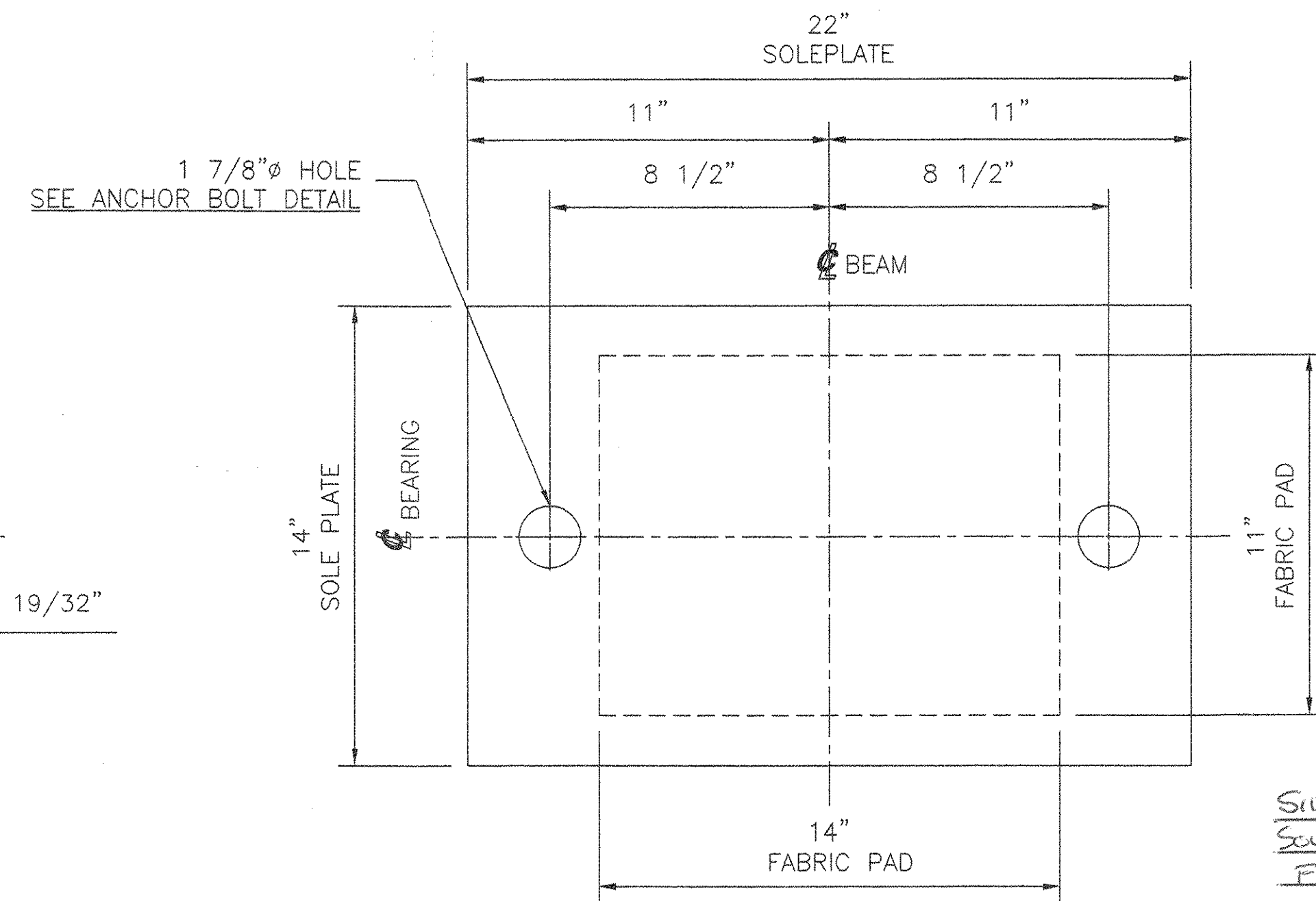
02/03/2004



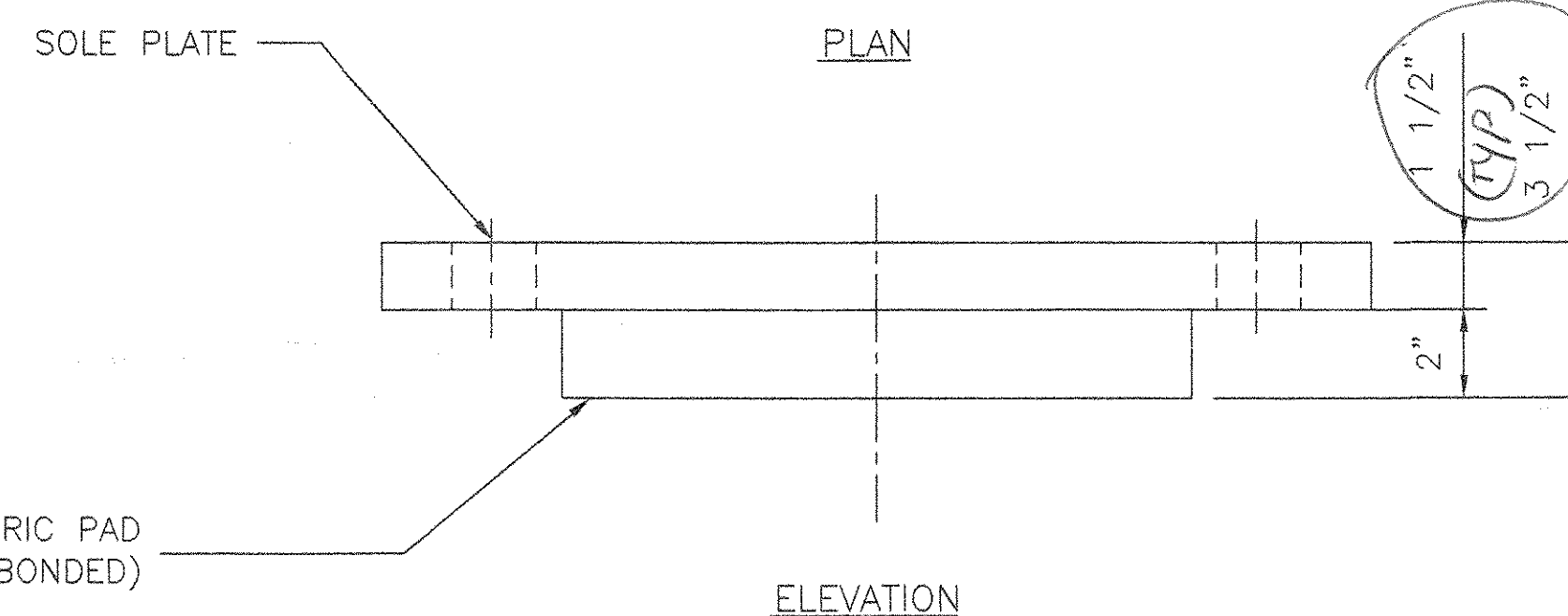
SIDE ELEVATION
SOLE PLATE ONLY
EXPANSION BEARING



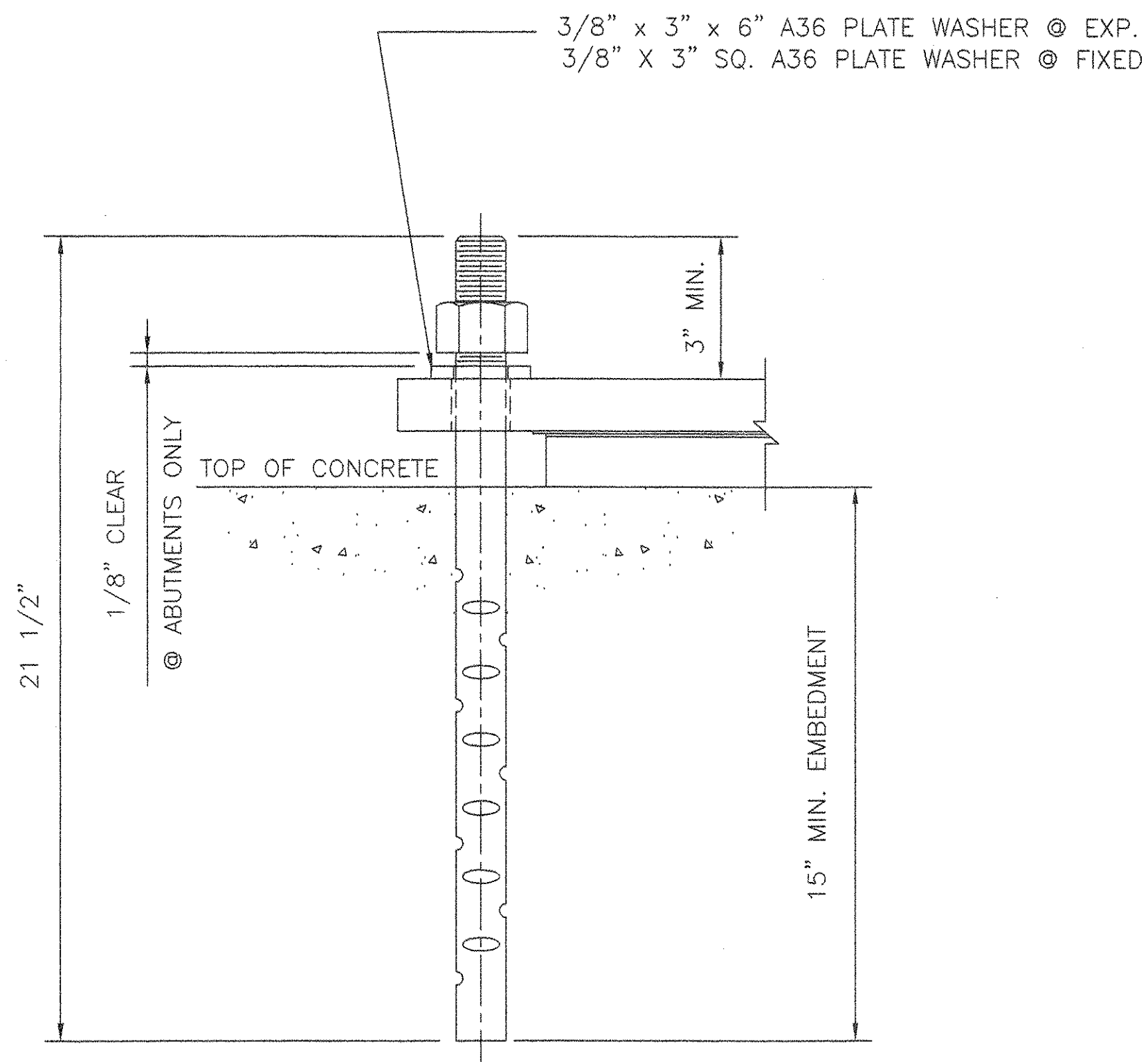
EXPANSION BEARING
LOCATE AT ABUTMENTS 1 AND 2
QTY - 8



SIDE ELEVATION
SOLE PLATE ONLY
FIXED BEARING



FIXED BEARING
LOCATE AT PIER
QTY - 4



ANCHOR BOLT DETAIL

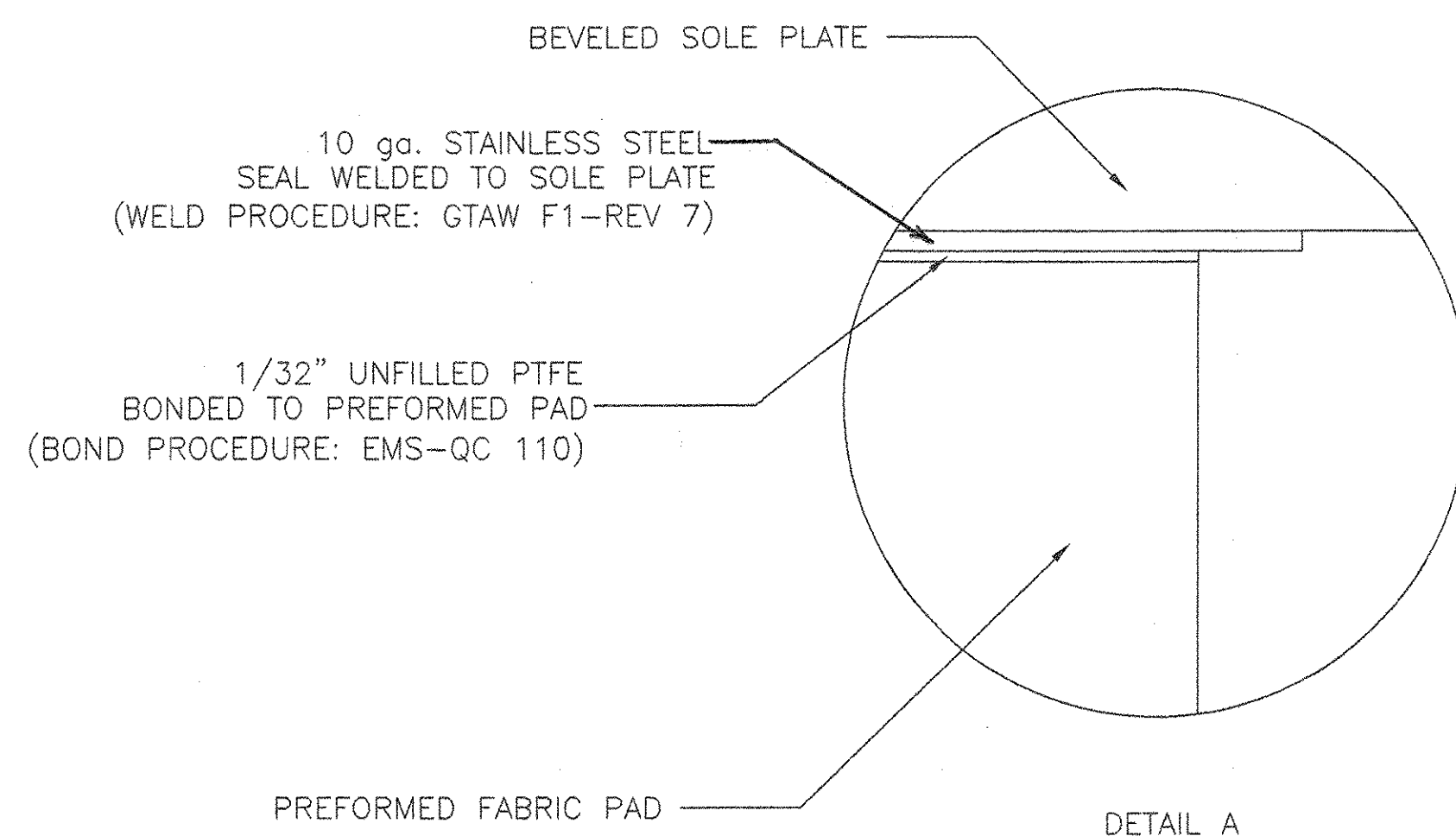
1 1/2" X 21 1/2" LG. ASTM F1554 GR. 55
SWEDGED ANCHOR BOLT W/4" OF THREADS
ONE END, ONE ASTM A563 HEAVY HEX NUT &
ONE A36 PLATE WASHER. ALL GALV.

BEARING DATA AND NOTES

- STEEL - ASTM A709 GR36 ZINC METALLIZED AND PAINTED WITH AN APPROVED SEALER
- PTFE - ASTM D4894 VIRGIN UNFILLED
- STAINLESS STEEL - ASTM A240 TYPE 304 WITH 30 RMS FINISH OR LESS ON SLIDING SURFACE.
- PREFORMED FABRIC PADS - AASHTO 18.4.9.1 DIV. 2
- ANCHOR BOLTS - F1554 GR55 SWEDGED AND GALVANIZED
- BEARINGS SHALL BE SHIPPED AS COMPLETE UNITS AND SHALL BE WRAPPED FOR PROTECTION DURING TRANSIT AND STORAGE.
- BEARINGS SHALL BE MARKED FOR IDENTIFICATION WITH LOCATION.
- COSMEC, INC. REPRESENTATIVE: MR. MATT McANDREWS (508) 668-6600
- ⊗ INDICATES THICK END OF SOLE PLATE
- LOCATION OF FABRICATION: 70 SOUTH ST. WALPOLE, MA 02081
- PREFORMED BEARING PAD DESIGN CRITERIA:
 - ALLOWABLE BEARING PRESSURE ON CONCRETE = 1000 PSI
 - MINIMUM ALLOWABLE DESIGN ROTATION = 0.015 RADIAN
 - HORIZONTAL CAPACITY SHALL BE A MINIMUM OF 6% OF THE VERTICAL LOAD
 - DESIGN LOAD PER BEARING: ABUTMENTS = 90 KIPS & PIER = 150 KIPS

- ALL BEARING DEVICES SHALL BE METALIZED AS PER SUBSECTION 531.04 (B) AND 506.15 (b) OF THE GENERAL SPECIAL PROVISIONS. AREAS OF METALIZING DAMAGED BY FIELD WELDING OR HANDLING SHALL BE REPAIRED IN ACCORDANCE WITH SUPPLEMENTAL SPECIFICATIONS 513 WHICH REFERENCES ASTM A760.
- ALL EDGE CORNERS SHALL BE ROUNDED TO 1/16" RADIUS.
- PROTECT PTFE AND STAINLESS STEEL SURFACES FROM WELD SPATTER, DIRT, OR ANY OTHER FOREIGN MATERIALS.
- BEARINGS MUST NOT BE DISASSEMBLED.

THE CONTRACTOR SHALL VERIFY THAT ALL
ASTM SPECIFICATIONS NOTED ON THESE
PLANS CORRESPOND TO THE APPROPRIATE
AASHTO SPECIFICATIONS REFERENCED
IN THE STANDARD SPECIFICATIONS.



DETAIL A

ACCEPTED	DATE: 6/10/04	BY: [Signature]
ACCEPTED AS CORRECTED		
REJECTED, REVISE AND RESUBMIT		
REJECTED, UNACCEPTABLE		

Checking is only for conformance with the design concept of the Project and compliance with the information given in the Contract Documents. The Engineer assumes no liability for errors or omissions that may be contained herein. The Contractor, by approving and submitting these documents, warrants their accuracy as stipulated on the Contractor's Shop Drawing Stamp.

DUBOIS AND KING

STATE OF VERMONT

AGENCY OF TRANSPORTATION
BRIDGE NO. 11
HIGHWAY TH 5
BRIDGE ON CHURCH STREET
OVER WILLIAMS RIVER

TOWN OF CHESTER
PROJECT NO. BHO 1442 (32)

COSMEC, INC.

70 SOUTH STREET
WALPOLE, MA 02081

SCALE: 1/4" = 1" DRAWN BY: DCW CHECKED BY: PJM
DATE: 05.21.04 DATE: 05.21.04

COSMEC PREFORMED FABRIC PAD BEARINGS

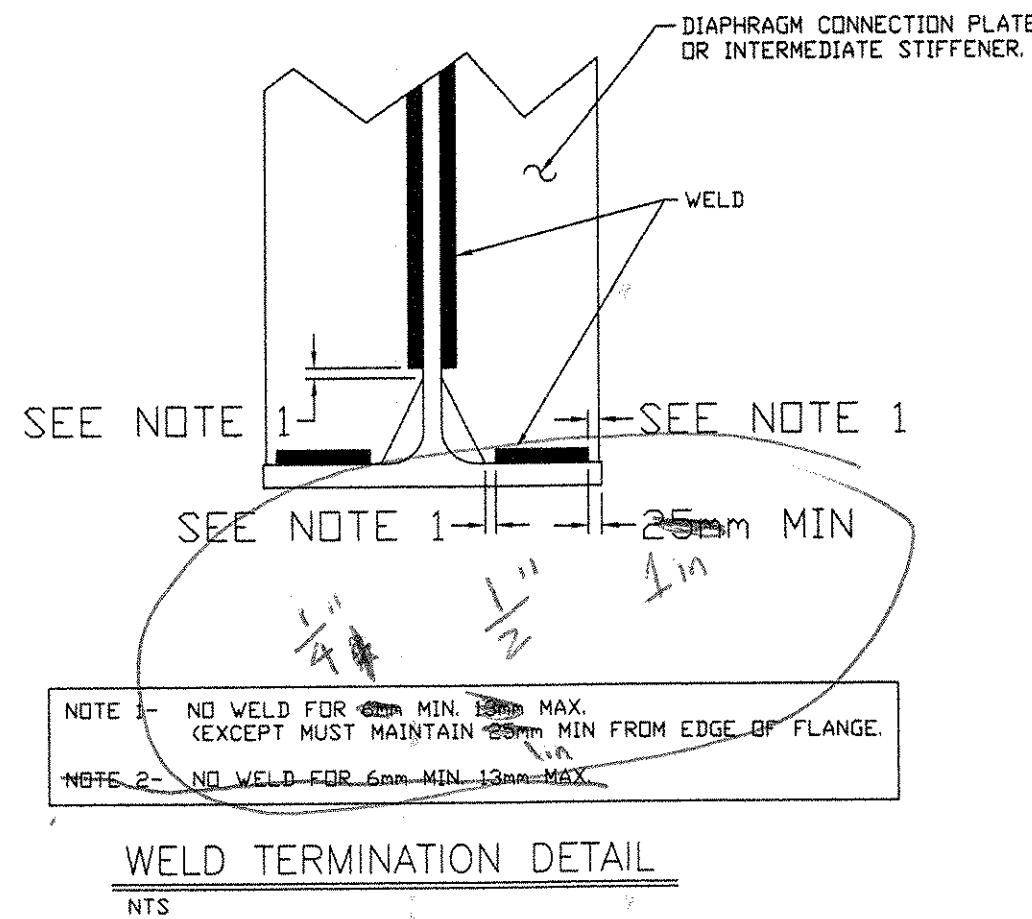
CUSTOMER: F.W. WHITCOMB S.D. NUMBER: 60063 DRAWING NUMBER: 4187 REV.

GENERAL NOTES

17th EDITION
DATED 2002

STRUCTURAL STEEL:

- ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO STATE OF VERMONT AGENCY OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, DATED 2001, AND ITS LATEST REVISIONS, AND THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, AND ITS LATEST REVISIONS, AND AASHTO/AWS D15, 2001.
- ALL STRUCTURAL STEEL SHALL BE FABRICATED USING PROCEDURES AND TOLERANCES IN ACCORDANCE WITH SECTION 506.03 OF THE STANDARD SPECIFICATIONS.
- ALL DIMENSIONS ARE GIVEN AT 68°F UNLESS OTHERWISE NOTED.
- ALL MEMBERS AND/OR PLATES MARKED (CVN) AND ONLY THOSE MEMBERS MUST MEET THE CHARPY V-NOTCH REQUIREMENTS FOR MAIN MEMBERS AS INDICATED IN SECTION 714 OF THE VERMONT AGENCY OF TRANSPORTATION SPECIFICATIONS OF CONSTRUCTION.
- ALL STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270 GRADE 50W.
- ALL FIELD CONNECTIONS SHALL BE MADE USING 7/8 INCH DIAMETER BOLTS MEETING ASTM DESIGNATION A325 TYPE 3 (AASHTO M164). HOLES SHALL BE 15/16 INCH DIAMETER.
- ANY BOLT HOLES IN THE WEB OF THE FASCIA GIRDERS NOT OTHERWISE FILLED, SHALL BE FILLED WITH BUTTON HEAD OR HEXAGONAL HEAD BOLTS COMPATIBLE WITH THE GIRDER STEEL. THE BOLTS SHALL BE TIGHTENED IN ACCORDANCE WITH SUBSECTION 506.19.
- ALL MATERIALS SHALL BE BLAST CLEANED BEFORE OR AFTER FABRICATION. THE FINAL SURFACE APPEARANCE AFTER FABRICATION SHALL MEET THE REQUIREMENTS OF SSPC-SP10.
- THIS IS A UNPAINTED PROJECT.
- [IT] DENOTES HOLES TO BE DRILLED FULL SIZE IN FIELD SPLICE PLATES BY MEANS OF NUMERICALLY CONTROLLED DRILLING EQUIPMENT. NC DRILLED SPLICE PLATES SHALL THEN BE USED AS TEMPLATES TO DRILL FULL SIZE HOLES IN GIRDER WEBS AND FLANGES WHILE ASSEMBLES PER "SA" DRAWINGS.
- FIELD SPLICE MATERIAL SHALL BE BOLTED IN PLACE FOR SHIPMENT.



WELD TERMINATION DETAIL

NTS

BILL OF MATERIAL

REV. NO.	SHIP MARK	PIECE MARK	QTY.	TYPE OF MATERIAL	SIZE OF MATERIAL	GRADE	LENGTH	FINISH	NOTES	WEIGHT

LIST OF ABBREVIATIONS

B	BEAM	MAX	MAXIMUM
BC	BOTTOM CHORD	MIN	MINIMUM
BOB	BOTTOM OF BASE PLATE	ML	MATERIAL LOSS
BOC	BOTTOM OF CONCRETE	MS	MISCELLANEOUS
BOP	BOTTOM OF PLATE	NS	NEAR SIDE
BOS	BOTTOM OF STEEL	NTS	NOT TO SCALE
BR	BRACE	D/ALL	OVERALL
BRG	BEARING	OC	ON CENTERS
CB	BOTH SIDES	OD	OUTSIDE DIAMETER
C	COLUMN	OH	OPPOSITE HAND
C/C	CENTER TO CENTER	OPNG	OPENING
CLR	CLEAR	OPP	OPPOSITE
CLC	CONCRETE	PC	POINT OF CURVE
CONC	CONCRETE	PL	PLATE
CONST	CONSTRUCTION	PR	PORTAL
CONT	CONTINUOUS	PRJ	PROJECTION
CONN	CONNECTION	R	RISER
CTR	CENTER	R.D	RUNNING DIMENSION
CVN	CHARPY V-NOTCH REQUIRED	REF	REFERENCE
D	DEPTH	REQ'D	REQUIRED
DET	DETAIL	REV	REVISION
DIAG	DIAGRAM	S	STAIRS
DIA	DIAMETER	SCH	SCHEDULE
DN	DOWN	SECT	SECTION
DWG	DRAWING	SH	SHEET
EA	EACH	SP	SPACE
EL	ELEVATION	SPA	SPACING
EM	EMBEDDED MATERIAL	SPEC	SPECIFICATION
ENB	ENBEDDED	SQ	SQUARE
EQ	EQUAL	SR	SAG ROD
EXIT	EXISTING	SS	STAINLESS STEEL
F	FRAME	ST	STRINGER
FB	FLOOR BEAM	STIFF	STIFFENER
FCM	FRACTURE CRITICAL MEMBER	STRUCT	STRUCTURAL
FL	FINISH FLOOR	STL	STEEL
FLG	FLANGE	STD	STANDARD
FLR	FLOOR	SW	SIDEWALK BRACKET
FP	FLOOR PLATE	SYMM	SYMMETRICAL
(FS)	FAR SIDE	SWT	SHIPPING WEIGHT
G	GIRT	T	TRUSS
GA	GAUGE	TC	TOP CHORD
GALV	GALVANIZED	THK	THICKNESS
GDSL	GAUGE OUTSTANDING LEG	THD	THREAD
GR	GRATING	T&B	TOP & BOTTOM
GRD	GROUND	TOC	TOP OF CONCRETE
H	HANGER	TOD	TOP OF GRATING
HR	HORIZONTAL	TOS	TOP STEEL
HR	HANDRAIL	TYP	TYPICAL
HS	HIGH-STRENGTH	UND.	UNLESS NOTED OTHERWISE
ID	INSIDE DIAMETER	VERT	VERTICAL
IN	INCHES	VB	VERTICAL BRACING
L	LADDERS	WP	WORKING POINT

APPROVED AS CORRECTED
DATE: 7/14/04
FOR APPROVAL
DATE: 7/22/04
BY: [Signature]

EASTERN BRIDGE LLC
RURAL RTE 2, BOX 302
CLAREMONT, NH 03742
603-542-5802

REVISIONS		
NO.	DATE	DESCRIPTION
1	7/14/04	FOR APPROVAL

ITEM NO: 506.50
FRANK W. WHITCOMB
CONSTRUCTION CORP.
CONTRACT: BHD 1442 (32)
CHURCH STREET (TH 5) OVER WILLIAMS RIVER
BRIDGE NO. 11
CHESTER, VT
TITLE: GENERAL NOTES
DRAWN BY: 4/20/04
DIANA BACON
SHOP ORDER: 5025
DWG. NO.: GN1
BLOCK NO.:
SHEET NO.: 1 OF 1

53 026 55

[illegible][illegible]

V.A.O.T.
RECEIVED
CK'D BY _____ CK'D BY JK
JUL 20 2004
RESUBMIT _____ APPROVED ✓
BY _____ DATE 07-21-04

WELDING PROCEDURES WERE
NOT REVIEWED BY BCA OF
DUBOIS & KING, INC

[illegible]

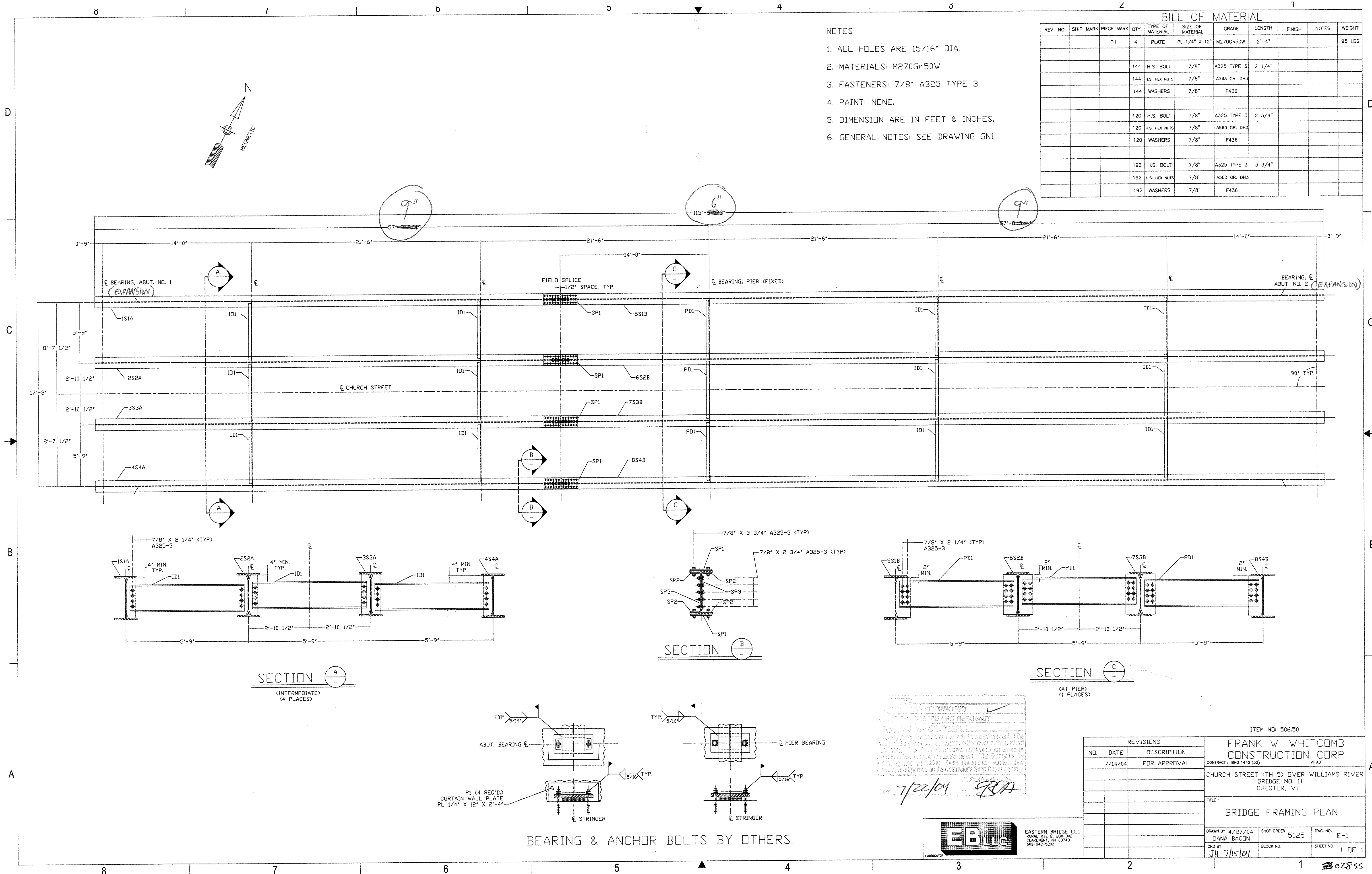
V.A.O.T.
RECEIVED
CK'D BY _____ CK'D BY JWC
JUL 20 2004
RESUBMIT _____ APPROVED ☒
BY _____ DATE 07-21-04

REVISIONS NO. DATE DESCRIPTION 7/14/04 FOR APPROVAL 			ITEM NO: 506.50 FRANK W. WHITCOMB CONSTRUCTION CORP. CONTRACT : BHO 1442 (32) VT ADT CHURCH STREET (TH S) OVER WILLIAMS RIVER BRIDGE NO. 11 CHESTER, VT TITLE : WELD PROCEDURES AND FABRICATION PROCEDURES DRAWN BY 4/20/04 SHOP ORDER DWG. NO. WPS1 DANA BACON 5025 CKD BY BLOCK NO. SHEET NO. 1 OF 1 JH 7/15/04		
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BILL OF MATERIALS
WAS NOT CHECKED BY
THE REVIEWER. TYPICAL ALL SHEETS

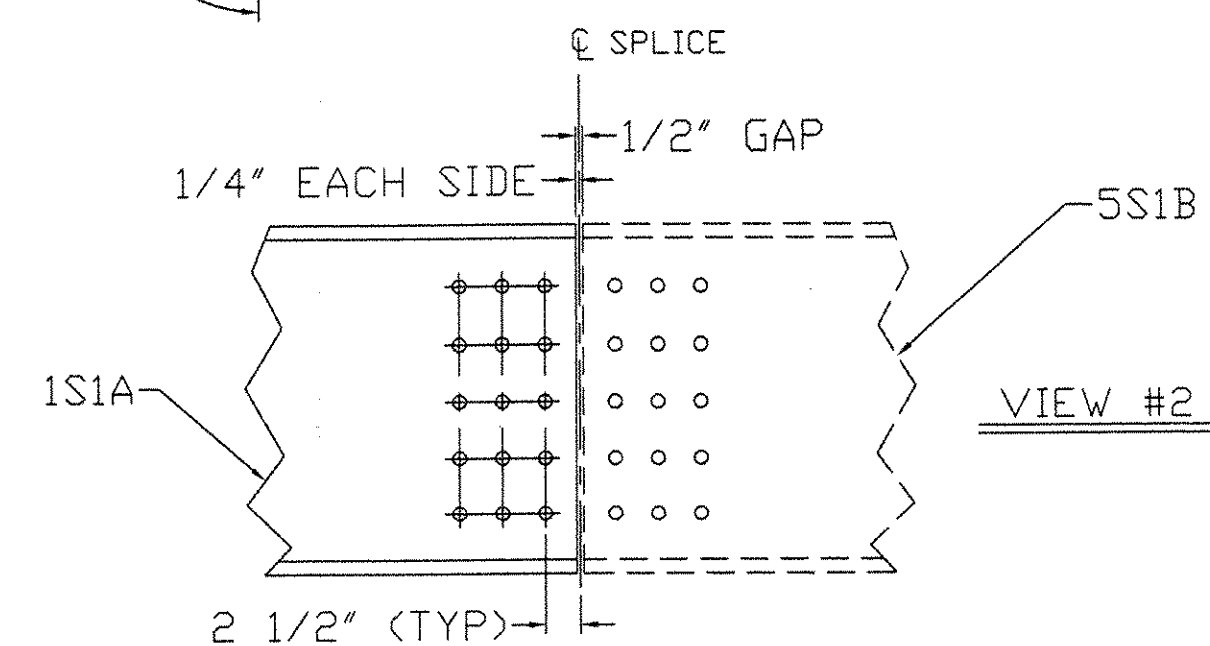
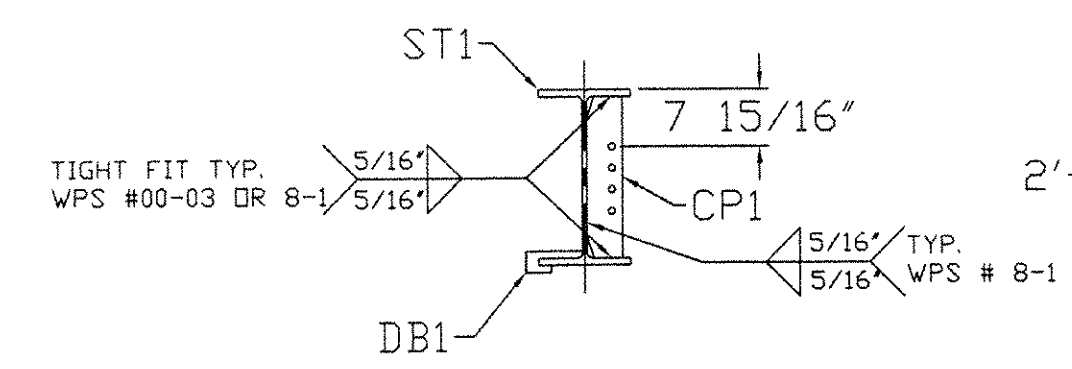
BILL OF MATERIAL										
REV. NO.	SHIP MARK	PIECE MARK	QTY.	TYPE OF MATERIAL	SIZE OF MATERIAL	GRADE	LENGTH	FINISH	NOTES	WEIGHT
		P1	4	PLATE	PL 1/4" X 12"	M270GR50W	2'-4"			95 LB
			144	H.S. BOLT	7/8"	A325 TYPE 3	2 1/4"			
			144	H.S. HEX NUTS	7/8"	A563 GR. DH3				
			144	WASHERS	7/8"	F436				
			120	H.S. BOLT	7/8"	A325 TYPE 3	2 3/4"			
			120	H.S. HEX NUTS	7/8"	A563 GR. DH3				
			120	WASHERS	7/8"	F436				
			192	H.S. BOLT	7/8"	A325 TYPE 3	3 3/4"			
			192	H.S. HEX NUTS	7/8"	A563 GR. DH3				
			192	WASHERS	7/8"	F436				

- NOTES:
1. ALL HOLES ARE 15/16" DIA.
 2. MATERIALS: M270Gr50W
 3. FASTENERS: 7/8" A325 TYPE 3
 4. PAINT: NONE.
 5. DIMENSION ARE IN FEET & INCHES.
 6. GENERAL NOTES: SEE DRAWING GNI



[illegible]

-
- C BRG. ABUT. #1
- REF. LINE
- 0'-9"
- 11'-4 3/4"
- 3/4"
- 1 1/4"
- 3/4"
- 11'-4 3/4"
- 8'-9 1/2" TO THE END OF BEAM
- 0 END OF BEAM
- C FIELD SWICE
- RUNNING DIMENSIONS ARE TAKEN ALONG REF. BASE LINE.
- CAMBER DIAGRAM-CAMBER DIMENSIONS AT RUNNING DIMENSIONS SHOWN.



FABRICATOR

			REVISIONS NO. DATE 7/14/04 DESCRIPTION FOR APPROVAL			FRANK W. WHITCOMB CONSTRUCTION CORP. CONTRACT : BHD 1442 (32) VT AET		
						CHURCH STREET (TH S) OVER WILLIAMS RIVER		
						BRIDGE NO. 11		
						CHESTER, VT		
						TITLE :		
						STRINGER DETAIL 1S1A		
			DRAWN BY 4/23/04		SHOP ORDER	DWG. NO.		
			DANA BACON		5025	1		
			CHK BY JH 7/15/04		BLOCK NO.	SHEET NO.	1 OF 1	

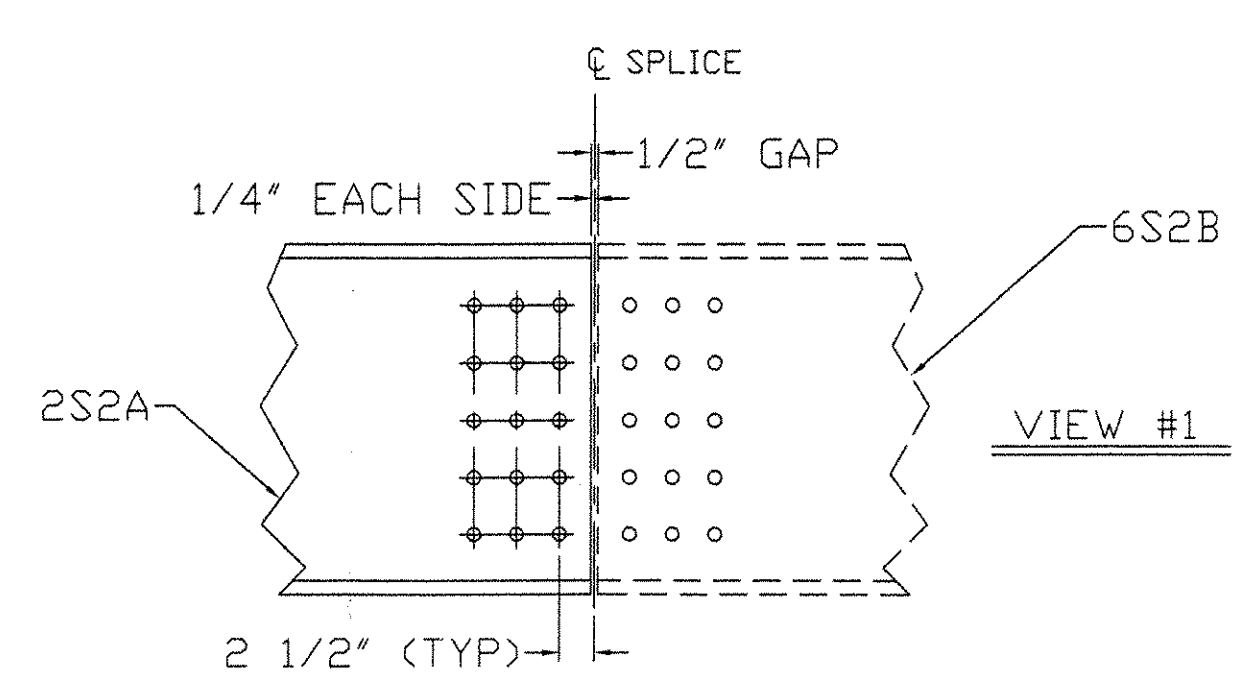
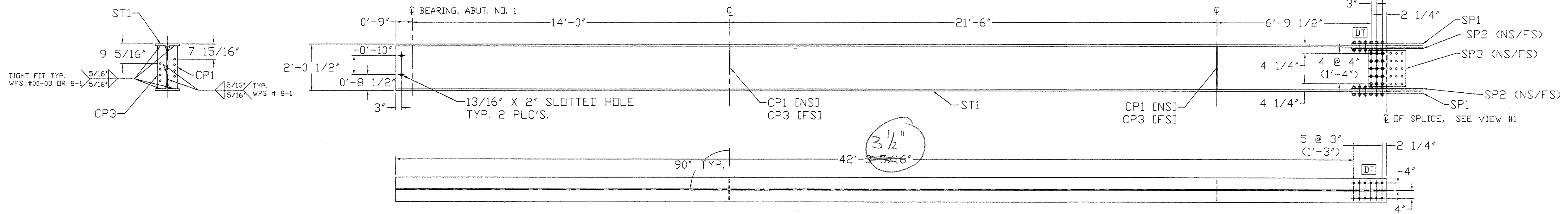
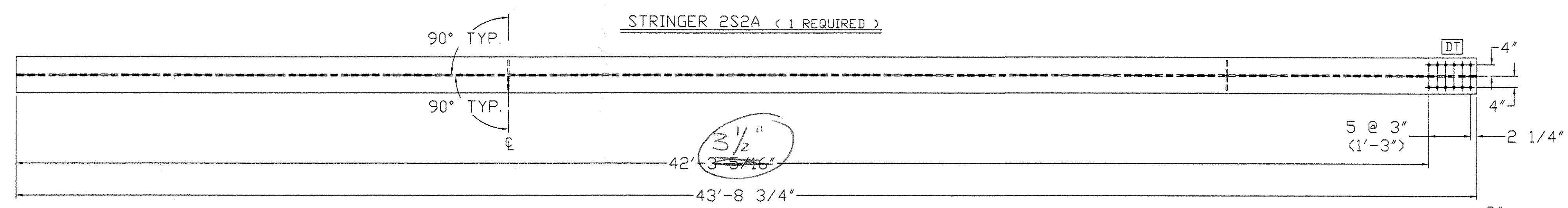
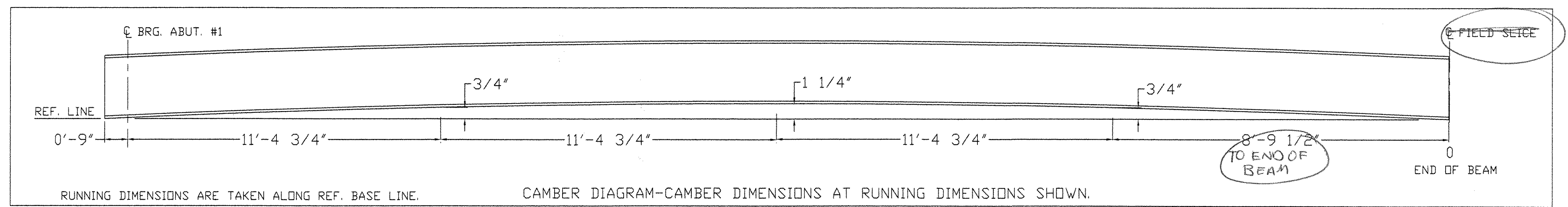
8 7 6 5 4 3 2 1

W24 X 131	
W	2'-0 1/2" X 5/8"
F	1'-0 7/8" X 15/16"

- NOTES:
1. ALL HOLES ARE 15/16" DIA.
 2. MATERIALS: M270G+50W (UN)
 3. FASTENERS: 7/8" A325 TYPE 3
 4. PAINT: NONE.
 5. DIMENSION ARE IN FEET & INCHES.
 6. GENERAL NOTES: SEE DRAWING GNI
 7. ENDS OF BEAMS SHALL BE CUT SQUARE AT ABUTMENTS.
 8. ALL STIFFENERS ARE 90° TO BOTTOM FLANGE.

BILL OF MATERIAL											
REV. NO.	SHIP MARK	PIECE MARK	QTY.	TYPE OF MATERIAL	SIZE OF MATERIAL	GRADE	LENGTH	FINISH	NOTES	WEIGHT	
	2S2A	ST1	1	W	W24 X 131	M270G+50W TZ	43'-8 3/4"		CVN	5729#	
	2S2A	CP1	2	PLATE	PL 1/2" X 5"	M270G+50W	1'-10 5/8"			32#	
	2S2A	CP3	2	PLATE	PL 1/2" X 5"	M270G+50W	1'-10 5/8"			32#	
	2S2A	SP1	2	PLATE	PL 3/4" X 12"	M270G+50W	3'-2"		CVN	194#	
	2S2A	SP2	4	PLATE	PL 3/4" X 5"	M270G+50W	3'-2"		CVN	162#	
	2S2A	SP3	2	PLATE	PL 3/8" X 19"	M270G+50W	1'-8"		CVN	61#	

6230



ACCEPTED
ACCEPTED AS COMPLETED
REVISIONS
REVISION UNACCEPTABLE
7/22/04
DUBOIS AND KERR

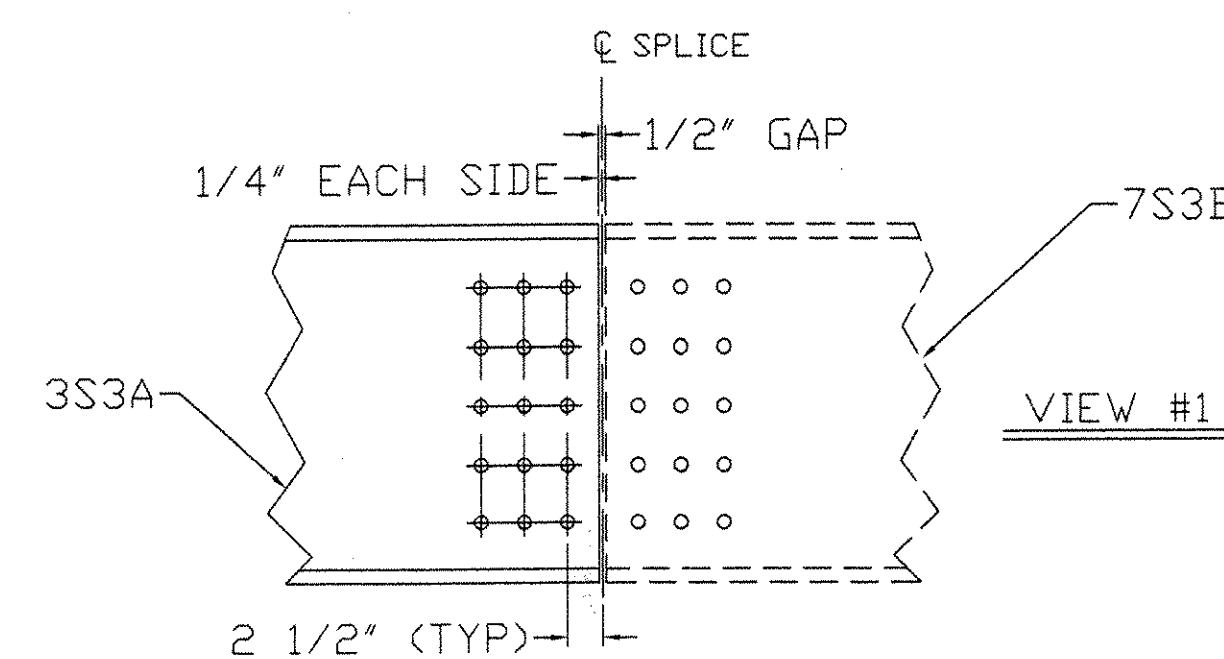
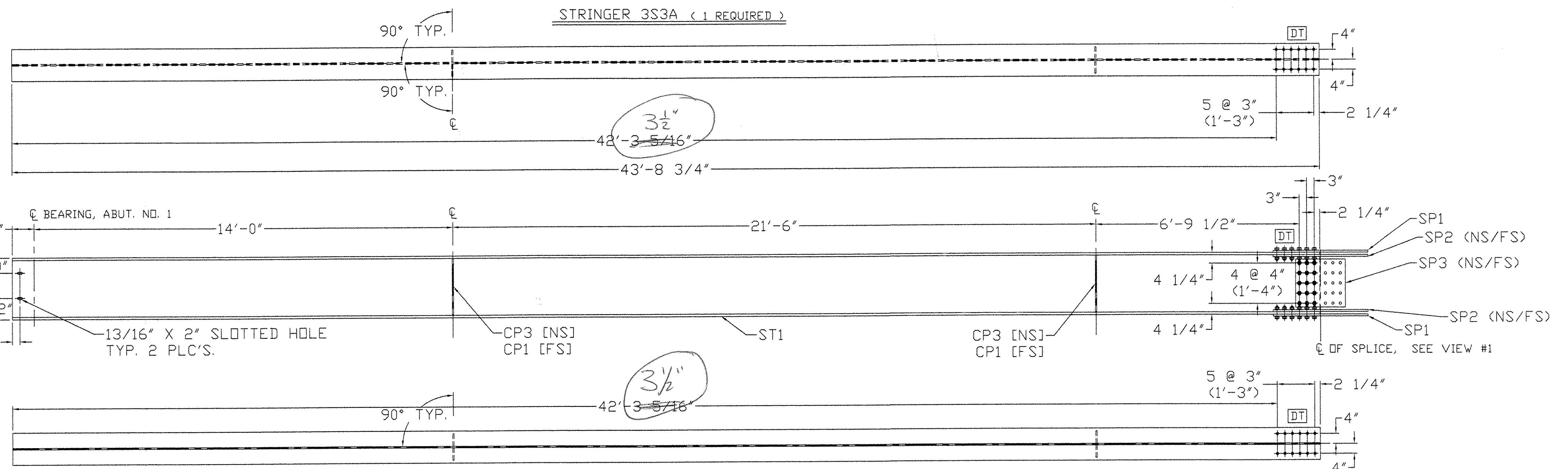
EASTERN BRIDGE LLC
RURAL RTE 8, BOX 208
CLAREMONT, NH 03743
603-542-5802

REVISIONS			ITEM NO: 506.50	
NO.	DATE	DESCRIPTION	FRANK W. WHITCOMB CONSTRUCTION CORP.	
7/14/04		FOR APPROVAL	CONTRACT: BHD 1442 (32) VT ADT	
			CHURCH STREET (TH 5) OVER WILLIAMS RIVER BRIDGE NO. 11 CHESTER, VT	
			TITLE: STRINGER DETAIL 2S2A	
DRAWN BY: 4/23/04		SHOP ORDER: 5025	DWG. NO.: 2	
DANA BACON		BLOCK NO.	SHEET NO.: 1 OF 1	
CKD BY: 7/15/04				

63055

NOTES:

1. ALL HOLES ARE 15/16" DIA.
2. MATERIALS: M270Gr50W (UN)
3. FASTENERS: 7/8" A325 TYPE 3
4. PAINT: NONE.
5. DIMENSION ARE IN FEET & INCHES.
6. GENERAL NOTES: SEE DRAWING GNI
7. ENDS OF BEAMS SHALL BE CUT SQUARE AT ABUTMENTS.
8. ALL STIFFENERS ARE ^{90°} TO BOTTOM FLANGE

[illegible]

STATUS CORRECTED

PROJECTED REVISION AND RESUBMIT

RECEIVED UNACCEPTABLE

checking it only for conformance with the design concept of Project and compliance with the information given in the Contract Documents. The Employer assumes no liability for errors or omissions that may be contained herein. The Contractor, by signing and submitting these documents, warrants that they are prepared as required by the Contractor's Shop Drawing Submittal Procedures.

Contractor's Representative

7/22/04 [Signature]

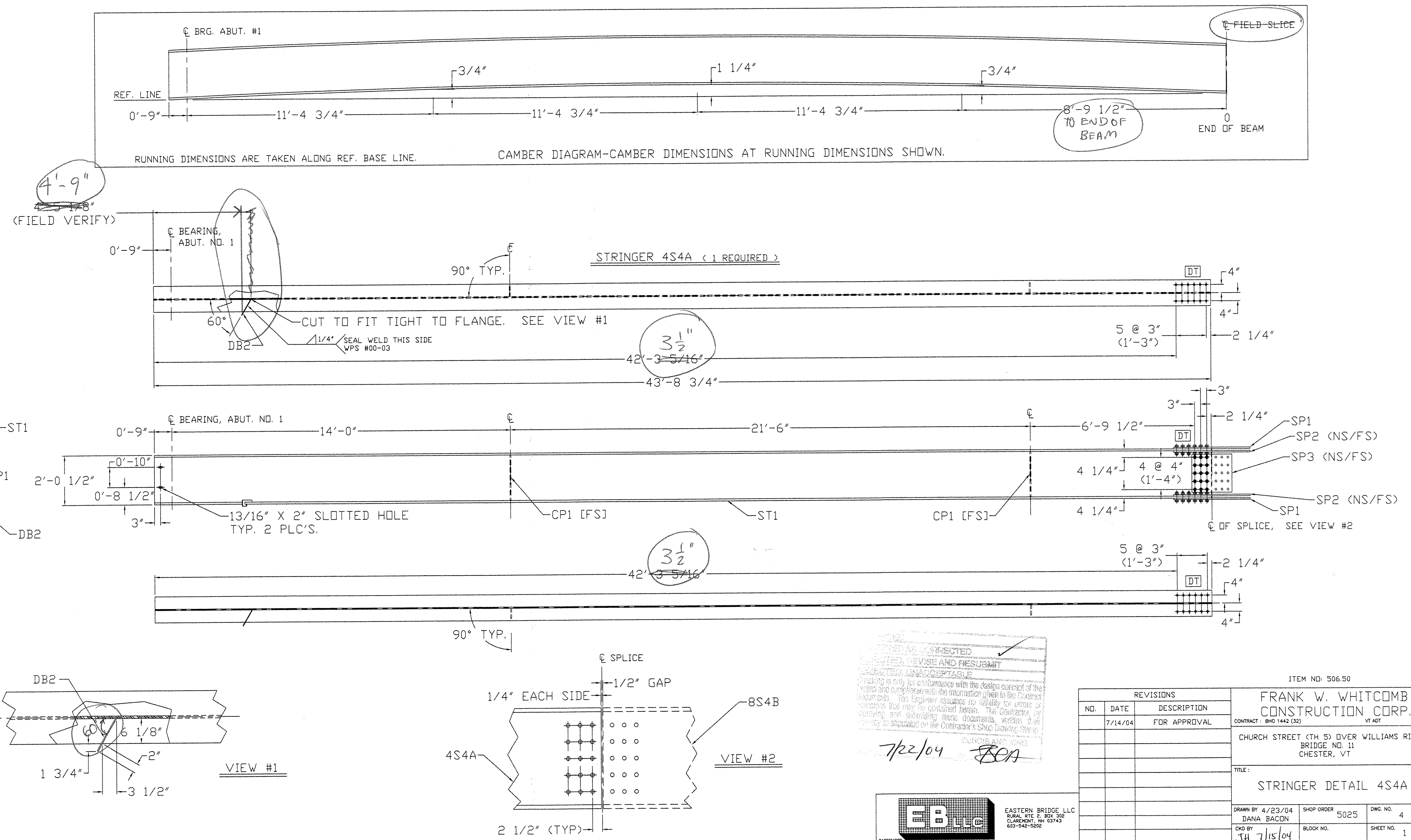
REVISIONS			FRANK W. WHITCOMB CONSTRUCTION CORP.		
NO.	DATE	DESCRIPTION	CONTRACT : BHD 1442 (32) VT A07		
	7/14/04	FOR APPROVAL			
			CHURCH STREET (TH S) OVER WILLIAMS RIVER		
			BRIDGE NO. 11		
			CHESTER, VT		
			TITLE :		
			STRINGER DETAIL 3S3A		
			DRAWN BY 4/23/04 DANA BAGDON	SHOP ORDER 5025	DWG. NO. 3
			CHK'D BY JH 7/15/04	BLOCK NO.	SHEET NO. 1 OF 1

ITEM NO: 506.50	
FRANK W. WHITCOMB CONSTRUCTION CORP.	
TRACT : BHD 1442 (32)	VT ADF
MURCH STREET (TH S) OVER WILLIAMS RIVER BRIDGE NO. 11 CHESTER, VT	
STRINGER DETAIL 3S3A	

EB LLC
EASTERN BRIDGE LLC
RURAL RTE 2, BOX 392
CLAREMONT, NH 03743
603-542-5202

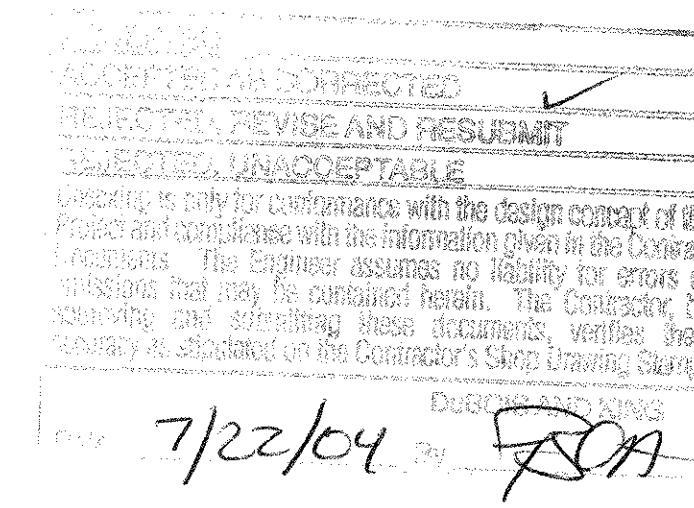
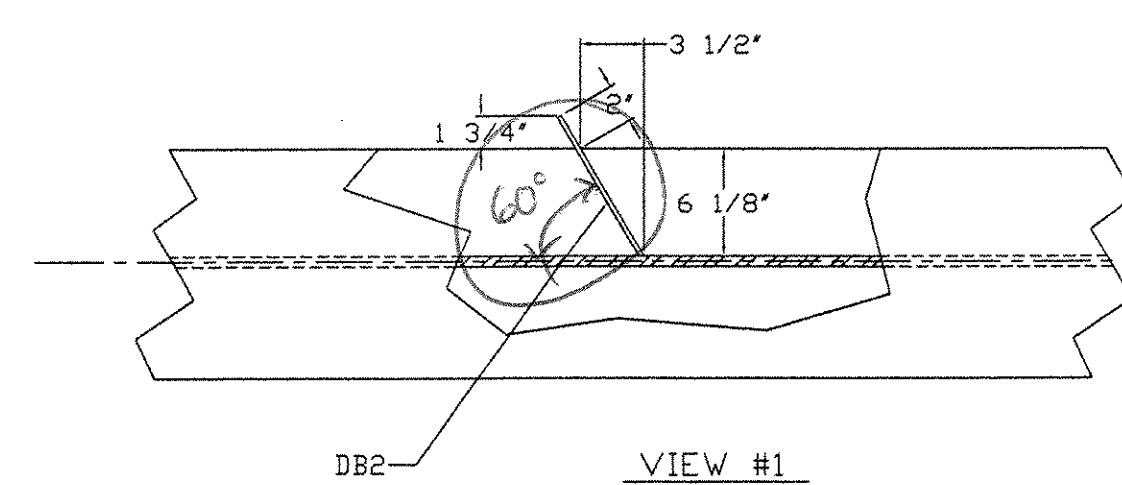
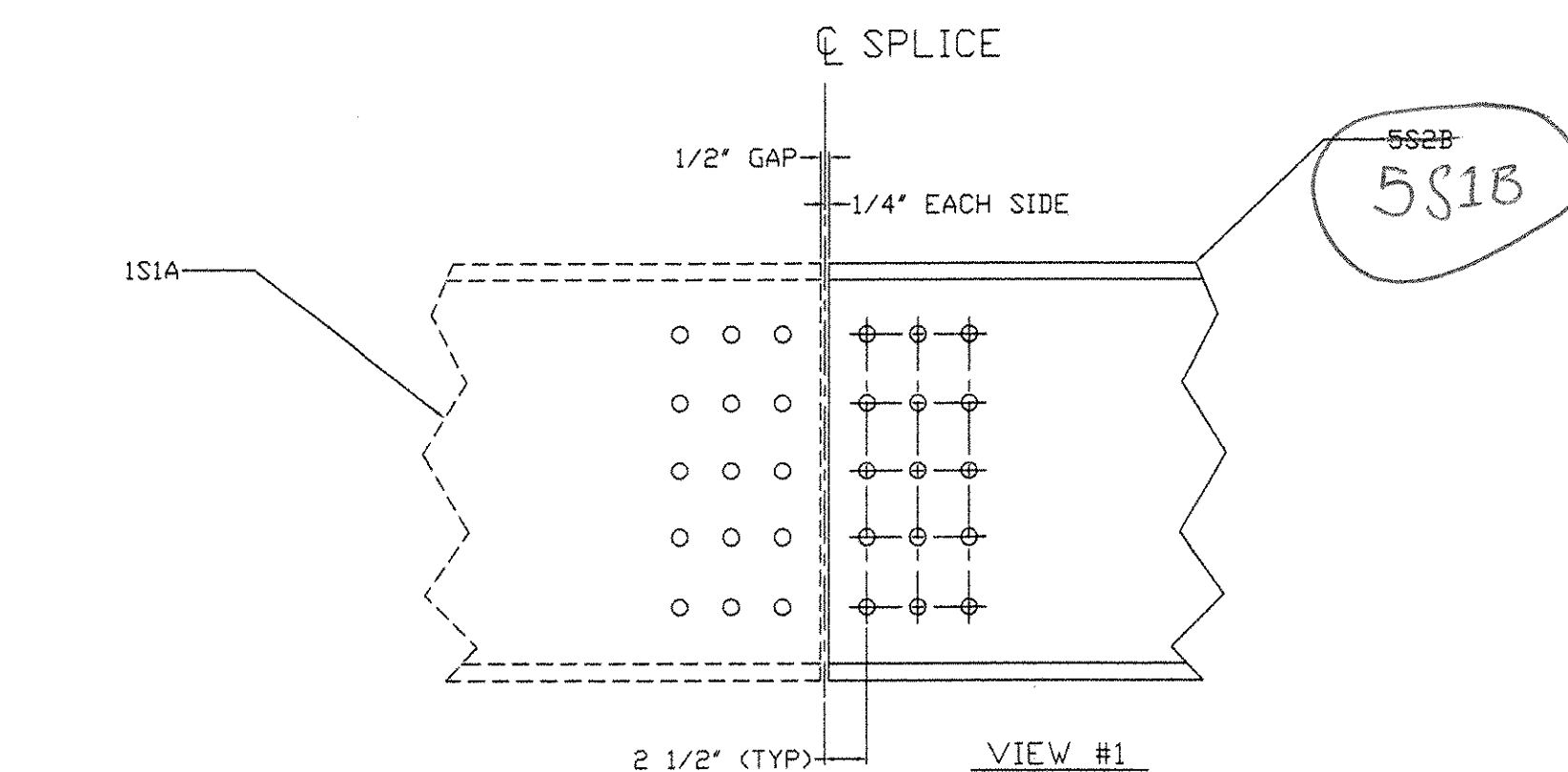
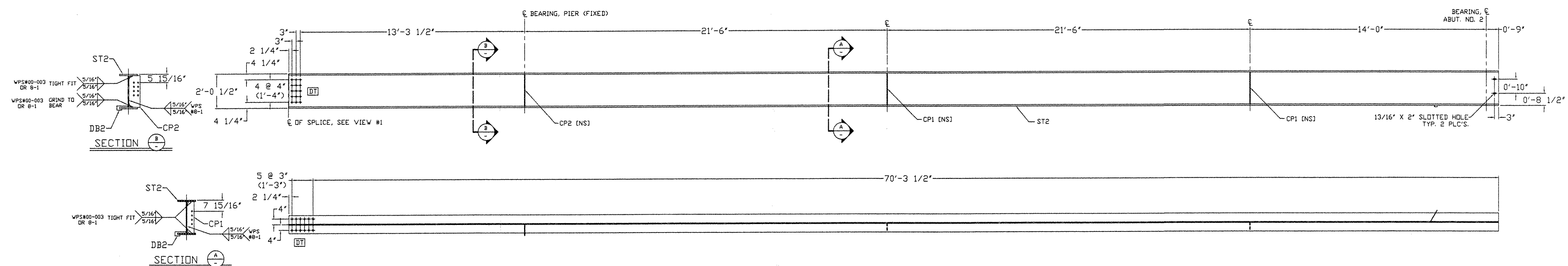
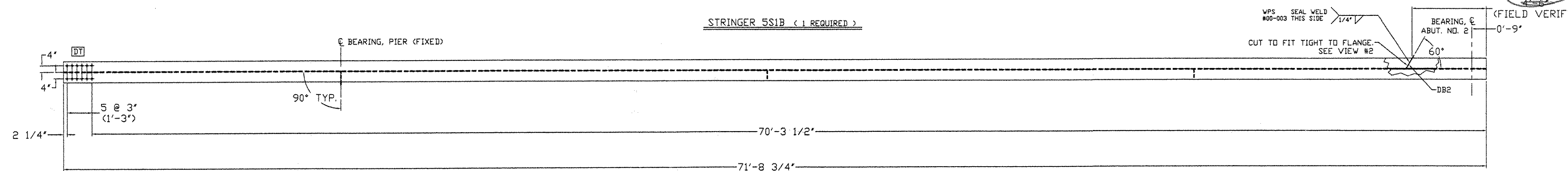
	W24 X 131
W	2'-0 1/2" X 5/8"
F	1'-0 7/8" X 15/16"

- NOTES:
1. ALL HOLES ARE 15/16" DIA.
 2. MATERIALS: M270Gr50W <UN>
 3. FASTENERS: 7/8" A325 TYPE 3
 4. PAINT: NONE.
 5. DIMENSION ARE IN FEET & INCHES.
 6. GENERAL NOTES: SEE DRAWING GN1
 7. ENDS OF BEAMS SHALL BE CUT SQUARE AT ABUTMENTS.
 8. ALL STIFFENERS ARE 90° TO BOTTOM FLANGE.

[illegible]

9457

- | | |
|-----------|--------------------|
| W24 X 131 | |
| W | 2'-0 1/2" X 5/8" |
| F | 1'-0 7/8" X 15/16" |



REVISIONS NO. DATE DESCRIPTION			FRANK W. WHITCOMB CONSTRUCTION CORP. CONTRACT: BHD 1442 (32) VT ADT		
	7/14/04	FOR APPROVAL	CHURCH STREET (TH 5) OVER WILLIAMS RIVER BRIDGE NO. 11 CHESTER, VT		
			TITLE:		
			STRINGER DETAIL 5S1B		
			DRAWN BY 4/26/04 DANA BACON	SHOP ORDER 5025	DWS. NO. 5
			CND BY TH 7/15/04	BLOCK NO.	SHEET NO. 1 OF 1

EB LLC

EASTERN BRIDGE LLC
RURAL RTE 2, BOX 302
CLAREMONT, NH 03743
603-542-5202

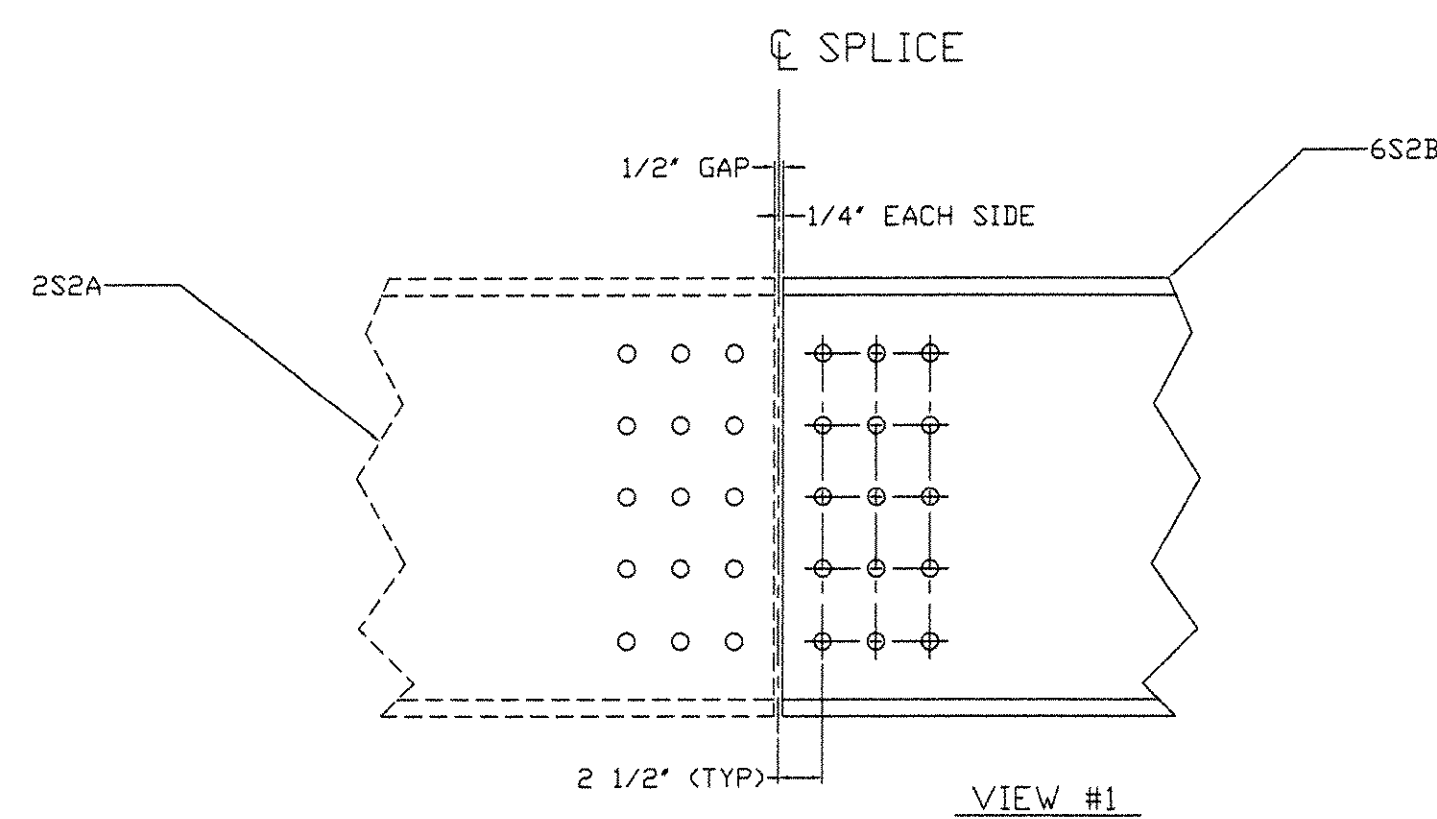
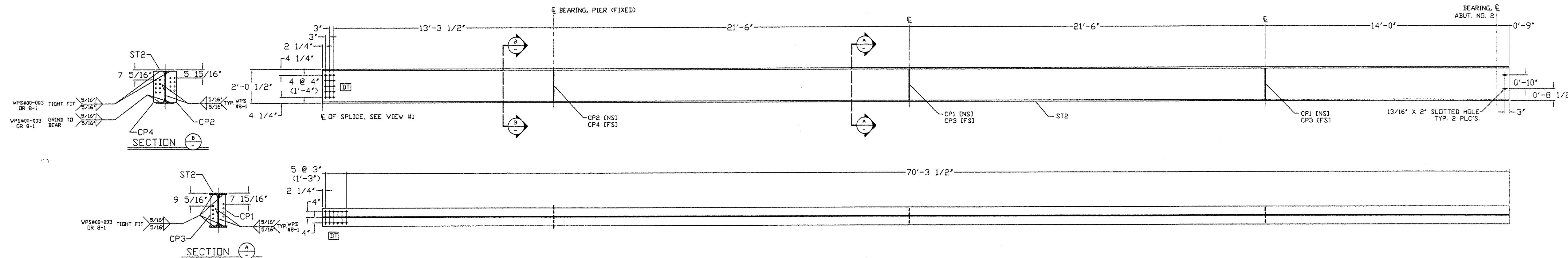
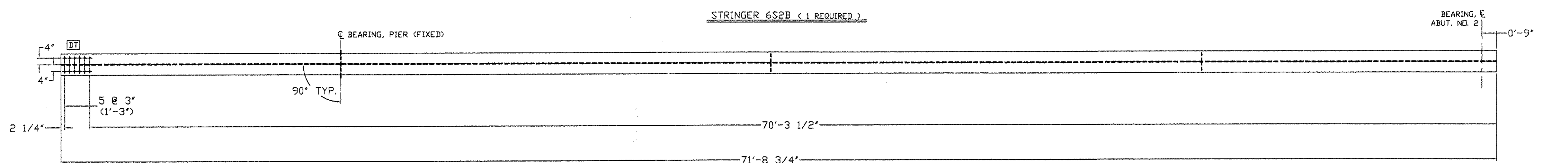
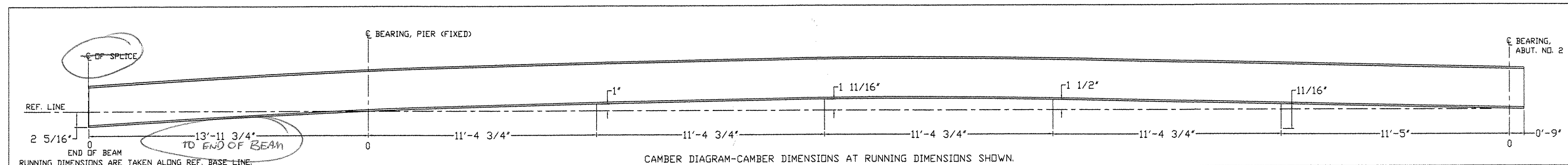
W24 X 131
W 2'-0 1/2" X 5/8"
F 1'-0 7/8" X 15/16"

- NOTES:
1. ALL HOLES ARE 15/16" DIA.
 2. MATERIALS: M270GR50W (UN)
 3. FASTENERS: 7/8" A325 TYPE 3
 4. PAINT: NONE.
 5. DIMENSION ARE IN FEET & INCHES.
 6. GENERAL NOTES: SEE DRAWING G01
 7. ENDS OF BEAMS SHALL BE CUT SQUARE AT ABUTMENTS.
 8. ALL STIFFENERS ARE 90° TO BOTTOM FLANGE.

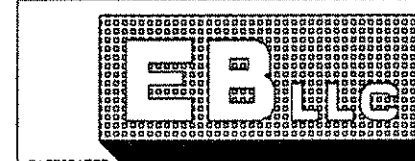
BILL OF MATERIAL

REV. NO.	SHIP MARK	PIECE MARK	QTY.	TYPE OF MATERIAL	SIZE OF MATERIAL	GRADE	LENGTH	FINISH	NOTES	WEIGHT
	6S2B	ST2	1	W	W24 X 131	M270GR50W T2	71'-8 3/4"		CVN	9397#
	6S2B	CP1	2	PLATE	PL 1/2" X 5"	M270GR50W	1'-10 5/8"			32#
	6S2B	CP2	1	PLATE	PL 1/2" X 8"	M270GR50W	1'-10 5/8"			26#
	6S2B	CP3	2	PLATE	PL 1/2" X 5"	M270GR50W	1'-10 5/8"			32#
	6S2B	CP4	1	PLATE	PL 1/2" X 8"	M270GR50W	1'-10 5/8"			26#

9513



7/22/04 RCP



EASTERN BRIDGE LLC
RURAL RTE 2, BOX 202
CLANDON, WI 53743
608-342-5806

REVISIONS		
NO.	DATE	DESCRIPTION
1	7/14/04	FOR APPROVAL

ITEM NO: 506.50

FRANK W. WHITCOMB
CONSTRUCTION CORP.

CONTRACT: BHO 1442 (32) VT ADT

CHURCH STREET (TH 5) OVER WILLIAMS RIVER
BRIDGE NO. 11
CHESTER, VT

TITLE:
STRINGER DETAIL 6S2B

DRAWN BY: 4/26/04 SHOP ORDER: 5025 DWG. NO.: 6

DANA BACON

CHK BY: JH 7/15/04 BLOCK NO.: SHEET NO.: 1 OF 1

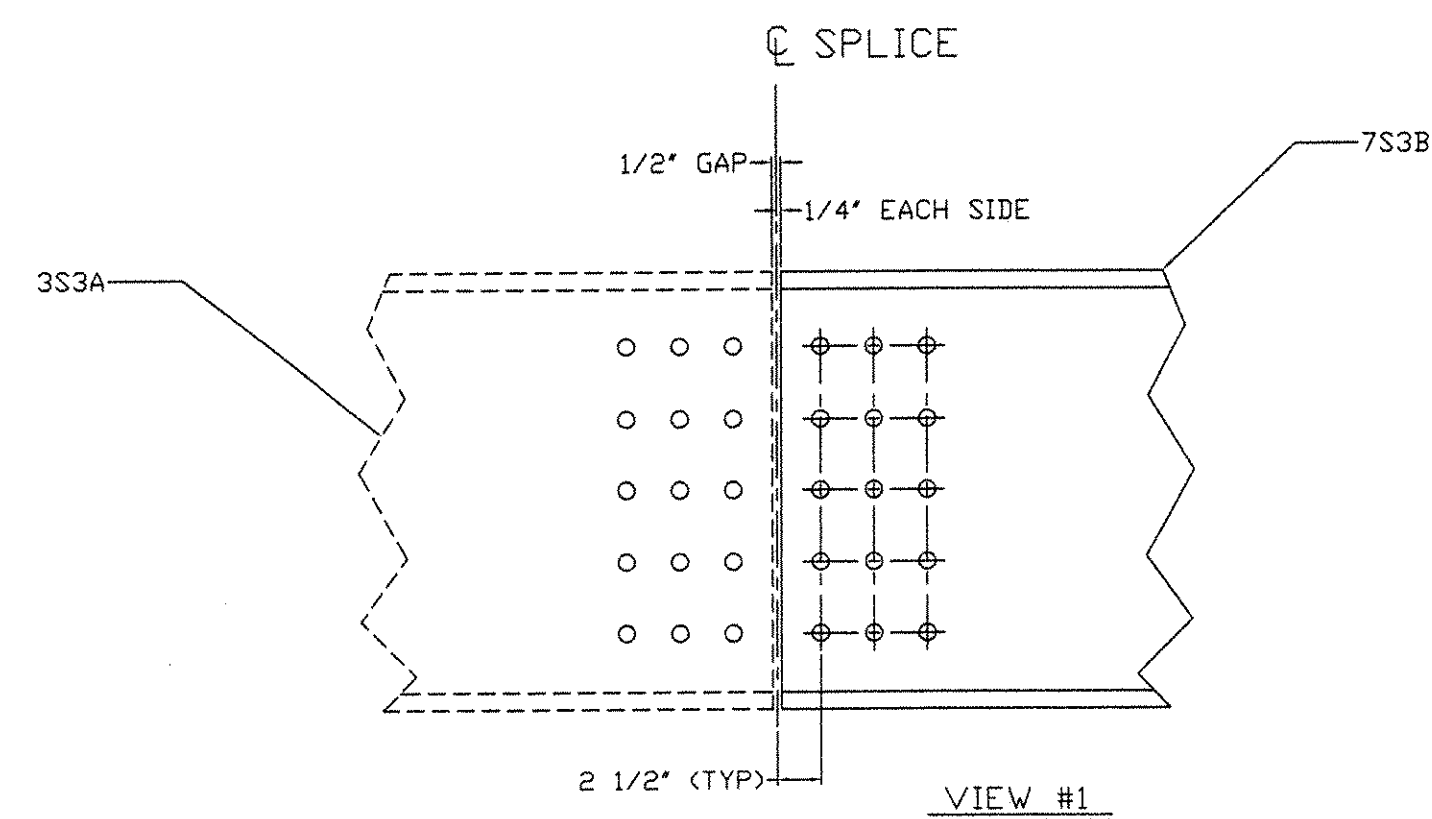
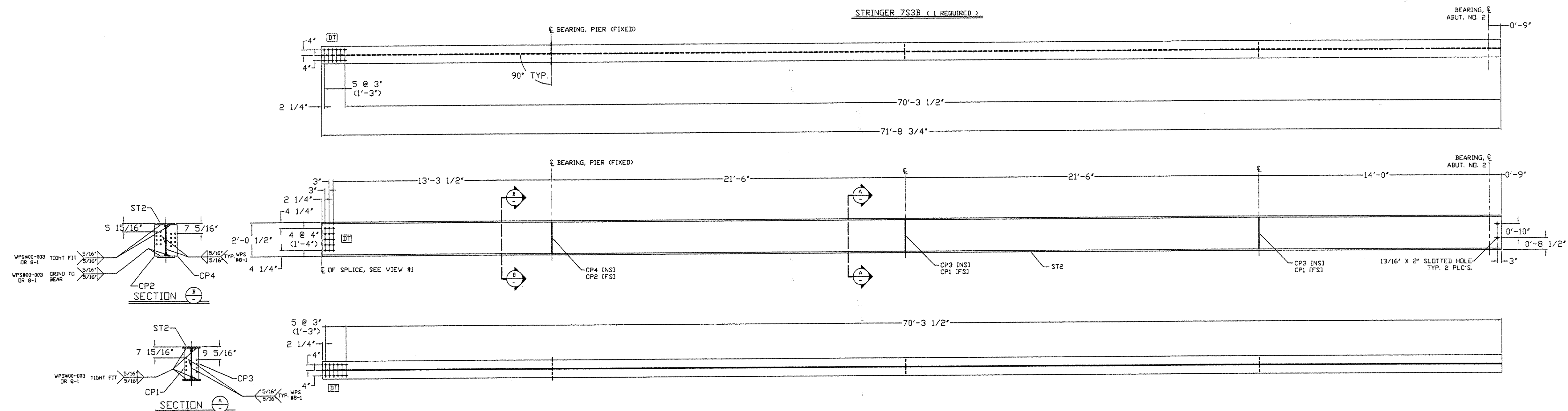
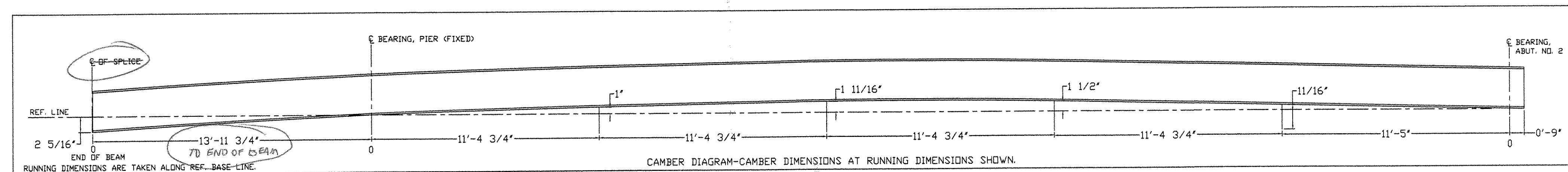
03455

W24 X 131
W 2'-0 1/2" X 5/8"
F 1'-0 7/8" X 15/16"

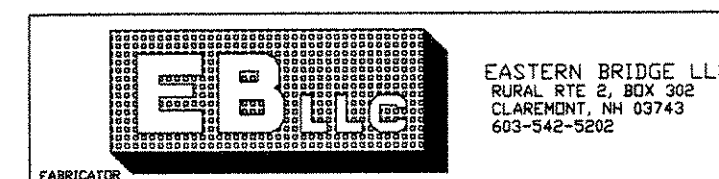
- NOTES:
1. ALL HOLES ARE 15/16" DIA.
 2. MATERIALS: M270GR50W (UND)
 3. FASTENERS: 7/8" A325 TYPE 3
 4. PAINT: NONE.
 5. DIMENSION ARE IN FEET & INCHES.
 6. GENERAL NOTES: SEE DRAWING GNI
 7. ENDS OF BEAMS SHALL BE CUT SQUARE AT ABUTMENTS.
 8. ALL STIFFENERS ARE 90° TO BOTTOM FLANGE.

BILL OF MATERIAL

REV. NO.	SHIP MARK	PIECE MARK	QTY.	TYPE OF MATERIAL	SIZE OF MATERIAL	GRADE	LENGTH	FINISH	NOTES	WEIGHT
	7S3B	ST2	1	W	W24 X 131	M270GR50W TZ	71'-8 3/4"		CVN	9397#
	7S3B	CP1	2	PLATE	PL 1/2" X 5"	M270GR50W	1'-10 5/8"			32#
	7S3B	CP2	1	PLATE	PL 1/2" X 8"	M270GR50W	1'-10 5/8"			26#
	7S3B	CP3	2	PLATE	PL 1/2" X 5"	M270GR50W	1'-10 5/8"			32#
	7S3B	CP4	1	PLATE	PL 1/2" X 8"	M270GR50W	1'-10 5/8"			26#



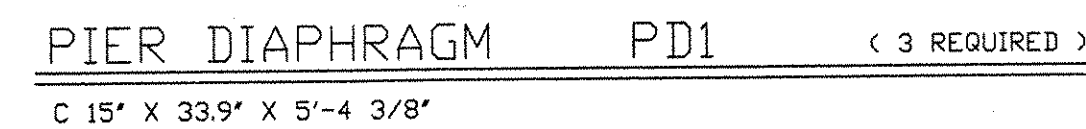
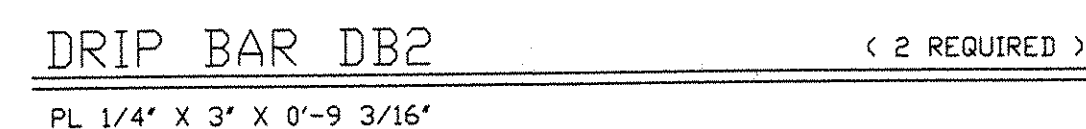
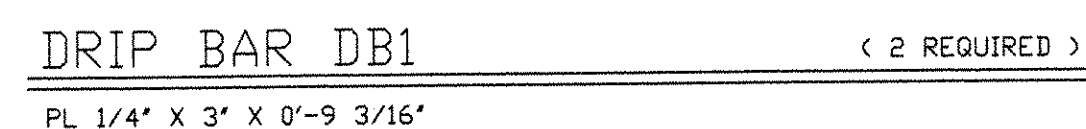
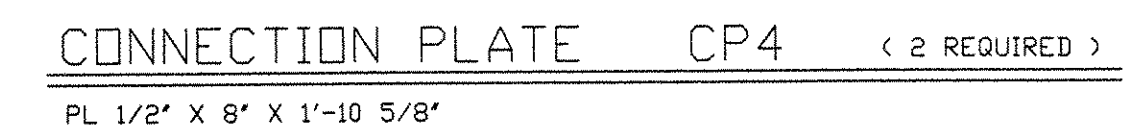
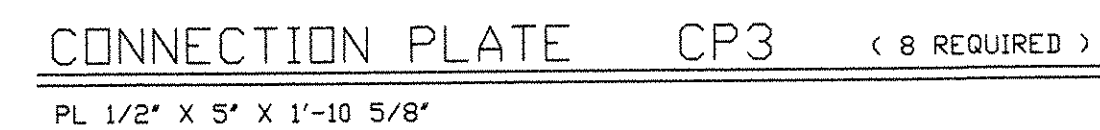
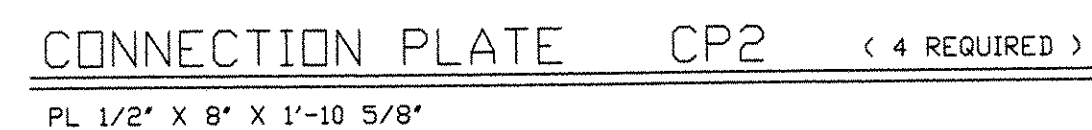
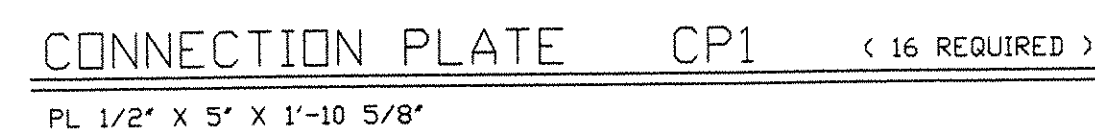
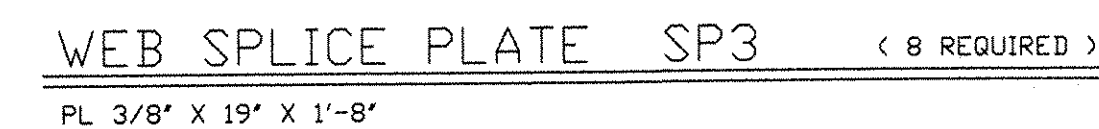
7/22/04 788A

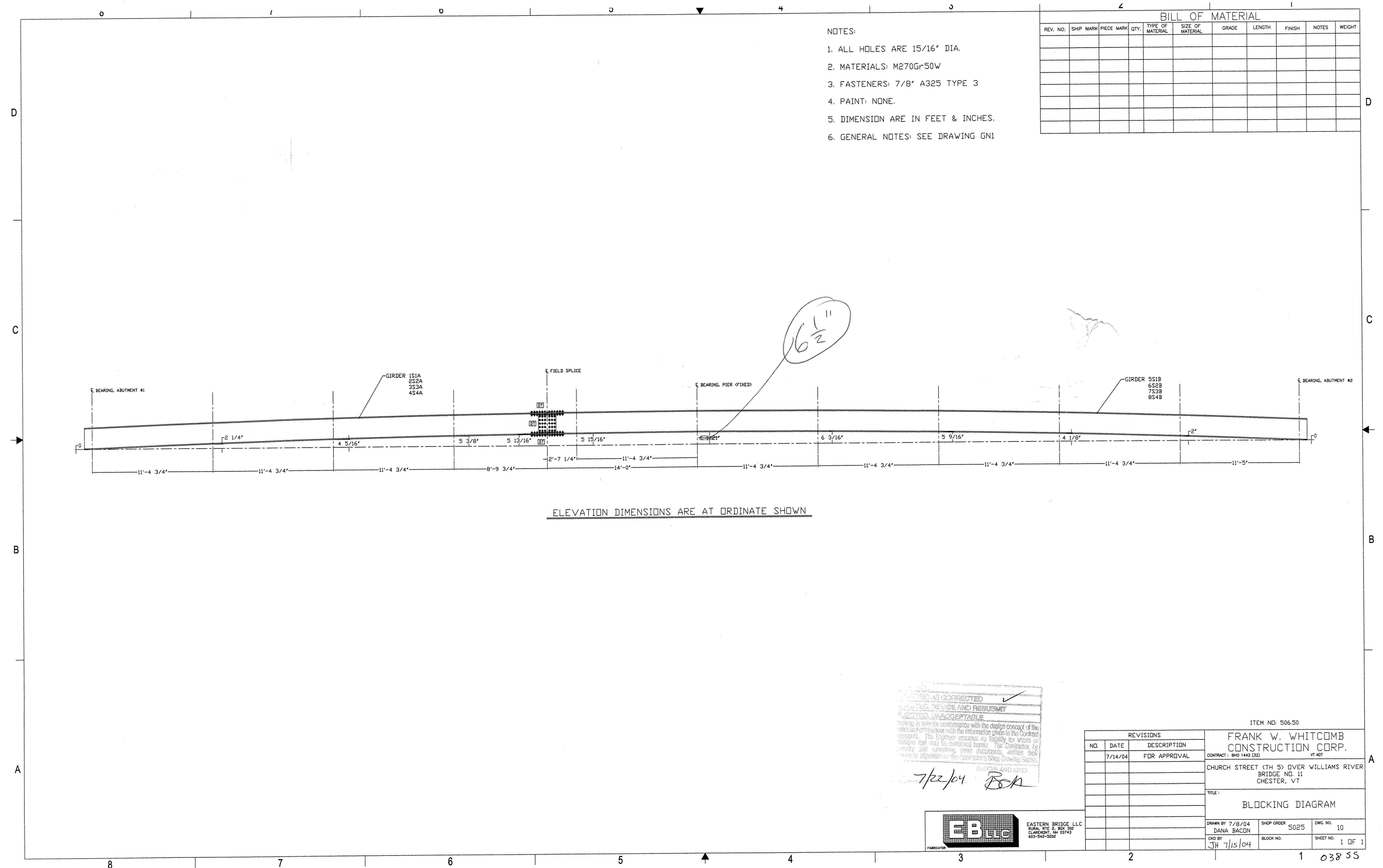


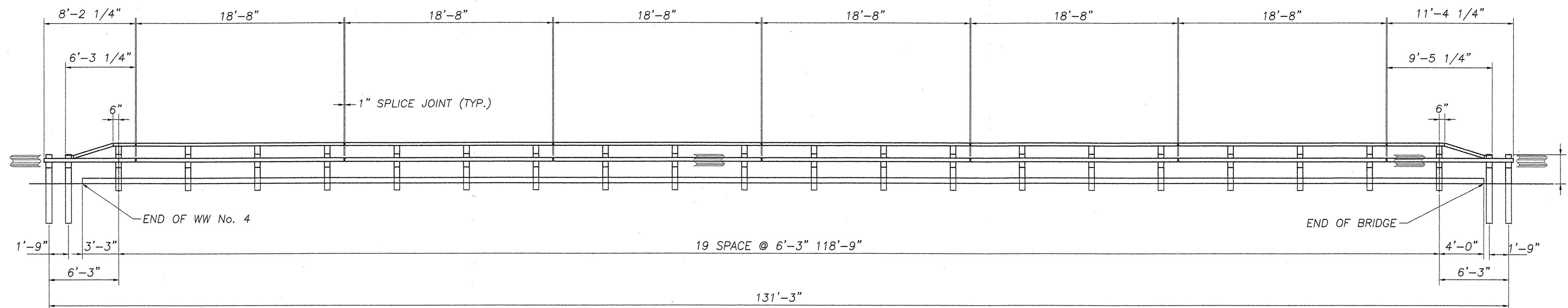
REVISIONS			ITEM NO. 506.50	
NO.	DATE	DESCRIPTION	FRANK W. WHITCOMB CONSTRUCTION CORP.	
7/14/04		FOR APPROVAL	CONTRACT : BND 1442 (32)	
			CHURCH STREET (TH 5) OVER WILLIAMS RIVER BRIDGE NO. 11 CHESTER, VT	
			TITLE : STRINGER DETAIL 7S3B	
			DRAWN BY 4/26/04 DANA BACON	SHOP ORDER 5025
			CHK BY JH 7/15/04	BLOCK NO. SHEET NO. 1 OF 1

035 SS

9457

2728





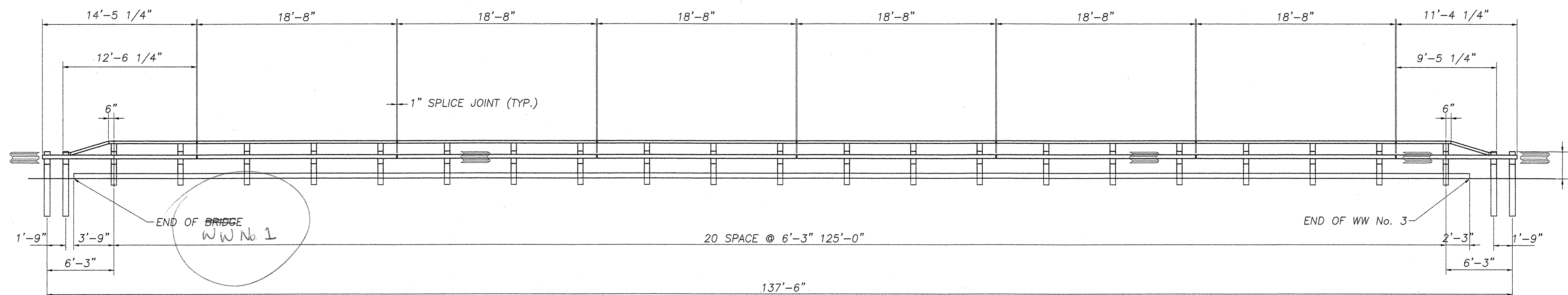
ELEVATION - RIGHT BRIDGE RAILING

BILL OF MATERIAL (BOTH SIDES OF BRIDGE)		
Qty.	Description	Material
41	FASCIA MOUNTED POST W8x24 x 4'-1"	A572 GR 50
8	TRANSITION POST W8x24 x 6'-0"	A572 GR 50
14	SPLICE CHANNEL C7x12.25 x 2'-7"	A572 GR 50
98	TUBE BLOCKOUTS TS 8x4x3/16 x 6" LG	A500 GR B
12	RAIL TUBE TS 8x4x3/16 x 18'-8" OAL	A500 GR B
1	RAIL TUBE TS 8x4x3/16 x 14'-5 1/4" OAL	A500 GR B
2	RAIL TUBE TS 8x4x3/16 x 11'-4 1/4" OAL	A500 GR B
1	RAIL TUBE TS 8x4x3/16 x 8'-2 1/4" OAL	A500 GR B
22	10 GA. HDSB RAIL 12'-6" / 6'-3"	M180 B2
1	10 GA. HDSB RAIL 6'-3"	M180 B2
12	HAND RAIL TUBE TS 6x3x1/4 x 18'-8" OAL	A500 GR B
1	HAND RAIL TUBE (DROP END) TS 6x3x1/4 x 12'-6 1/4"	A500 GR B
2	HAND RAIL TUBE (DROP END) TS 6x3x1/4 x 9'-5 1/4"	A500 GR B
1	HAND RAIL TUBE (DROP END) TS 6x3x1/4 x 6'-3 1/4"	A500 GR B
28	SPLICE PLATE - HAND RAIL TUBE 3/4"x5"x2'-7"	A572 GR 50
41	SHELF ANGLE L5x3 1/2x3/8 6 1/2" LG.	A36
49	5/8" x 13" HEX BOLT	A325
49	5/8" x 10" HEX BOLT	A325
236	5/8" x 2" HEX BOLT	A325
41	5/8" x 4 1/2" HEX BOLT	A325
56	3/4" x 4" HEX BOLT	A325
49	RECTANGULAR WASHER 3/16" x 1-3/4" x 3"	A572 GR 50
147	5/8" HEX NUT	A563 DH
228	5/8" HEX NUT	A563
56	3/4" HEX NUT	A563
56	3/4" FLAT WASHER	F436
563	5/8" ROUND WASHER	F436
176	5/8" x 1 1/4" PANEL SPLICE BOLT	A307
176	5/8" DBL RECESSED NUT	A563A
82	1" U-SHAPED ANCHOR RODS	A449
328	1" HEX NUT	A194 GR 2H
164	1/4" x 2" x 5" PLATE WASHER	A572 GR 50
328	1" PLAIN WASHER	F436

(*) PROVIDED BY OTHERS, NOT SUPPLIED BY HIGHWAY SAFETY CORPORATION. SHOWN FOR REFERENCE ONLY.

TOTAL ITEM 525.43 LENGTH = 269'

THE BILL OF MATERIALS HAS NOT BEEN CHECKED BY THE REVIEWER.



ELEVATION - LEFT BRIDGE RAILING

REVISIONS		
No.	Remarks	Date
0	Initial submittal	5/28/04

ACCEPTED
ACCEPTED AS CORRECTED
REJECTED, REVISE AND RESUBMIT
REJECTED, UNACCEPTABLE

Checked by: *[Signature]*
Date: 6/15/04

HIGHWAY SAFETY CORP.
GLASTONBURY, CT

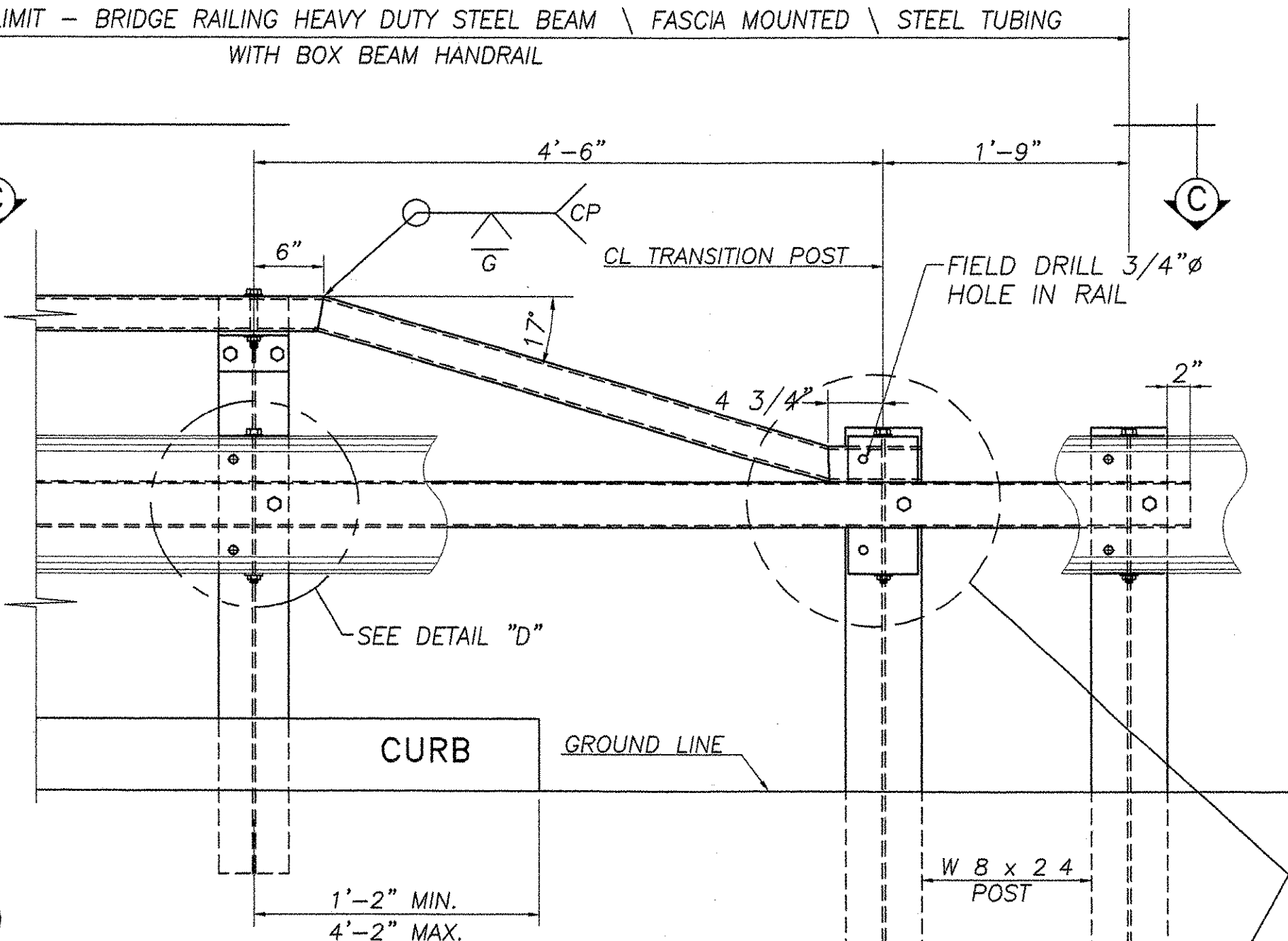
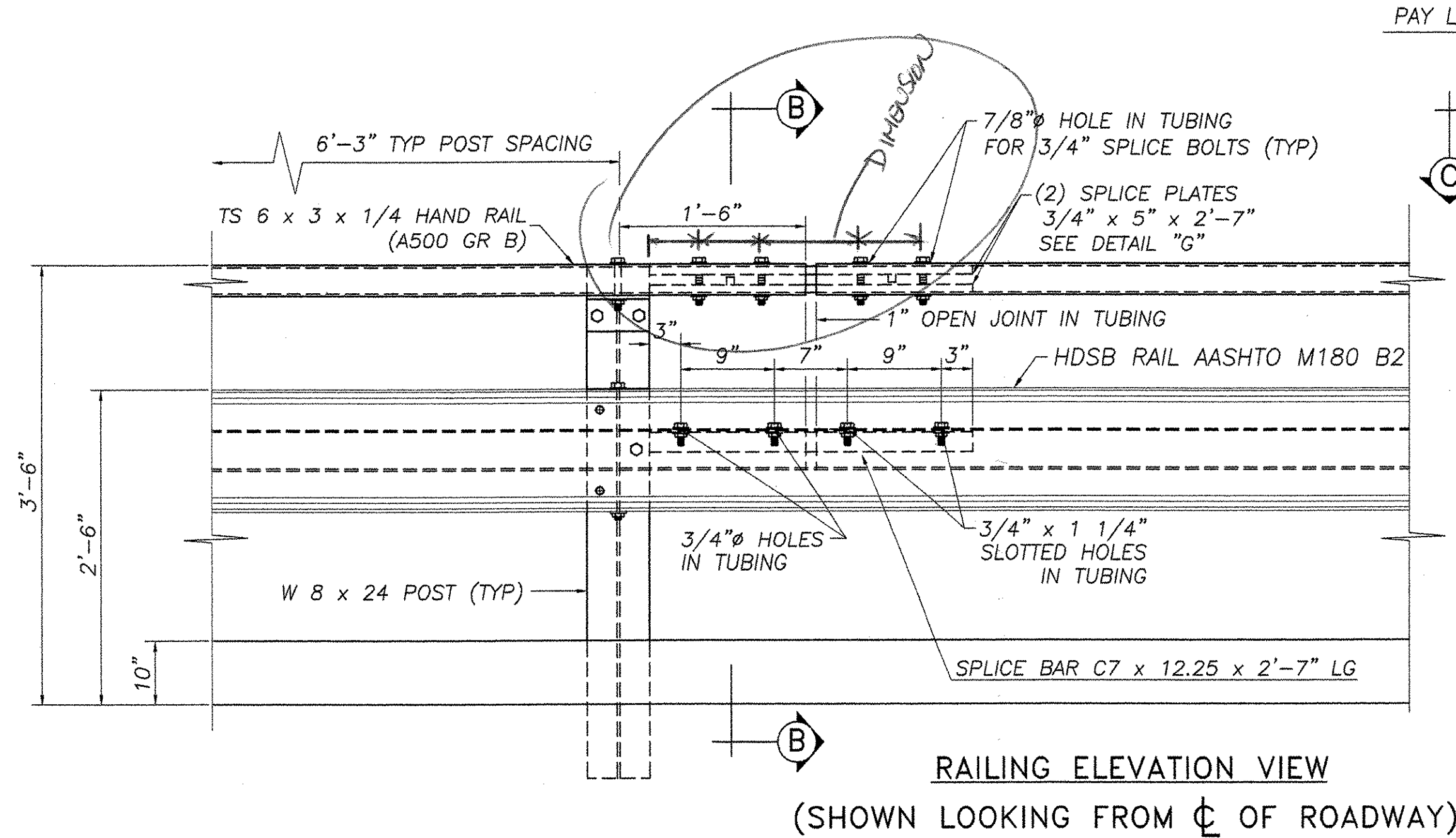
ITEM 525.43 STEEL BRIDGE RAIL - HDSB W/HANDRAIL
BRIDGE NO. 11 HIGHWAY NO. TH 5
TOWN OF CHESTER, WINDSOR COUNTY
CHURCH STREET OVER WILLIAMS RIVER
PROJECT NO. CH 3-9637

GENERAL CONTRACTOR: *[Signature]*
SUB CONTRACTOR: F.R. LAFAYETTE, INC.

DRAWN: MHM
CHECKED: *[Signature]*
DATE: 5/15/04
SCALE: N.T.S.
HSC REFERENCE NO.: 1445
SIZE: D
REVISION: 0
SHEET NO.: 1 of 2

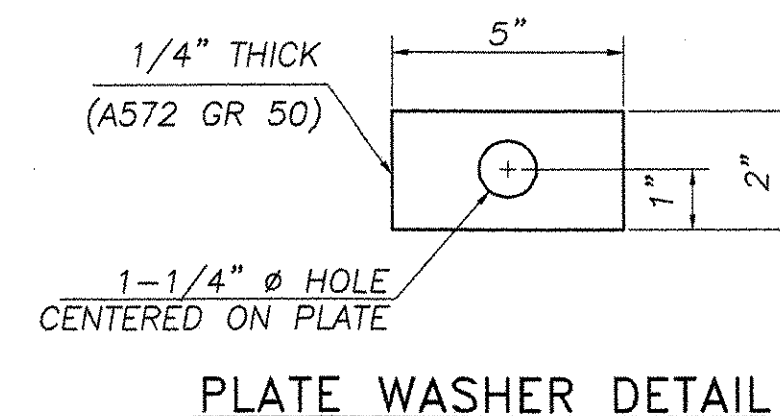
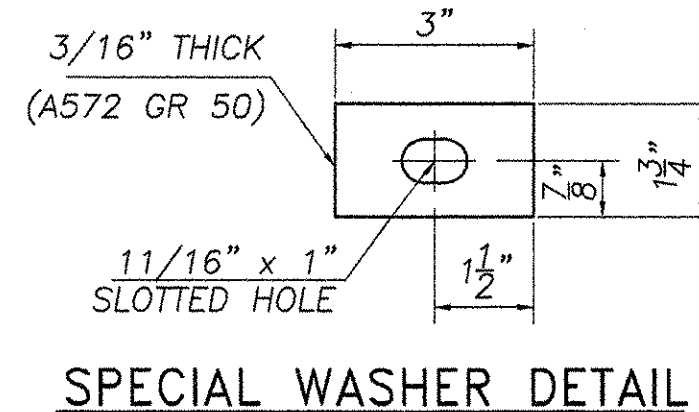
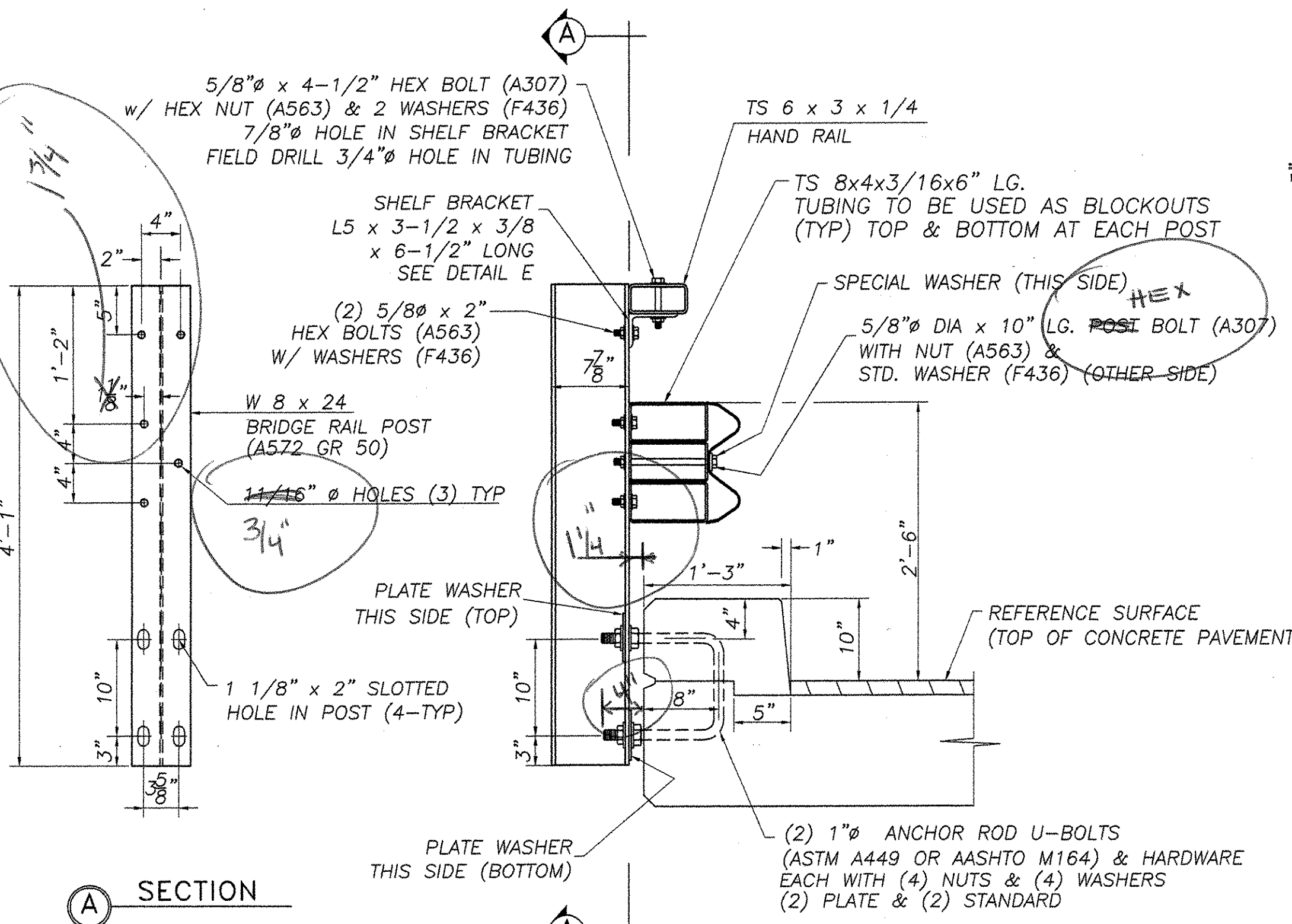
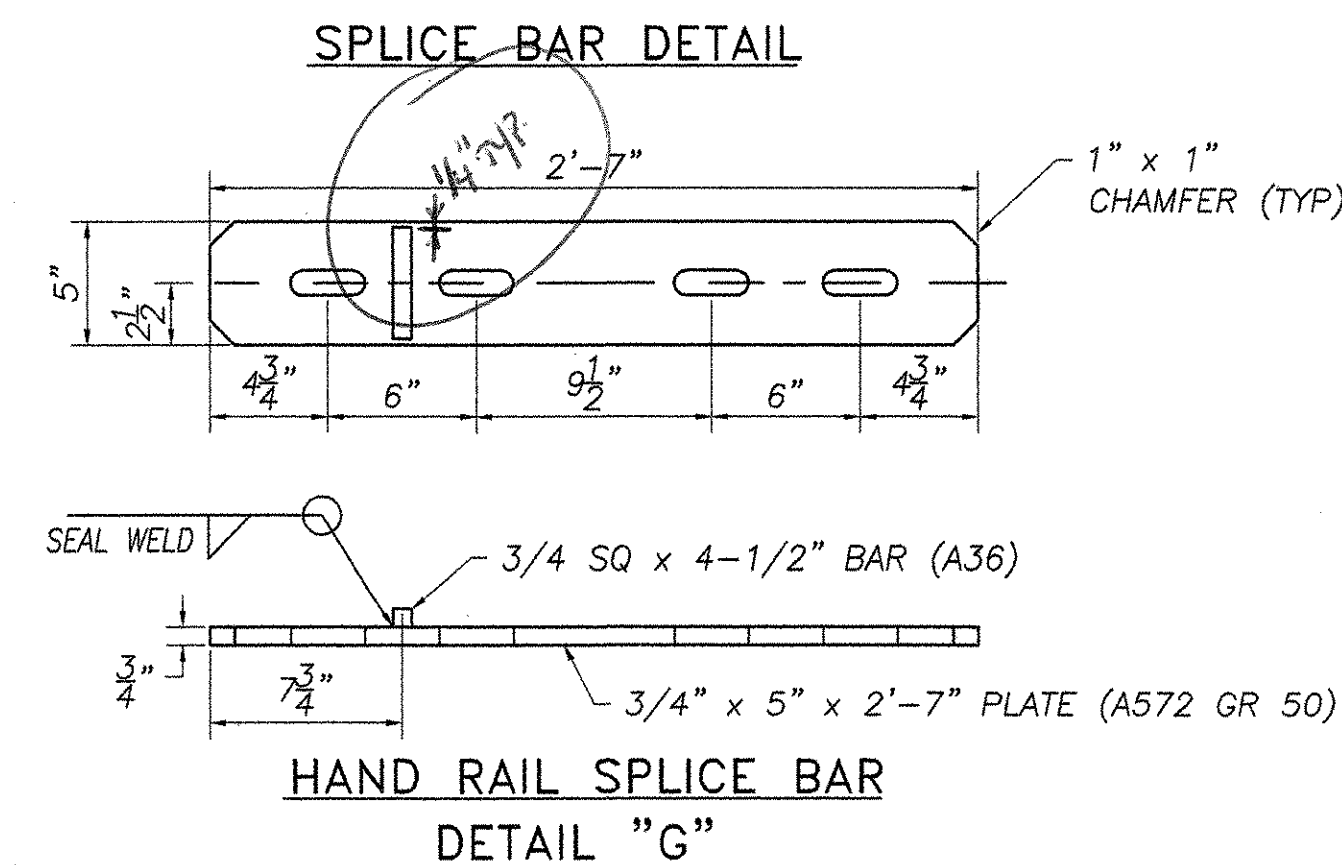
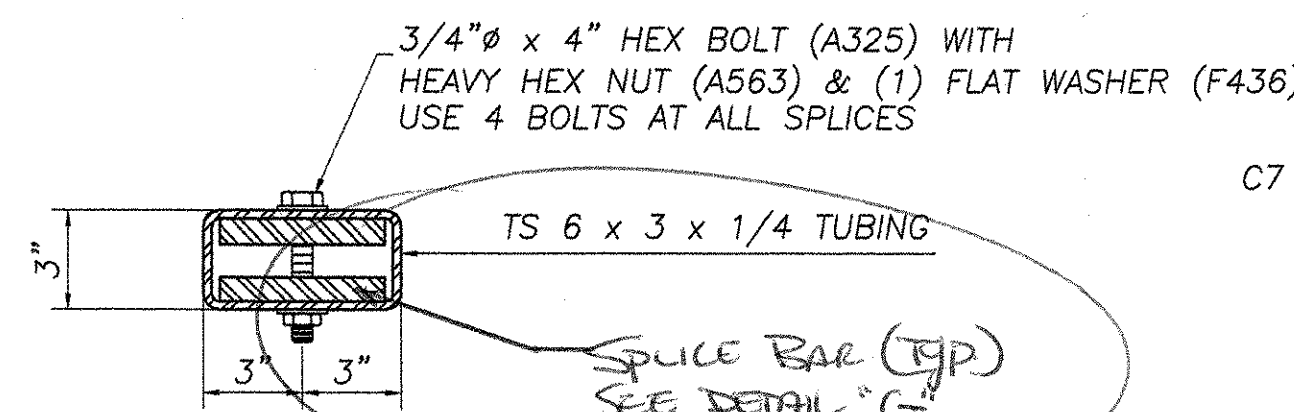
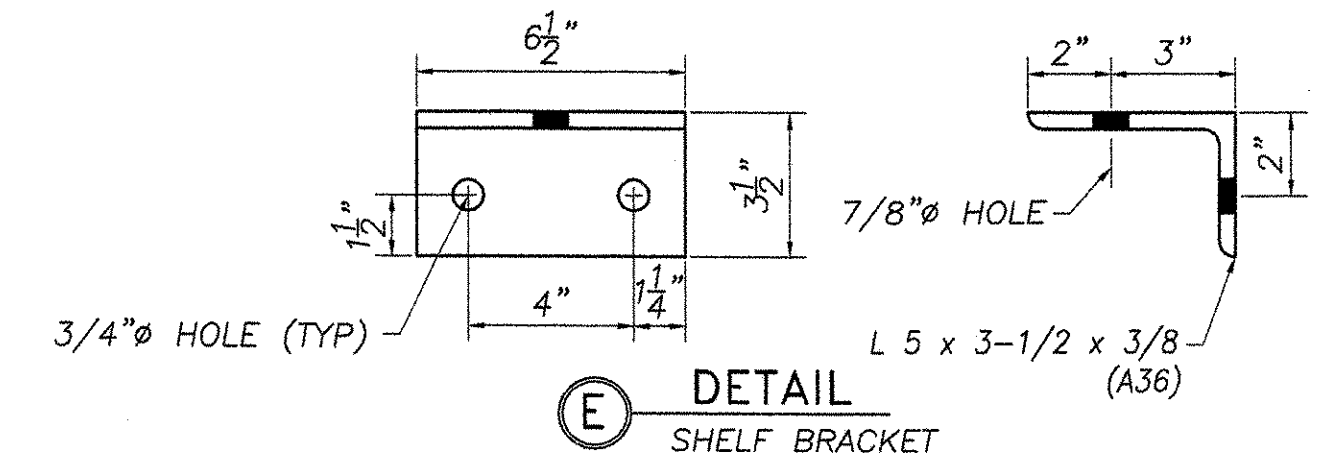
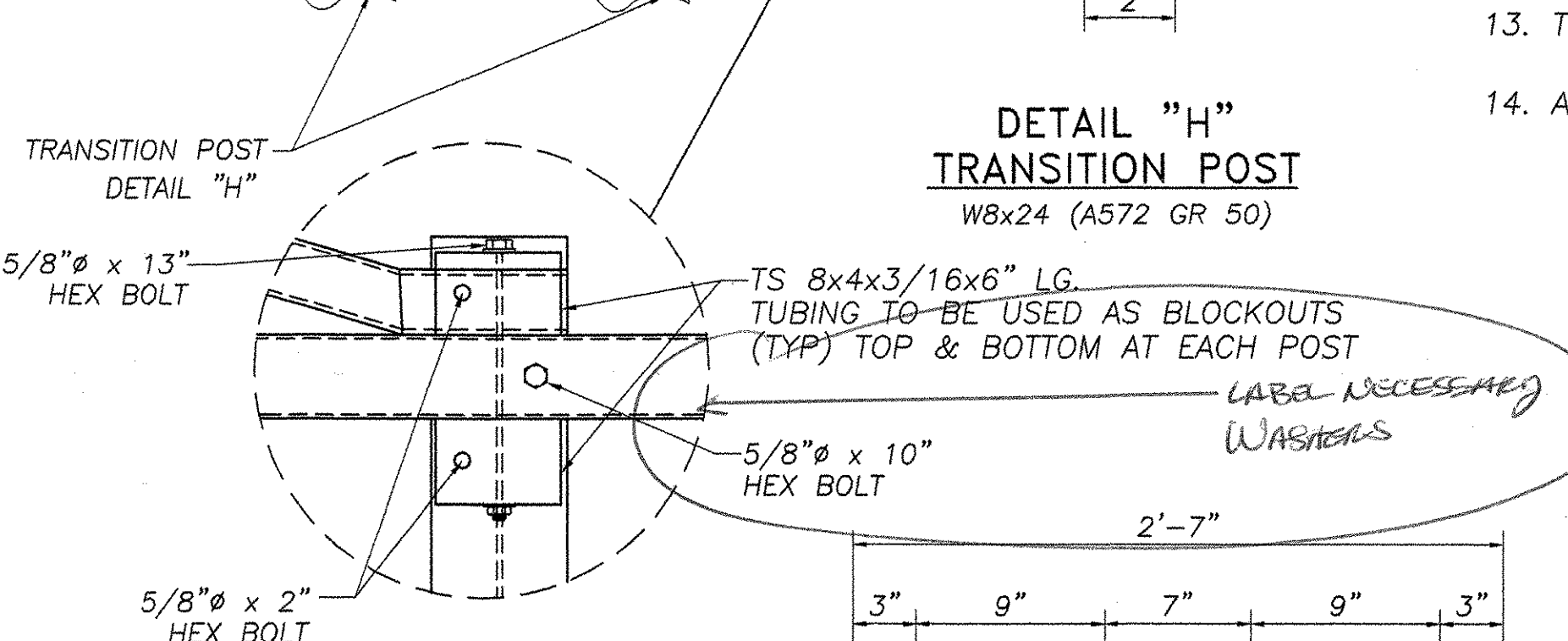
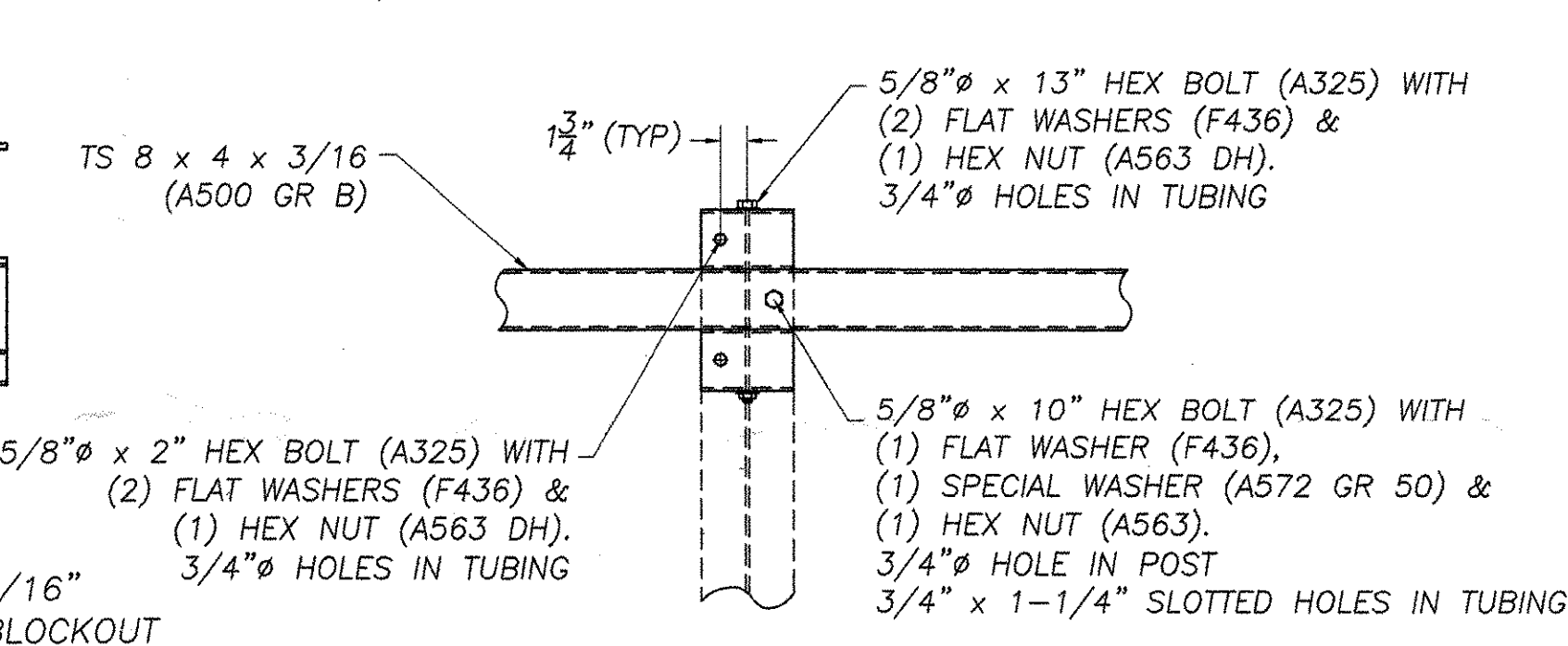
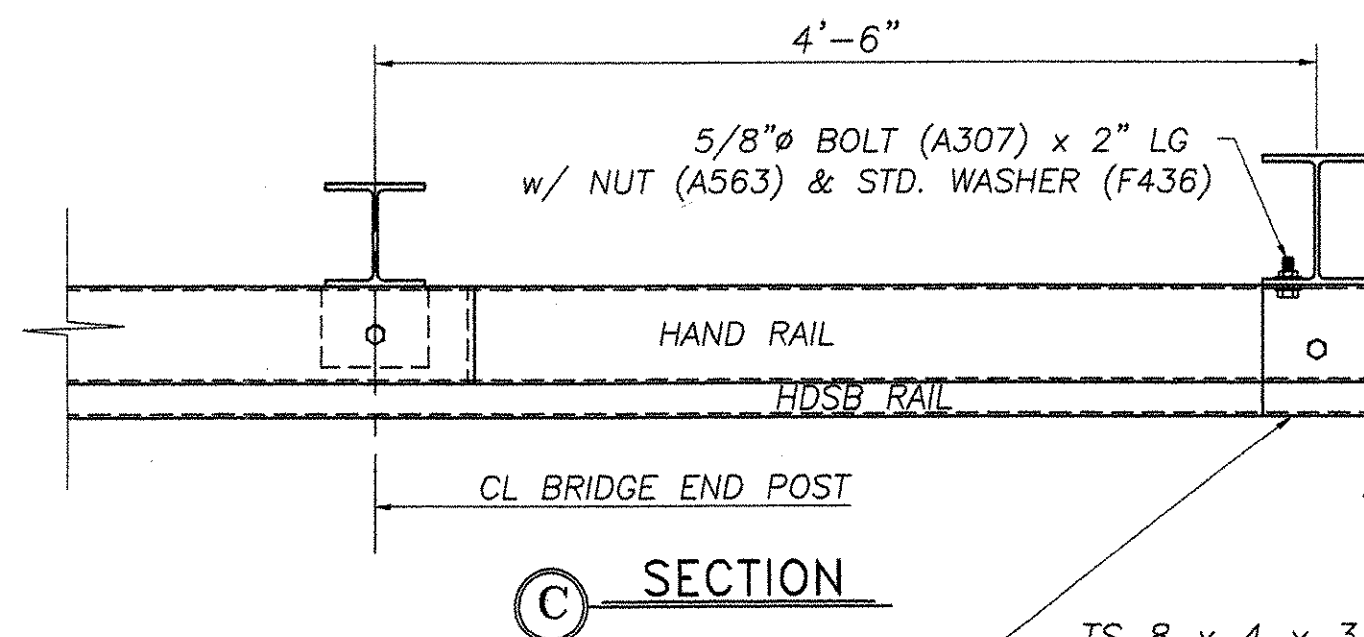


PAY LIMIT - BRIDGE RAILING HEAVY DUTY STEEL BEAM / FASCIA MOUNTED / STEEL TUBING
WITH BOX BEAM HANDRAIL



GENERAL ERECTION NOTES

1. Heavy duty steel beam guard rail shall conform to VT. specification 732. AASHTO M180 B2
2. Structural steel tubing shall conform to VT. specification 732. ASTM A500 gr B
3. Anchor bolts, nuts and washers shall be galvanized in accordance with AASHTO M 232 and shall conform to VT. specification 714 unless otherwise noted.
4. Bridge rail posts, special washers, splice bars and plate washers shall conform to AASHTO M 223 / M 223M and shall be galvanized after fabrication in accordance with AASHTO M 111. Prior to galvanizing all corners and edges of steel plates, shapes, etc., shall be ground to a 1/16" radius.
5. See standard drawing G-1 and G-1d for additional details concerning guard rail.
6. See standard drawing SB-R6-82 for approach rail details and for information relative to schedule I and schedule II. All approach rail shall be heavy duty steel beam guard rail. Also see Std. Dwg. SB-R6-82 for handrail details (except end details) if hand rail is required.
7. All posts shall be set normal to grade.
8. Splices for the steel beam guard rail shall lap in the direction of traffic.
9. See Standard drawing G-1 for details of delineators. A delineator shall be located at every fifth post. Payment shall be subsidiary to other items.
10. A railing joint splice shall be provided at each superstructure expansion joint.
11. All field cut or drilled areas shall be coated with zinc rich paint.
12. For radii less than 950 feet, the steel tubing shall be shop bent to fit the applicable curve.
13. The drop-weight tear test in section 732 shall not apply to the structural tubing on this standard.
14. All bolts and related hardware shall conform to AASHTO M164 type 1 bolts, hot dipped or mechanically galvanized per specification.



FASCIA MOUNTED POST WITH CURB
W 8 x 24 (A572 GR 50)

SPECIAL WASHER DETAIL

REVISIONS		
No.	Remarks	Date
0	Initial submittal	5/22/04

ACCEPTED
REJECTED, REVISE AND RESUBMIT
REJECTED, UNACCEPTABLE

6/15/04

By: RJA

HIGHWAY SAFETY CORP.
GLASTONBURY, CT

ITEM 525.43 STEEL BRIDGE RAIL - HDSB W/HANDRAIL
BRIDGE NO. 11 HIGHWAY NO. TH 5
TOWN OF CHESTER, WINDSOR COUNTY
CHURCH STREET OVER WILLIAMS RIVER
PROJECT NO. CH 3-9637

DRAWN: MHM
CHECKED: [Signature]
DATE: 5/15/04
SCALE: N.T.S.
HSC REFERENCE NO.: 1445
SIZE: D
REVISION: 0
SHEET NO.: 2 of 2

SUB CONTRACTOR: F.R. LAFAYETTE, INC.

